## BUJILISEAM OPERATIONS

APPENDIX K ROAD TRANSPORT ASSESSMENT

## road transport assessment

## relating to the proposed bulli seam operations

prepared for bhp billiton by traffix traffic \& transport planners ref: 07 267_v7 May 2009


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## 1. introduction

## 1.1 context

Illawarra Coal Holdings Pty Ltd (ICHPL) (a wholly owned subsidiary of BHP Billiton Pty Limited) owns and operates the Appin Mine and West Cliff Colliery. The longwall operations are supported by three pit tops (figure 1):

7 West Cliff pit top;
7 Appin East pit top; and
7 Appin West pit top.

This report has been prepared for ICHPL to present the results of an assessment of the traffic implications of a proposal to continue operations at the Appin Mine and West Cliff Colliery for some 30 years and to increase run-of-mine (ROM) coal production from 7.5 million tonnes per annum (Mtpa) up to 10.5 Mtpa . The proposed development is known as the Bulli Seam Operations (the Project).

This report has been prepared by TRAFFIX for ICHPL as part of the Environmental Assessment (EA) for the Project. This study has been undertaken with reference to the DirectorGeneral's Environmental Assessment Requirements (EARs) for the Project issued by the New South Wales (NSW) Department of Planning (DoP) on 18 August 2008. In relation to road transport, the EARs require a detailed assessment, as follows:
"Transport - including a detailed assessment of the potential impacts of the Project on the safety and performance of the road network."

As part of the assessment process an environmental risk assessment (Appendix $N$ of the EA) was undertaken. This included a facilitated, risk based workshop involving experts across a range of disciplines and experienced ICHPL personnel. The objective of the assessment was to identify key potential environmental issues for inclusion in the EA. Transport/traffic volume increases was identified as a key potential environmental issue.

This report has been compiled to address the EARs and the road transport related issue identified in the environmental risk assessment. The assessment has also been prepared with reference to the NSW Road and Traffic Authority (RTA) Guide to Traffic Generating Developments (2002) and the RTA Road Design Guide (1996), where relevant.


Mine Subsidence Engineering Consultants (MSEC) (2009) has identified key items of road infrastructure located above the Project longwall mining area, and includes predicted subsidence impacts which are described in the Subsidence Assessment (Appendix A of the EA). This report specifically assesses the potential road transport impacts associated with Project-related traffic movements.

Port Kembla Coal Terminal (PKCT) is a major coal export facility in southern NSW, located in the Inner Harbour of Port Kembla, near Wollongong, and is a destination for delivery of coal from the Appin Mine and West Cliff Colliery. The terminal is responsible for receiving, assembling and loading coal from the Southern and Western NSW coalfields, which is transported by ship to international and domestic markets. PKCT submitted an EA to DoP in 2008, which assessed increased road receivals and a change to delivery hours for coal deliveries by public road to PKCT, and is subject to separate approval.

Cardno Eppell Olsen (2008) prepared a Traffic Study for the PKCT EA, which assessed the potential road transport impacts associated with the PKCT proposal. PKCT is currently seeking approval to remove road receival restrictions and increase public road receivals to 24 hours per day, 7 days per week for a maximum of 10 Mtpa of coal received by public road. For the purposes of this assessment, it is assumed that the PKCT EA will be approved ${ }^{1}$. The PKCT Traffic Study included assessment of coal truck movements associated with existing ICHPL operations on the route from the Appin East pit top and West Cliff pit top to PKCT, and coal deliveries associated with other companies.

However, this assessment only considers Project traffic associated with the Appin Mine and West Cliff Colliery. The Project would operate within the road receival limits specified in the PKCT EA. Data from the PKCT Traffic Study (Cardno Eppell Olsen, 2008) has been used in this report and is referenced where incorporated.

## 1.2 existing operations

The Appin Mine is accessed via the Appin West and Appin East pit tops. The West Cliff Colliery is accessed via the West Cliff pit top. The West Cliff and Appin East pit tops are located on Appin Road, as shown on figure 1. The Appin West pit top is located on Douglas Park Drive, approximately 4 kilometres (km) south of Douglas Park (figure 1). The Appin Mine and West Cliff Colliery currently produce up to 7.5 million tonnes (Mt) of ROM coal per year. ROM coal from the Appin Mine is brought to the surface at the Appin East pit top. The Appin West pit top currently only provides access to the underground mining operations for underground personnel and mine equipment and supplies. ROM coal is transported from the Appin East pit top to the West Cliff pit top by road.

[^0]The location of the three pit tops are shown on figure 1 which also includes the destinations of coal as discussed below. A schematic description of the operations is provided on figure 2.

The majority of coal is currently delivered by truck from the West Cliff pit top to PKCT or to the BlueScope Steelworks (also at Port Kembla) for transport to domestic and overseas customers. Road haulage operations occur up to 24 hours per day, seven days per week. A small volume of ROM coal is also transported by road to the Dendrobium Washery located at the BlueScope Steelworks (product coal from Dendrobium Washery is then transported via internal roads to PKCT). Small volumes of coal for the domestic market is also transported by road to Corrimal Coke Works, Coalcliff Coke Works and other minor customers from time to time.

Coal wash from the West Cliff Washery is hauled by internal road to West Cliff Coal Wash Emplacement for disposal. Dendrobium Washery coal wash is also transported by road to the West Cliff Coal Wash Emplacement by loading empty coal haulage trucks that have delivered coal and would otherwise return to the West Cliff pit top empty.

The existing operational workforce at the Appin Mine and West Cliff Colliery comprises up to 875 people.

## 1.3 summary project description

The main activities associated with the development of the Project would include:

7 continued development of underground mining operations within existing coal leases and new mining leases to facilitate a total ROM coal production rate of up to 10.5 Mtpa ;
7. ongoing exploration activities within existing exploration tenements;

7 upgrade of the existing West Cliff Washery to support the increased ROM coal production;

7 continued mine gas drainage and capture for beneficial utilisation at the West Cliff Ventilation Air Methane Project and Appin-Tower Power Project;

7 continued generation of electricity by the existing Appin-Tower Power Project (owned and operated by Energy Development Limited utilising coal bed methane drained from the Bulli Seam;

7 upgrade of existing surface facilities and supporting infrastructure (e.g. service boreholes, ventilation shafts, gas drainage equipment, waste water treatment and waste water disposal);
7. continued and expanded placement of coal wash at the West Cliff Coal Wash Emplacement;


7 continued road transport of ROM coal from the Appin East pit top to the West Cliff Washery;

7 continued road transport of ROM coal from Appin East pit top and West Cliff pit top via the public road network to the Dendrobium Washery at Port Kembla;

7 continued road transport of product coal from the West Cliff Washery via the public road network to BlueScope Steelworks, PKCT, Corrimal and Coalcliff Coke Works and other customers;

7 ongoing surface monitoring and rehabilitation (including rehabilitation of mine related infrastructure areas that are no longer required) and remediation of subsidence effects; and

7 other associated minor infrastructure, plant, equipment and activities.

A detailed description of the Project is provided in Section 2 in the Main Report of the EA.

The annual rate of coal delivery to PKCT and BlueScope Steelworks would increase as a result of the Project. Deliveries to Corrimal and Coalcliff Coke Works, and Dendrobium Washery would also marginally increase above current levels. Delivery of coal to PKCT and BlueScope Steelworks would be up to approximately 7.5 Mtpa and 4 Mtpa , respectively. Coal deliveries to Corrimal and Coalcliff Coke Works would be up to 0.2 Mtpa (combined), and delivery of coal to Dendrobium Washery would be up to 0.5 Mtpa . While these are maximum anticipated deliveries to any destination, the peak Project coal delivery would not be more than 9.4 Mtpa.

The annual road movements on these routes would be increased from recent maximum levels. Operations in 2007 resulted in the highest recent trucking movements to PKCT (3.4 Mtpa), BlueScope Steelworks (1.9 Mtpa), Corrimal/Coalcliff Coke Works ( 0.15 Mtpa ) and Dendrobium Washery ( 0.4 Mtpa ). The potential impacts of the Project with regard to traffic movements would therefore be a result of additional coal haulage primarily to PKCT and BlueScope Steelworks, and would include some additional operational workforce traffic, additional operational deliveries and the continuation of these existing haulage activities over the extended life of the Project.

There is no rail traffic associated with the existing operations or the proposed Project.

The Project construction phase would require approximately 100 short-term employees (i.e. a total of some 975 employees). The workforce for the operational phase of the Project would be increased to 1,170 people. The maximum potential traffic generation associated with the operational phase of the Project has been assessed for the purposes of this road transport assessment.

## 2. existing traffic conditions

## 2.1 road classifications

Roads are classified according to a road hierarchy, in order to determine their functional role within the road network. Changes to traffic flows on the roads can then be assessed within the context of the road hierarchy. Roads are classified according to the role they fulfil and the volume of traffic they should appropriately carry given their classification. The RTA has set down the following guidelines for the functional classification of roads as stated in Table 1.2.5 of the RTA Road Design Guide (1996):
$\pi \quad$ Arterial Road: typically a main road fulfilling a role as a major inter-regional link (no limit to the traffic volumes).
$\pi$ Sub-arterial Road: defined as secondary inter-regional links, typically carrying volumes between 5,000 and 20,000 (max) vehicles per day ( 500 to 2,000 vehicles per hour).
„ Collector Road:
$\pi$ Local Road:
provides a link between local roads and regional roads, typically carrying between 2,000 and 10,000 vehicles per day ( 250 to 1,000 vehicles per hour). At volumes greater than 5,000 vehicles per day, residential amenity begins to decline noticeably.
provides access to individual allotments, carrying low volumes, typically less than 2,000 vehicles per day ( 250 vehicles per hour).

The RTA has also adopted a classification system relating to funding purposes. It defines roads as:

| \# State Roads: | performing an important state function for which the RTA funds <br> one hundred percent of the maintenance cost. State roads are <br> essentially arterial roads. |
| :--- | :--- |
| $\Rightarrow \quad$Regional Roads: <br> roads performing a significant regional function and for which the <br> RTA and relevant Council contribute fifty percent each towards <br> maintenance. Regional roads are essentially sub-arterial roads. |  |
| Local Roads: <br> relevant Council funds one hundred percent of the maintenance <br> cost. |  |

## 2.2 road hierarchy

The road hierarchy serving the wider regional location is shown on figure 3. A description of key regional roads within the context of the wider Project area is provided below:

| F6 Southern Freeway: | The F6 Southern Freeway is an arterial road, which provides <br> the major link road between Sydney and Wollongong/Port <br> Kembla. The freeway extends southwards from Princes <br> Highway at Waterfall and follows the Woronora Plateau to <br> the Bulli Tops Interchange, where it meets the Princes |
| :--- | :--- |
| Highway, Appin Road and Mount Ousley Road (which forms |  |
| part of the freeway itself). The F6 Southern Freeway then |  |
| descends the Illawarra Escarpment and parallels the |  |
| Wollongong metropolitan area for its entire length. Once on |  |
| the coastal plain, the freeway continues south for a further |  |
|  | 20 km, where it again meets Princes Highway to the north of |
| Yallah. |  |



今 $\qquad$ 5km road transport assessment: bulli seam operations project road which provides a major link between Sydney and Campbelltown, from where it continues south to Albury on the Victorian border before continuing on to Melbourne.

7 Masters Road: Masters Road is an arterial road (Main Road No. 602) which connects Springhill Road to the F6 Southern Freeway and The Avenue further to the west.

Springhill Road is an arterial road (Main Road No. 581) that connects Five Islands Road to the south with Corrimal Road in the north. It primarily serves the Port Kembla industrial facilities in addition to providing an important access route to the Wollongong centre from the south.

7 Northern Distributor: The Northern Distributor is an arterial road (Main Road No. 626) through Wollongong's northern suburbs, extending from the F6 Southern Freeway at Gwynneville to Bellambi Lane at Bellambi.

7 Lawrence Hargrave Drive: Lawrence Hargrave Drive is a major sub-arterial route (Main Road No. 185) which generally runs along the coastline from the F6 Southern Freeway in the north to Bulli in the south.

## 2.3 roads on key haulage routes

The routes used for on-road haulage from the Appin Mine and the West Cliff Colliery are summarised in table 1, and are described further below.
table 1: key haulage routes

| Route | Destination |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | West Cliff pit top (i.e. from Appin East pit top) | PKCT | BlueScope Steelworks | Dendrobium Washery | Corrimal Coke Works | Coalcliff Coke Works |
| Appin Road | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Mount Ousley Road | - | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | - |
| F6 Southern Freeway (South) | - | $\checkmark$ | $\checkmark$ | $\checkmark$ | $\checkmark$ | - |
| Masters Road | - | $\checkmark$ | $\checkmark$ | $\checkmark$ | - | - |
| Springhill Road | - | $\checkmark$ | $\checkmark$ | $\checkmark$ | - | - |
| University Avenue/ Graham Avenue | - | - | - | - | $\checkmark$ | - |
| Northern Distributor | - | - | - | - | $\checkmark$ | - |
| Railway Street | - | - | - | - | $\checkmark$ | - |
| F6 Southern Freeway (North) | - | - | - | - | - | $\checkmark$ |
| Lawrence Hargrave Drive | - | - | - | - | - | $\checkmark$ |
| Princes Highway | - | - | - | - | - | $\checkmark$ |

## Appin Road

Appin Road provides the main vehicular access for the township of Appin. To the north of Appin, it is also known as Narellan-Appin Road. In the Campbelltown area, Appin Road is a divided road, with two or three travel lanes in each direction, and additional turn lanes at major intersections, which are signal controlled. It has a speed limit of 80 kilometres per hour ( $\mathrm{km} / \mathrm{hr}$ ). Beyond the urban area, the speed limit is $70 \mathrm{~km} / \mathrm{hr}$, and the carriageway reduces to an undivided road with a single travel lane in each direction. In the rural area between Campbelltown and Appin, it has a speed limit of $80 \mathrm{~km} / \mathrm{hr}$, and a single travel lane in each direction. The road is undulating, with no particularly steep grades or sharp bends. In the township of Appin, the speed limit is $50 \mathrm{~km} / \mathrm{hr}$, with a $40 \mathrm{~km} / \mathrm{hr}$ school zone, and some kerbside parking is permitted.

To the east of Appin, it is also known as Bulli-Appin Road. Between Appin and Bulli Tops, Appin Road has one or two travel lanes in each direction, with a $100 \mathrm{~km} / \mathrm{hr}$ speed limit. This section is undulating, with no particularly steep grades or sharp bends.

Appin Road forms the primary access road between Appin East pit top and West Cliff pit top, and links to Mount Ousley Road. A private access road is located along Appin Road for accessing West Cliff pit top, whilst several public roads can be used to access Appin East pit top from Appin Road. Haulage trucks access Appin East pit top via Sheriff Road (entry) and George Street (exit). The section of Appin Road east of Appin carries haulage trucks to PKCT, BlueScope Steelworks, Dendrobium Washery and Corrimal and Coalcliff Coke Works. Appin Road also carries delivery and employee traffic coming from Wollongong via Mount Ousley Road and the Princes Highway, traffic from Sydney via the F6 Southern Freeway and local traffic from Campbelltown and northern areas.

## Mount Ousley Road

Mount Ousley Road consists of many steep descents; a posted speed limit of $40 \mathrm{~km} / \mathrm{h}$ applies for heavy vehicles on descent from Clive Bissell Drive to the F6 Southern Freeway. North of Picton Road, Mount Ousley Road consists of a four-lane carriageway, divided by a jersey barrier. Steep grades characterise the road, and extensive realignment works have been undertaken during the period 1970 to 1992. Noise barriers are also provided on Mount Ousley Road adjacent to the residential areas.

South of Picton Road, Mount Ousley Road commences a 6 km descent from the escarpment to the coastal plain. An additional lane for slow vehicles is provided along this section for northbound traffic, and an additional southbound lane is also provided over the first 3 km . At the foot of the escarpment, the route splits in two with most traffic heading to the right along the F6 Southern Freeway. Mount Ousley Road continues to the left to meet the Princes Highway at Fairy Meadow.

This section of the route is constructed to a good standard with a high level of traffic management and safety devices provided, these include:

л a jersey kerb median safety barrier;
„ lane line and edgeline markings;
л guideposts and reflectors;
„ high level of advisory and regulatory signage; and
$\pi$ emergency stopping lane and arrestor beds plus a safety ramp on the Mount Ousley Road descent for southbound vehicles.

The main intersections are located at:
„ Clive Bissell Drive;
л New Mount Pleasant Road; and
л Mount Ousley Road/F6 Southern Freeway.

Interchanges are provided at:
„ Picton Road;
7 Princes Highway; and
入 Appin Road.

Mount Ousley Road carries haulage trucks to PKCT, BlueScope Steelworks, Dendrobium Washery and Corrimal Coke Works. Mount Ousley Road also carries delivery and employee traffic coming from Wollongong.

## F6 Southern Freeway

The F6 Southern Freeway forms part of the arterial route linking Sydney and Wollongong, and is split into two sections; Waterfall to Bulli Tops and Mount Ousley to Yallah. It carries the name "F6" as that was both its legal classification and route number for many years.

The section of the F6 Southern Freeway between Mount Ousley Road and Masters Road is predominantly four lanes with a jersey barrier median. Two additional lanes are provided between the Northern Distributor and Princes Highway interchanges.

The speed limits are $80 \mathrm{~km} / \mathrm{h}$ (Mount Ousley Road to Gipps Road), $90 \mathrm{~km} / \mathrm{h}$ (Gipps Road to Princes Highway) and $100 \mathrm{~km} / \mathrm{h}$ (south of Princes Highway). Access is restricted along the length of the route as is typical of a freeway. A central median jersey kerb is provided along the route with standard freeway treatments including lane line and edgeline markings, street lighting and pavement reflectors.

Interchanges or grade separations are provided at:
„ Masters Road;
„ Mount Keira Road;
„ Princes Highway; and
„ Northern Distributor.

Adjoining development and landuses along the route include the University of Wollongong, industrial and residential areas. Noise barriers are provided along residential and noise sensitive areas of the F6 Southern Freeway.

This section of road carries haulage trucks to PKCT, BlueScope Steelworks, Dendrobium Washery and Corrimal Coke Works, as well as providing a link for workforce and delivery traffic. The F6 Southern Freeway carries delivery and employee traffic coming from south of Wollongong, and traffic coming from Sydney to the north.

## Masters Road

Masters Road is an arterial road connecting the F6 Southern Freeway and Springhill Road. It consists of dual carriageways with a total of six lanes with a posted speed limit of $80 \mathrm{~km} / \mathrm{h}$. Access to Masters Road is restricted to the intersection with Drummond Street with a separate interchange provided at The Avenue.

Masters Road was constructed in 1978 to provide a direct connection between the F6 Southern Freeway and Springhill Road, to eliminate heavy vehicles from the Mount St Thomas residential area. The development along Masters Road is largely industrial. At its western end, Masters Road meets the F6 Southern Freeway and The Avenue at a grade separated interchange. At its eastern end, Masters Road meets Springhill Road at an expansive signalised 'T-junction' with slip lanes (also signalised) provided for left turn movement to/from Masters Road.

This section of road carries haulage trucks to PKCT, BlueScope Steelworks and Dendrobium Washery. Masters Road is not exposed to a significant level of delivery or employee traffic associated with the existing Appin Mine and West Cliff Colliery.

## Springhill Road

Springhill Road is an arterial road connecting Masters Road and PKCT, as well as being part of the main link from Wollongong to Port Kembla, Warrawong and Shellharbour. The road was purpose built as a high capacity, access restricted route as a result of port development in the period 1955-1961. It is a single carriageway with six lanes, with a posted speed limit of $80 \mathrm{~km} / \mathrm{h}$. There are four signalised intersections along the section of interest at:

## „ Masters Road intersection;

„ Tom Thumb Road intersection;
„ Port Kembla Road intersection; and
$\pi$ Five Islands Road intersection.

Springhill Road north of Masters Road carries haulage trucks to PKCT. Springhill Road south of Masters Road carries haulage trucks to BlueScope Steelworks and Dendrobium Washery. Springhill Road is not exposed to a significant level of delivery or employee traffic associated with the existing Appin Mine and West Cliff Colliery.

## University Avenue/Graham Avenue

University Avenue/Graham Avenue is a local road that carries a single lane of traffic in either direction. It provides a link between the F6 Southern Freeway and the Northern Distributor. This length of University Avenue/Graham Avenue is used by haulage trucks on route to the Corrimal Coke Works from the West Cliff pit top. A roundabout with left turn slip lane is provided at the grade interchange of the F6 Southern Freeway off-ramp (southbound) with University Avenue.

## Northern Distributor

The Northern Distributor is an arterial road through Wollongong's northern suburbs, extending from the F6 Southern Freeway at Gwynneville to Bellambi Lane at Bellambi. Extension of the distributor to Princes Highway at Molloy Street, Bulli, is underway and due for completion in 2009, however these changes occur north of the section of the Northern Distributor that is required to access the Corrimal Coke Works.

The Northern Distributor is a dual carriageway with two lanes in each direction. Access to the Northern distributor is restricted along its length. There are four signalised intersections and three grade-separated interchanges along the route. The intersections are at Bellambi Lane, Rothery Street, Railway Street, and Towradgi Road. The interchanges are at Princes Highway, University Avenue, and the F6 Southern Freeway.

The Northern Distributor carries haulage trucks to the Corrimal Coke Works on Railway Street. The Northern Distributor is not exposed to a significant level of delivery or employee traffic associated with the existing Appin Mine and West Cliff Colliery.

## Railway Street

Railway Street serves as an access road for Corrimal Coke Works. It generally carries a single lane of traffic in either direction, however two lanes are provided on approach to the Northern Distributor. Railway Street carries haulage trucks to the Corrimal Coke Works which is accessed from the southern side of Railway Street, to the east of the Northern Distributor. It is not exposed to a significant level of delivery or employee traffic associated with the existing Appin Mine and West Cliff Colliery.

## Lawrence Hargrave Drive

Lawrence Hargrave Drive is a sub-arterial road linking the Princes Highway at Bulli with the Royal National Park and the Southern Freeway at Helensburgh. Lawrence Hargrave Drive is predominantly a two-lane undivided road, with a short section at Thirroul having additional lanes, in an environment that changes from urban to rural.

It provides the only vehicular access to numerous localities along the coast, such as Stanwell Park, Coalcliff and Wombarra. Its intersection with Princes Highway at Helensburgh is controlled with a roundabout. At the intersection of Lawrence Hargrave Drive with Otford Road/Lady Wakehurst Drive, Lawrence Hargrave Drive has a hairpin bend. Priority is given to northbound traffic on Lawrence Hargrave Drive, with "Stop" controls for vehicles approaching on Lawrence Hargrave Drive from the west and on Lady Wakehurst Drive from the east. Lawrence Hargrave Drive follows a steep grade down southbound to Stanwell Park. It has a $60 \mathrm{~km} / \mathrm{hr}$ speed limit, and has double centre lines along most of its length, and kerbside parking is not permitted.

Lawrence Hargrave Drive carries haulage trucks to the Coalcliff Coke Works at Coalcliff. Lawrence Hargrave Drive is not exposed to a significant level of delivery or employee traffic associated with the existing Appin Mine and West Cliff Colliery.

## 2.4 local area roads

The following discussion provides a review of roads in the proximity of the Appin Mine and West Cliff Colliery that are not used for coal haulage ${ }^{1}$.

## F5 South Western Freeway/Hume Highway

The F5 South Western Freeway/Hume Highway is an arterial road which provides a major link between Sydney and Campbelltown/Mittagong. The F5 South Western Freeway/Hume Highway provides a link for employee and delivery traffic from the local Wollondilly area (e.g. Camden/Narellan and Mittagong) and Sydney/Newcastle. The F5 South Western Freeway/Hume Highway does not carry any coal haulage trucks associated with the existing operations or the proposed Project.

## Princes Highway

The Princes Highway is an arterial route through Wollongong's northern suburbs, stretching from south of the CBD to the top of Bulli Pass. Two small sections of this road are used by haul trucks returning from the southern and northern haul routes at the interchange with Mount Ousley Road/Appin Road. A small section (approximately 600 metres [m]) of this road is used by haulage trucks returning from the south and approximately 1 km is used by haulage trucks returning from the north. The Princes Highway actually follows the Northern Distributor and Bellambi Lane from North Wollongong to Bellambi, however signage does not indicate this. It is a state road and classified as State Highway No. 1. The applicable speed limit is $60 \mathrm{~km} / \mathrm{h}$, with the exception of the $40 \mathrm{~km} / \mathrm{h}$ school zone outside Bulli Primary School.

North of Bellambi Lane, the Princes Highway is a four-lane undivided road with on-street parking permitted in some locations. Peak period clearway restrictions apply to maximise available traffic capacity. The section between Bellambi Lane and Hospital Road will be bypassed by the extension of the Northern Distributor in 2009.

Princes Highway is an arterial road providing a link between Bulli and Bulli Tops over what is known as the Bulli Pass. It connects the coastal region below the escarpment, north of Wollongong, to the F6 Southern Freeway/Mount Ousley Road and Appin Road. It provides a link for employee and delivery traffic from the Wollongong area (i.e. traffic travelling up the Bulli Pass from north of Fairy Meadow) to Appin Road. Except for a small section of the interchange with Appin Road, the Princes Highway does not carry any coal haulage trucks associated with the existing operations or the proposed Project.

[^1]
## Picton Road

Picton Road provides a major link between Picton and Mount Keira Road (and on to Mount Ousley Road via Link Road). It runs south from Picton through Maldon, under the F5 South Western Freeway/Hume Highway, and past Wilton to meet Mount Ousley Road north of Clive Bissell Drive. It provides a link for employee and delivery traffic from the local Wollondilly area (e.g. Bargo, Tahmoor, Mittagong and Picton), and collects traffic coming from Remembrance Driveway, the Hume Highway and Menangle Road towards the Appin Mine and West Cliff Colliery. Picton Road does not carry any coal haulage trucks associated with the existing operations or the proposed Project.

## Mount Keira Road

Mount Keira Road provides sub-arterial road function between Wollongong West and Picton Road. It runs north-west from the Princes Highway in West Wollongong and ascends the Illawarra Escarpment before it meets Picton Road. It provides a link for employee and delivery traffic from the Wollongong area via Picton Road. Mount Keira Road does not carry any coal haulage trucks associated with the existing operations or the proposed Project.

## Wilton Road

Wilton Road provides a link between the rural townships of Appin and Wilton. It runs south-west from Appin to Wilton and provides a link through Wilton to Picton Road. It provides a link for employee and delivery traffic from the local Wollondilly area (e.g. Bargo, Tahmoor, Thirlmere, Picton, Campbelltown, Camden and Narellan), and Wollongong to the south-east by collecting traffic coming from Picton Road towards the Appin Mine and West Cliff Colliery. Wilton Road does not carry any coal haulage trucks associated with the existing operations or the proposed Project.

Broughtons Pass Weir is located on Wilton Road. Heavy vehicle access at Broughtons Pass Weir crossing is restricted, therefore heavy vehicle traffic only accesses the Appin West pit top from Picton Road to the west.

## Macarthur Road

Macarthur Road is a rural collector road that provides a link between the Picton Road exchange and Wilton Road, bypassing the town of Wilton by allowing traffic travelling north-west on Picton Road to access Appin without travelling through Wilton. It provides a link for employee and delivery traffic from Wollongong to the south-east by collecting traffic coming from Picton Road towards the Appin Mine and West Cliff Colliery. Macarthur Road does not carry any coal haulage trucks associated with the existing operations or the proposed Project.

## Douglas Park Drive

Douglas Park Drive is a rural collector road that provides a link between Douglas Park and Wilton Road between Wilton and Appin. It travels south from Camden Road in Douglas Park over the F5 South Western Freeway/Hume Highway and serves as an access road for the Appin West pit top via Wilton Road or Douglas Park. Douglas Park Drive does not carry any coal haulage trucks associated with the existing operations or the proposed Project.

## 2.5 recent annual average daily traffic volumes

The RTA publishes traffic volume data at selected locations on its roads. Available data on roads used by the Appin Mine and West Cliff Colliery trucks was collated, and is presented below.

The most recent data available from RTA was from surveys undertaken in 2007 (Southern Region). This data is presented in table 2. The traffic volumes reported at each location is the Annual Average Daily Traffic (AADT), which is assessed as the total volume of traffic recorded at the location taken over a calendar year, divided by the number of days in that year. The complete historical data set is provided in attachment a and is discussed further in section 3.2 of this assessment in the context of background traffic growth rates.

The University Avenue/Graham Avenue off-ramp link from the F6 Southern Freeway to the Northern Distributor has not been included in analysis of background and future traffic volumes given its function is as an on/off-ramp between arterial roads. Increases in traffic volumes on University Avenue/Graham Avenue due to the Project are considered very minor (i.e. in the order of an additional eight truck movements per day or less than one movement per hour).

Railway Street has not been included in the analysis of background and future traffic volumes due to a lack of traffic counts and given that increases in traffic volumes on Railway Street due to the Project are also considered very minor (i.e. in the order of an additional eight truck movements per day or less than one movement per hour).

Douglas Park Drive and Macarthur Road have not been included in the analysis of background and future traffic volumes due to a lack of traffic counts and given that the increase in traffic volumes on these roads would occur during peak hour when employee traffic arrives/departs. Intersection operation for peak hour movements along these roads has been assessed in section 2.10. Section 4.2 includes assessment of intersections on Douglas Park Drive and Macarthur Road in consideration of additional Project traffic.
table 2: annual average daily traffic on haulage routes

| Road | Location | Year | AADT (Vehicles per day) |
| :---: | :---: | :---: | :---: |
| Route to/from PKCT |  |  |  |
| Appin Road | Appin, West of Princes Highway | 2007 | 9,030 |
| Mount Ousley Road | Mount Pleasant, South of Clive Bissell Drive | 2006 | 39,881 |
| F6 Southern Freeway (South) | Mount Ousley, South of Mount Ousley Road | 2005 | 42,220 |
|  | West Wollongong at Gipps Road overpass | 2007 | 69,771 |
| Masters Road | Mount St Thomas, West of Springhill Road | 2003 | 25,226 |
| Springhill Road | Mount St Thomas, North of Masters Road | 2005 | 35,179 |
| Route to/from BlueScope Steelworks and Dendrobium Washery |  |  |  |
| Appin Road | Appin, West of Princes Highway | 2007 | 9,030 |
| Mount Ousley Road | Mount Pleasant, South of Clive Bissell Drive | 2006 | 39,881 |
| F6 Southern Freeway (South) | Mount Ousley, South of Mount Ousley Road | 2005 | 42,220 |
|  | West Wollongong at Gipps Road overpass | 2007 | 69,771 |
| Masters Road | Mount St Thomas, West of Springhill Road | 2003 | 25,226 |
| Springhill Road | Mount St Thomas, North of Five Islands | 2005 | 40,524 |
| Route to/from Corrimal Coke Works |  |  |  |
| Appin Road | Appin, West of Princes Highway | 2007 | 9,030 |
| Mount Ousley Road | Mount Pleasant, South of Clive Bissell Drive | 2006 | 39,881 |
| F6 Southern Freeway (South) | Mount Ousley, South of Mount Ousley Road | 2005 | 42,220 |
| Northern Distributor | Towradgi, South of Towradgi Road | 2007 | 31,517 |
| Route to/from Coalcliff Coke Works |  |  |  |
| Appin Road | Appin, West of Princes Highway | 2007 | 9,030 |
| F6 Southern Freeway (North) | Bulli Tops, North of Appin Road | 2006 | 37,941 |
| Lawrence Hargrave Drive | Stanwell Park, West of Bald Hill Road | 2003 | 5,546 |

Source: RTA Traffic Volume Data (2003, 2005, 2006, 2007, 2008).

The results in table 2 demonstrate the variance in traffic volumes on the key routes used by the haulage contractor's trucks. The F6 Southern Freeway carries almost 70,000 vehicles per day south of Mount Ousley Road, while Lawrence Hargrave Drive carries around 5,500 vehicles per day at Stanwell Park. The historical changes to these traffic levels are discussed further in section 3 of this report.

## 2.6 summary of the pkct proposal

The PKCT facility is located in Port Kembla and functions as a coal export node for a number of coal mining operations in the southern and western NSW coalfields. PKCT operates 24 hours per day, 365 days per year and exports approximately 10.8 Mtpa. Currently PKCT's deliveries are limited by a restriction on receiving hours (and therefore tonnage) for public road deliveries. Note that although PKCT receives 10.8 Mtpa currently, much of this coal is delivered from internal roads within the Port Kembla industrial area (e.g. Dendrobium Washery), so the existing amount delivered directly to PKCT by public road is much less.

PKCT is currently seeking approval to remove the restriction and increase public road receivals to 24 hours per day, 7 days per week for a maximum of 10 Mtpa of coal received by public road. As part of the EA lodged with DoP in 2008, PKCT prepared a Traffic Study to determine the effects of increased road deliveries on haulage routes (Cardno Eppell Olsen, 2008). Data from the PKCT Traffic Study has been used in this report and is referenced where incorporated.

## 2.7 traffic surveys

A programme of traffic surveys was conducted along the routes used by the ICHPL haulage trucks for the PKCT Traffic Study (Cardno Eppell Olsen, 2008). Individual traffic surveys were also conducted along the haulage route by RTA in 2007 and Masson Wilson Twiney (2008). These surveys recorded hourly traffic volumes and vehicle classifications by direction during several periods from 2006 to 2008 at the following locations (figure 4 and attachment b):

त Site 1: Bulli-Appin Road at Appin near Kings Fall Bridge (21 to 27 October 2007) (Masson Wilson Twiney, 2008).
$\pi$ Site 2: Appin Road north of Princes Highway (14 to 20 August 2006) (Cardno Eppell Olsen, 2008).
त Site 3: Mount Ousley Road at Mount Pleasant (26 February to 4 March 2007) (Cardno Eppell Olsen, 2008).
त Site 4: F6 Southern Freeway, north of Princes Highway interchange at West Wollongong (23 February to 1 March 2007) (Cardno Eppell Olsen, 2008).
\# Site 5: F6 Southern Freeway, south of Princes Highway near footbridge (23 February to 1 March 2007) (Cardno Eppell Olsen, 2008).
$\pi$ Site 6: Masters Road at Mount St. Thomas (23 February to 1 March 2007) (Cardno Eppell Olsen, 2008).
त Site 7: Springhill Road at Coniston (8 to 21 March 2008) (Cardno Eppell OIsen, 2008).
$\pi$ Site 8: Springhill Road north of Five Islands Road (Cardno Eppell Olsen, 2008) ${ }^{2}$.
$\pi$ Site 9: Northern Distributor, between Railway Street and Bellambi Lane (8 to 21 March 2008) (Cardno Eppell Olsen, 2008).
$\pi$ Site 10: Lawrence Hargrave Drive east of Princes Highway (March 2007) (RTA, 2007).
$\pi$ Site 11: Lawrence Hargrave Drive between Walker St and Stonehaven Rd, Stanwell Tops (21 to 27 October 2007) (Masson Wilson Twiney, 2008).
$\pi \quad$ Site 12: Princes Highway at Bulli Pass (21 to 27 October 2007) (Masson Wilson Twiney, 2008).
„ Site 13: Princes Highway north of Bellambi Lane, Russell Vale (21 to 27 October 2007) (Masson Wilson Twiney, 2008).
\# Site 14: Narellan-Appin Road north of Appin township, south of Brian Road (21 to 27 October 2007) (Masson Wilson Twiney, 2008).

[^2]

RTA survey results also recorded average daily traffic volumes and vehicle classifications by direction during several periods from 2007 to 2008 at the following locations along local roads:
„ Site 15: Remembrance Driveway north of Finns Road (December 2008).
„ $\quad$ Site 16: Menangle Road at Nepean River Bridge (December 2008).
$\pi$ Site 17: Remembrance Driveway 0.5 km north of Regreme Road (December 2008).
„ Site 18: Menangle Road east of Picton-Oakdale Road (April 2008).
„ $\quad$ Site 19: Picton Road at Nepean River Bridge (April 2008).
$\pi \quad$ Site 20: Remembrance Driveway 0.8 km south of Tahmoor Post Office (December 2008).
$\pi \quad$ Site 21: Picton Road west of Mount Keira Road (June 2007).

Preliminary survey results by Transport and Traffic Associates (2008) for the proposed Wilton Quarry on Wilton Road also recorded average daily traffic volumes and heavy vehicles by direction at the following location:
„ Site 22: Wilton Road at Clements Creek.

The traffic surveys at these 22 locations were supplemented by estimates from ICHPL including coal truck movements to and from the Appin Mine and West Cliff Colliery and estimates of existing deliveries and workforce light vehicle movements.

## 2.8 average weekday traffic volumes

The surveyed two-way average weekday traffic volumes at the automatic survey locations are summarised in table 3 below. These are the average two-way daily flows over the Monday to Friday period. The table also summarises the types of vehicles at each location, being light, rigid or articulated vehicles. Light vehicles include motorcycles, cars, vans, 4WDs, and utes, including those towing a trailer or caravan. Heavy vehicles include rigid vehicles (i.e. single unit trucks and buses with two to four axles) and articulated vehicles (i.e. semi-trailers, rigid trucks with trailers and B Doubles).

It should be noted that the articulated vehicles in table 3 represent all articulated vehicles at each location, not just those movements generated to and from the Appin Mine and West Cliff Colliery (i.e. table 3 includes other articulated truck movements associated with deliveries to/from local and regional agricultural, industrial, retail industries, etc.). As described in section 2.5, University Avenue/Graham Avenue, Railway Street, Douglas Park Drive and Macarthur Road are not included in analysis of background and future traffic volumes.
table 3: two-way average weekday daily traffic volumes (veh/day)

| Site | Road and Location | Light Vehicles (\%) |  | Rigid Vehicles (\%) |  | Articulated Vehicles |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Number | Percent | Number | Percent | Number | Percent |  |
| To/from PKCT |  |  |  |  |  |  |  |  |
| 1 | Bulli-Appin Road, Appin near Kings Fall Bridge (2007) ${ }^{1}$ | 7,148 | 87.5\% | 689 | 8.4\% | 337 | 4.1\% | 8,174 |
| 2 | Appin Road north of Princes Highway $(2006)^{2}$ | 7,909 | 82.4\% | 787 | 8.2\% | 897 | 9.4\% | 9,593 |
| 3 | Mount Ousley Road at Mount Pleasant $(2007)^{2}$ | 36,997 | 86.3\% | 2,286 | 5.3\% | 3,593 | 8.4\% | 42,876 |
| 4 | F6 Southern Freeway north of Princes Highway interchange at West Wollongong (2007) ${ }^{2}$ | 67,728 | 89.4\% | 4,325 | 5.7\% | 3,734 | 4.9\% | 75,787 |
| 5 | F6 Southern Freeway south of Princes Highway near footbridge (2007) ${ }^{2}$ | 65,862 | 89.8\% | 3,727 | 5.1\% | 3,761 | 5.1\% | 73,350 |
| 6 | Masters Road at Mount St. Thomas $(2007)^{2}$ | 22,968 | 86.5\% | 1,481 | 5.6\% | 2,090 | 7.9\% | 26,539 |
| 7 | Springhill Road at Coniston (2008) ${ }^{2}$ | 14,915 | 92.7\% | 617 | 3.8\% | 558 | 3.5\% | 16,090 |
| To/from BlueScope Steelworks and Dendrobium Washery |  |  |  |  |  |  |  |  |
| 8 | Springhill Road north of Five Islands Road | Not available (4.7\% Heavy Vehicles indicatively) |  |  |  |  |  | 1,480 ${ }^{\text {² }}$ |
| Route to/from Corrimal Coke Works |  |  |  |  |  |  |  |  |
| 9 | Northern Distributor between Railway Street and Bellambi Lane (2008) ${ }^{2}$ | 21,063 | 92.2\% | 1,471 | 6.4\% | 307 | 1.3\% | 22,841 |
| Route to/from Coalcliff Coke Works |  |  |  |  |  |  |  |  |
| - | F6 Southern Freeway (North) | (Not available) |  |  |  |  |  | 37,941* |
| 10 | Lawrence Hargrave Drive east of Princes Highway (2007) ${ }^{3}$ | 3,846 | 95.5\% | 124 | 3.1\% | 58 | 1.4\% | 4,028 |
| 11 | Lawrence Hargrave Drive at Stanwell Tops (2007) ${ }^{1}$ | 5,494 | 93.6\% | 300 | 5.1\% | 77 | 1.3\% | 5,871 |

table 3: two-way average weekday daily traffic volumes (veh/day) (continued)

| Site | Road and Location | Light Vehicles (\%) |  | Rigid Vehicles (\%) |  | Articulated Vehicles |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Number | Percent | Number | Percent | Number | Percent |  |
| Other Local Roads |  |  |  |  |  |  |  |  |
| 12 | Princes Highway at Bulli Pass (2007) ${ }^{1}$ | 9,948 | 94.6\% | 505 | 4.8\% | 63 | 0.6\% | 10,516 |
| 13 | Princes Highway north of Bellambi Lane, Russell Vale (2007) ${ }^{1}$ | 24,061 | 93.3\% | 1,538 | 6.0\% | 181 | 0.7\% | 25,780 |
| 14 | Narellan-Appin Road north of Appin township, south of Brian Road (2007) ${ }^{1}$ | 9,015 | 90.9\% | 625 | 6.3\% | 276 | 2.8\% | 9,916 |
| 15 | Remembrance Driveway north of Finns Road (2008) ${ }^{3}$ | 10,292 | 94.7\% | 457 | 4.2\% | 117 | 1.1\% | 10,866 |
| 16 | Menangle Road at Nepean River Bridge $(2008)^{3}$ | 6,185 | 96.8\% | 171 | 2.7\% | 35 | 0.5\% | 6,391 |
| 17 | Remembrance Driveway 0.5 km north of Regreme Road (2008) ${ }^{3}$ ^ | 3,462 | 94.8\% | 155 | 4.2\% | 35 | 1.0\% | 3,652 |
| 18 | Menangle Road east of Picton-Oakdale Road (2008) ${ }^{3}$ | 2,671 | 94.0\% | 135 | 4.8\% | 36 | 1.3\% | 2,842 |
| 19 | Picton Road at Nepean River Bridge (2008) ${ }^{3}$ | 7,170 | 89.6\% | 509 | 6.4\% | 322 | 4.0\% | 8,001 |
| 20 | Remembrance Driveway 0.8 km south of Tahmoor Post Office (2008) ${ }^{3}$ | 7,113 | 93.9\% | 374 | 4.9\% | 86 | 1.1\% | 7,573 |
| 21 | Picton Road west of Mount Keira Road $(2007)^{3}$ | 9,105 | 85.5\% | 314 | 2.9\% | 1,233 | 11.6\% | 10,652 |
| 22 | Wilton Road at Clements Creek (2008) ${ }^{4}$ | 1,902 | 89.6\% | 223 Heavy Vehicles (10.4 \%) |  |  |  | 2,125 |

* AADT.
$\wedge$ Southbound only.
${ }^{\top}$ Average of two-way mid-block peak hour counts.
${ }^{1}$ Masson Wilson Twiney (2008). ${ }^{2}$ Cardno Eppell Olsen (2008).
${ }^{3}$ RTA class surveys. Note this is average daily data. ${ }^{4}$ Transport and Traffic Associates (2008).


## 2.9 existing bulli seam operations traffic generation

### 2.9.1 appin/west cliff coal haulage

ICHPL provided data regarding coal haulage to PKCT, BlueScope Steelworks, Dendrobium Washery, Coalcliff and Corrimal for 2007, corresponding to the highest recent year of haulage from the Appin Mine and West Cliff Colliery. Number of movements has been calculated assuming an average load per truck of 36.5 tonnes ( t , which was calculated from historical data and used in the PKCT Traffic Study (Cardno Eppell Olsen, 2008). Truck haulage movements are summarised in table 4 below (two movements equates to one round trip).
table 4: coal hauling movements from west cliff/appin in 2007

| Truck movements | To PKCT | To BlueScope Steelworks | To Dendrobium Washery** | To Coalcliff Coke Works | To Corrimal Coke Works | To West Cliff pit top** | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Total in 2007 | 183,974 | 103,202 | 21,564 | 2,894 | 2,894 | 151,196 | 465,724 |
| Weekday Average* | 566 | 318 | 66 | 9 | 9 | 465 | 1,433 |

[^3]From past haulage surveys in 2007, $80 \%$ of truck trips occurred during weekdays, with $20 \%$ occurring on weekends (Cardno Eppell Olsen, 2008). This demonstrates that weekend traffic generation by the existing operations is significantly lower than that on weekdays.

### 2.9.2 appin/west cliff deliveries/visitor trips

ICHPL provided estimates of courier and delivery vehicle movements associated with the existing operations. This shows that at West Cliff pit top, a total of 577 delivery movements are made per week, or an average of 91 delivery movements per weekday (i.e. 7 heavy vehicle movements and 84 light vehicles), with the majority being couriers/visitors and deliveries to the site for parts and equipment. At Appin West pit top, a total of 519 delivery movements are made per week, or an average of 81 delivery movements per weekday (i.e. 23 heavy vehicle movements and 58 light vehicles). At Appin East pit top, a total of 130 delivery movements are made per week, or an average of 21 delivery movements per weekday (i.e. 1 heavy vehicle movement and 20 light vehicles). The majority of visitor/sale representatives/couriers/other deliveries occur on weekdays (i.e. 80 percent on weekdays), whereas heavy vehicle deliveries occur throughout the week. The type of vehicles used for these deliveries varied from utes and vans through to semi-trailers, as shown in table 5 below.

Table 5 also incorporates approximately 20 trucking movements per weekday associated with the removal of brine from the Appin West Water Treatment Plant to Unanderra for disposal. Sludge removal from the Appin West Treatment Plant Dynasand filter also results in minor trucking movements (i.e. approximately two movements per fortnight) to Badgery Creek, Sydney and is considered in table 5.
table 5: courier and delivery movements

| Type of Vehicle | West Cliff pit top |  | Appin West pit top |  | Appin East pit top |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Movements | Percent of Total | Movements | Percent of Total | Movements | Percent of Total |
| Truck (machine parts/brine and sludge removal/other deliveries) | 22 /week | 4\% | 152 /week | 29\% | $3 /$ week | 2\% |
| Truck (fuel) | 24 /week | 4\% | 2 /week | 1\% | $1 /$ week | 1\% |
| Truck (oversize loads) | 6 /week | 1\% | 2 /week | 1\% | $1 /$ week | 1\% |
| Small truck/light vehicles (visitors/sale representatives) | 32 /weekday | 35\% | 16 /weekday | 19\% | 4 /weekday | 19\% |
| Small truck/vans (couriers/other deliveries) | 52 /weekday | 56\% | 42 /weekday | 50\% | 16 /weekday | 77\% |
| Total | 577 /week | 100 | 519 /week | 100 | 130 /week | 100 |

Source: ICHPL data (2009).

ICHPL also provided estimates for the sources of key deliveries. Wollongong was the most common origin for delivery movements, followed by Mittagong, Newcastle/Sydney and the local Wollondilly area, as shown in table 6, below.
table 6: expected delivery routes

| Likely Route | Percent of <br> movements to/from <br> West Cliff pit top | Percent of <br> movements to/from <br> Appin West pit top | Percent of <br> movements to/from <br> Appin East pit top |
| :--- | :---: | :---: | :---: |
| Princes Highway (Bulli <br> Pass) | $40 \%$ | $25 \%$ | $40 \%$ |
| Mount Ousley Road | $40 \%$ | $25 \%$ | $40 \%$ |
| F6 Southern Freeway <br> (north) | $5 \%$ | $10 \%$ | $5 \%$ |
| Appin Road north of Appin | $10 \%$ | $5 \%$ | $30 \%$ |

Source: ICHPL data (2009).

Table 6 assumes that from Wollongong, one half of movements would use Bulli Pass, and the remainder would use Mount Ousley Road (i.e. travelling from south of Fairy Meadow). For deliveries coming from the local Wollondilly area, the main access is assumed to be split between Appin Road north of Appin and Picton Road/Wilton Road. Road deliveries from the Sydney/Newcastle area are assumed to be split between the inland highways (i.e. Hume Highway/Appin Road) and the coastal F6 Southern Freeway.

### 2.9.3 appin/west cliff employee traffic

Total movements generated by employees, on-site contractors, etc. were observed from ICHPL data to comprise approximately 520 movements per day to/from West Cliff pit top and approximately 800 movements per day to/from Appin East/Appin West pit tops. Approximately $80 \%$ of the Appin Mine traffic (employee and deliveries) services Appin West pit top, with the remainder going to Appin East pit top. Therefore approximately 160 movements are generated per day to/from Appin East pit top and approximately 640 movements are generated to/from Appin West pit top. The above employee movements were compared to the total number of employees at each pit top to determine the car pooling factor for respective pit tops. The distribution of employees between pit tops for the existing operations and the Project is described in Section 2 in the Main Report of the EA. A car pooling factor of 1.5 people per car at West Cliff pit top and 1.2 people per car at Appin East and Appin West pit tops was determined from observed ICHPL data.

Details of the approximate distribution of these employee/contractor movements were provided by ICHPL. Employee distribution to/from West Cliff/Appin East pit tops (i.e. access via Appin Road) and Appin West pit top (access via Douglas Park Drive) were very similar (+/-5\% difference in employee residence for each location below):
$\pi \quad 50$ percent to/from the south (i.e. Wollongong south of Fairy Meadow using F6 Southern Freeway and Appin Road).
$\pi \quad 30$ percent to/from the south (north of Fairy Meadow to Scarborough using Princes Highway up Bulli Pass and Appin Road).
„ 15 percent from local areas (i.e. Camden, Campbelltown, Douglas Park, Mittagong, Picton and Wilton), using Picton Road, Hume Highway, Remembrance Driveway, Menangle Road, Appin Road north of Appin and Wilton Road.
л 5 percent from the north (i.e. Sydney), using the Hume Highway and F6 Southern Freeway.

### 2.9.4 coal wash haulage

During the life of the Project, Dendrobium Washery would produce in the order of up to 27.2 Mt of coal wash (Cardno Forbes Rigby, 2008) which would require emplacement at the West Cliff Coal Wash Emplacement. Trucks delivering coal to PKCT/BlueScope Steelworks would continue to backhaul coal wash to the West Cliff Coal Wash Emplacement and as such, no additional truck trips would be required as a result of coal wash haulage from Dendrobium Washery.

### 2.9.5 contribution of appin/west cliff to traffic on haulage routes and local roads

Based on the data provided by ICHPL, the typical contribution of the Appin Mine and West Cliff Colliery traffic to the total traffic on the surveyed routes was estimated. This includes courier and delivery vehicles, coal haulage trucks and employee/contractor vehicles.

The assessment presented in table 7 assumes that the destinations of the employee, courier and delivery vehicle movements when they leave the pit tops are the same as their origins as estimated by ICHPL. Heavy vehicles include rigid and articulated vehicles as described in section 2.8.

Due to access for heavy vehicles travelling to Appin West pit top being restricted by Broughtons Pass Weir, it is assumed that all heavy vehicles travelling to Appin West pit top from Wollongong gain access via Picton Road west of Mount Keira Road, approaching Appin West pit top from south-east of Wilton. Heavy vehicles from the north (e.g. Campbelltown) would use the F5 South Western Freeway/Hume Highway and Picton Road. Due to travel distances, light vehicles from Wollongong (south of Fairy Meadow) are also assumed to gain access to the Appin West pit top via Picton Road west of Mount Keira Road.
table 7 estimated recent average weekday appin/west cliff traffic movements on haulage routes and local roads

| Site | Road and Location | Courier/ Deliveries |  | Coal Haulage <br> Heavy | Employee/ Contractor <br> Light | Total ICHPL Vehicle Movements |  | $\begin{gathered} \text { All } \\ \text { Vehicles* } \end{gathered}$ | Appin/ West Cliff (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Light | Heavy |  |  | Light | Heavy |  |  |
| To/from PKCT |  |  |  |  |  |  |  |  |  |
| 1 | Bulli-Appin Road, Appin near Kings Fall Bridge | 50 | 2 | 465 | 431 | 481 | 467 | 8,174 | 11.6\% |
| 2 | Appin Road north of Princes Highway | 109 | 8 | 968 | 769 | 878 | 976 | 9,593 | 19.3\% |
| 3 | Mount Ousley Road at Mount Pleasant | 42 | 4 | 959 | 340 | 382 | 963 | 42,876 | 3.1\% |
| 4 | F6 Southern Freeway north of Princes Highway interchange at West Wollongong | 42 | 4 | 950 | 340 | 382 | 954 | 75,787 | 1.8\% |
| 5 | F6 Southern Freeway south of Princes Highway near footbridge | 42 | 4 | 950 | 340 | 382 | 954 | 73,350 | 1.8\% |
| 6 | Masters Road at Mount St. Thomas | 0 | 0 | 950 | 0 | 0 | 950 | 26,539 | 3.6\% |
| 7 | Springhill Road at Coniston | 0 | 0 | 566 | 0 | 0 | 566 | 16,090 | 3.5\% |
| To/from BlueScope SteeIWorks and Dendrobium Washery |  |  |  |  |  |  |  |  |  |
| 8 | Springhill Road north of Five Islands Road** | 0 | 0 | 384 | 0 | 0 | 384 | 40,524 | 0.9\% |
| Route to/from Corrimal Coke Works |  |  |  |  |  |  |  |  |  |
| - | Northern Distributor, Towradgi, south of Towradgi Road^ | 0 | 0 | 9 | 0 | 0 | 9 | 31,517 | 0.0\% |
| Route to/from Coalcliff Coke Works |  |  |  |  |  |  |  |  |  |
| - | F6 Southern Freeway (North) | 11 | 3 | 9 | 33 | 44 | 12 | 37,941 | 0.1\% |
| 10^^ | Lawrence Hargrave Drive east of Princes Highway | 0 | 0 | 9 | 0 | 0 | 9 | 4,028 | 0.2\% |

table 7 estimated recent average weekday appin/west cliff traffic movements on haulage routes and local roads (continued)

| Site | Road and Location | Courier/ <br> Deliveries |  | Coal <br> Haulage | Employee/ <br> Contractor | Total ICHPL <br> Vehicle <br> Movements | All <br> Vehicles | Appin/ <br> West <br> Cliff <br> (\%) |  |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Light | Heavy | Heavy | Light | Light | Heavy |  |  |  |
| Local Roads |  |  |  |  |  |  |  |  |  |
| 12 | Princes Highway at Bulli <br> Pass | 56 | 4 | 0 | 396 | 452 | 4 | 10,516 | $4.3 \%$ |
| 13 | Princes Highway north of <br> Bellambi Lane, Russell Vale | 56 | 4 | 0 | 396 | 452 | 4 | 25,780 | $1.8 \%$ |
| 14 | Narellan-Appin Road north <br> of Appin township, south of <br> Brian Road | 16 | 1 | 0 | 132 | 148 | 1 | 9,916 | $1.5 \%$ |
| 15 | Remembrance Driveway <br> north of Finns Road | 8 | 2 | 0 | 33 | 41 | 2 | 10,866 | $0.4 \%$ |
| 16 | Menangle Road at Nepean <br> River Bridge | 8 | 2 | 0 | 33 | 41 | 2 | 6,391 | $0.7 \%$ |
| 17 | Remembrance Driveway <br> 0.5 km north of Regreme <br> Road | 8 | 2 | 0 | 33 | 41 | 2 | 3,652 | $1.2 \%$ |
| 18 | Menangle Road east of <br> Picton-Oakdale Road | 8 | 2 | 0 | 33 | 41 | 2 | 2,842 | $1.5 \%$ |
| 19 | Picton Road at Nepean <br> River Bridge | 23 | 7 | 0 | 99 | 122 | 7 | 8,001 | $1.6 \%$ |
| 20 | Remembrance Driveway <br> 0.8 km south of Tahmoor <br> Post Office | 8 | 2 | 0 | 33 | 41 | 2 | 7,573 | $0.6 \%$ |
| 21 | Picton Road west of Mount <br> Keira Road | 14.5 | 14 | 0 | 320 | 335 | 14 | 10,652 | $3.3 \%$ |
| 22 | Wilton Road at Clements <br> Creek | 23 | 7 | 0 | 99 | 122 | 7 | 2,125 | $6.1 \%$ |

* Two-way survey data unless specified - sources presented in section 2.7.
** Only two-way peak hour data is available at Site 8. Therefore, a nearby indicative AADT count has been used for 'All Vehicles'.
^ Average Annual Daily Traffic (AADT).
${ }^{\wedge \wedge}$ Site 11 was removed from further assessment as Lawrence Hargrave Drive is covered under Site 10.


### 2.10 existing peak hour traffic conditions

A number of peak hour turning count surveys were undertaken on Thursday 9 ${ }^{\text {th }}$ April 2009 in order to determine the performance of critical intersections that are likely to be impacted upon by the Project. The results of the peak hour surveys are presented on figure 5. The results of the turning count surveys are provided in attachment $\mathbf{c}$.

The results of these surveys were analysed using the SIDRA computer program to determine their performance characteristics under existing traffic conditions. The SIDRA model produces a range of outputs, including Degree of Saturation (DOS) and Average Vehicle Delay per vehicle (AVD). The AVD is in turn related to a Level of Service (LOS) criteria. These performance measures can be interpreted using the following explanations:

Degree of Saturation (DOS) - the DOS is a measure of the operational performance of individual intersections. As both queue length and delay increase rapidly as DS approaches 1, it is usual to attempt to keep DS to less than 0.9. When DS exceeds 0.9 residual queues can be anticipated, as occurs at many major intersections throughout the metropolitan area during peak periods. In this regard, a practical limit at 1.1 can be assumed. For intersections controlled by roundabout or give way/stop control, satisfactory intersection operation is generally indicated by a DOS of 0.8 or less.

Average Vehicle Delay (AVD) (seconds/vehicle) - the AVD for individual intersections provides a measure of the operational performance of an intersection. In general, levels of acceptability of AVD for individual intersections depend on the time of day (motorists generally accept higher delays during peak commuter periods) and the road system being modelled (motorists are more likely to accept longer delays on side streets than on the main road system).


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10km road transport assessment: bulli seam operations project


Level of Service (LOS) - this is a comparative measure which provides an indication of the operating performance of an intersection as shown below:

| Level of Service <br> (LOS) | Average Vehicle <br> Delay (AVD) (s/veh) | Traffic Signals, <br> Roundabout | Give Way and Stop <br> Signs |
| :---: | :---: | :---: | :---: |
| A | less than 14 | Good operation. | Good operation. |
| B | 15 to 28 | Good with acceptable <br> delays and spare <br> capacity. | Acceptable delays and <br> spare capacity. |
| C 29 to 42 | Satisfactory. | Satisfactory but <br> accident study <br> required. |  |
| D | 43 to 56 | Operating near capacity. | Near capacity and <br> accident study <br> required. |
| E | 57 to 70 | At capacity; at signals <br> incidents will cause <br> excessive delays. <br> Roundabouts require <br> other control mode. | At capacity and <br> requires other control <br> mode. |
| F | More than 70 | Unsatisfactory and <br> requires additional <br> capacity. | Unsatisfactory and <br> requires other control <br> mode or major <br> treatment. |

Source: Adapted from RTA Guide to Traffic Generating Developments, 2002.

A summary of the modelled results are provided below in table 8. The SIDRA outputs are provided in attachment $\mathbf{d}$ which includes detailed results for individual lanes and approaches.
table 8: existing intersection performance: am and pm peak hour

| Site | Intersection Description | Control Type | Period | DOS | AVD (s/veh) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| HAULAGE ROUTES |  |  |  |  |  |  |
| A | Appin Road/George Street (Appin East pit top exit) | signals | AM | 0.557 | 16.3 | B |
|  |  |  | PM | 0.684 | 19.5 | B |
| B | Appin Road/Sheriff Road (Appin East pit top entry) | priority | AM | 0.180 | 14.5 | B |
|  |  |  | PM | 0.157 | 14.8 | B |
| C | Appin Road/West Cliff pit top access | priority | AM | 0.339 | 32.3 | C |
|  |  |  | PM | 0.336 | 24.9 | B |
| D | Appin Road/Princes Highway off-ramp | priority | AM | 0.312 | 18.5 | B |
|  |  |  | PM | 0.278 | 22.3 | B |
| E | Mount Ousley Road/F6 Southern Freeway | priority | AM | 1.776 | 467.0 | F |
|  |  |  | PM | 2.333 | 544.0 | F |
| F | Springhill Road/Masters Road ${ }^{1}$ | signals | AM | 0.844 | 30.1 | C |
|  |  |  | PM | 0.880 | 39.6 | C |
| G | Springhill <br> Road/Recycling Road | signals | AM | 0.908 | 28.4 | B |
|  |  |  | PM | 0.857 | 23.7 | B |
| H | Springhill Road/Port Kembla Road ${ }^{1}$ | signals | AM | 0.267 | 7.2 | A |
|  |  |  | PM | 0.447 | 10.5 | A |
| I | Railway Street/Corrimal Coke Works gate ${ }^{2}$ | priority | AM | 0.17 | 8.3 | A |
|  |  |  | PM | - | - | - |
| J | Princes Highway/Lawrence Hargrave Drive ${ }^{2}$ | priority | AM | 0.23 | 11.6 | A |
|  |  |  | PM | 0.14 | 15.5 | B |
| LOCAL ROADS |  |  |  |  |  |  |
| K | Picton Road/Almond | priority | AM | 0.400 | 26.6 | B |
|  | Street |  | PM | 0.500 | 31.0 | C |
| L | Wilton Road/Macarthur Road | priority | AM | 0.068 | 16.3 | B |
|  |  |  | PM | 0.075 | 15.6 | B |
| M | Wilton Rd/Douglas ParkDrive | priority | AM | 0.168 | 14.7 | B |
|  |  |  | PM | 0.170 | 15.6 | B |
| N | Appin Road/Wilton Road | priority | AM | 0.338 | 21.9 | B |
|  |  |  | PM | 0.446 | 15.8 | B |

[^4]It can be seen from table 8 that the majority of the above intersections have an overall LOS of A, B or C and operate satisfactorily under the existing 'base case' scenario, with moderate delays. The notable exception is the intersection of Mount Ousley Road with the F6 Southern Freeway, which operates at LOS F due to delays to traffic turning right out of Mount Ousley Road (figure 5). It should be noted that the existing operations contribute a low $3.1 \%$ of surveyed traffic movements along Mount Ousley Road, all of which is through traffic to the F6 Southern Freeway. No haulage trucks use the right turn across Mount Ousley Road. ICHPL have indicated that a negligible proportion of employee traffic use the right turn across Mount Ousley Road because it has a poor performance and therefore significant delays can result. It is recommended that the RTA make improvements to this intersection now as a matter of priority due to the existing low LOS.

Nevertheless, it should be noted that the most relevant use of this analysis is to compare the relative change in the performance parameters as a result of the proposed Project.

Previous intersection analysis (Masson Wilson Twiney, 2008) has also indicated that intersections at Railway Street/Corrimal Coke Works entry (i.e. Corrimal Coke Works haul route) and Princes Highway/Lawrence Hargrave Drive (i.e. Coalcliff Coke Works haul route) operate at a good LOS (table 8).

The detailed SIDRA outputs are provided in attachment d1.

## 3. future traffic conditions

## 3.1 planned changes to the surrounding road network

A number of minor road works are currently proposed within the road network used by the Appin Mine and West Cliff Colliery. These generally include maintenance programs and other minor works. Road safety measures including shoulder widening to Picton Road are also under review and implementation by the RTA.

The RTA's Northern Distributor extension to Molly Street is expected to be completed in 2009. This will result in a decrease in traffic from the Princes Highway, in the vicinity of Bellambi Road, as traffic redistributes onto the Northern Distributor. As this will primarily affect flows between Bellambi Road and Molloy Street (i.e. to the north of the Corrimal Coke Works haulage route) it is not expected to significantly impact on Project haulage routes.

Similarly, the RTA's Oak Flats to Dunmore upgrade of the Princes Highway to a four-lane divided carriageway (due for completion in late 2009) lies outside of the primary haul routes and will have minimal impact on the Project with the exception of potentially reduced travel times for a limited number of employees and deliveries from the south.

For safety reasons, the intersection between the Princes Highway and Lawrence Hargrave Drive in Bulli is to be reconstructed by the RTA with a single northbound lane from George Avenue to the intersection. It is scheduled for completion in 2011. The works include the removal of the right turn from the Princes Highway to Lawrence Hargrave Drive in Bulli in addition to restricting a number of surrounding intersections to left-in/left-out arrangements.

None of the abovementioned planned works by the RTA are expected to significantly affect the proposed Project. Section 4.3 of this report discusses other additional road changes as a result of the Project and future traffic growth.

## 3.2 traffic growth

Traffic volumes are expected to continue to increase on the haulage routes, regardless of the operations at the Appin Mine and West Cliff Colliery. This background traffic growth is associated with population growth in the local/regional area and growth in surrounding industry (e.g. agricultural, manufacturing, building/construction, etc.). The average daily volumes have varied considerably over time from route to route. It is expected that growth rates on the different routes would continue to vary in the future, due to the varying roles and linkages provided by each of the roads.

The Sydney-Wollongong Corridor Strategy (SWCS) provides an estimate of likely background traffic growth for Mount Ousley Road and the F6 Southern Freeway, which was also used in the PKCT Traffic Study (Cardno Eppell Olsen, 2008). The SWCS stated that total traffic volume is projected to grow by two percent a year until 2025, with heavy vehicle traffic growing at a slightly faster rate of 2.7 percent per annum.

Background growth data for the remainder of the haulage routes was compiled from RTA AADT data, the PKCT Traffic Study (Cardno Eppell Olsen, 2008) and the Metropolitan Colliery Traffic Assessment (Masson Wilson Twiney, 2008). A summary of the assumed traffic growth rates for background traffic levels are provided in table 9.
table 9: assumed background traffic growth rates

| Site | Road and Location | Source | Percent per annum* |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | Light Vehicles | Heavy Vehicles |
| To/from PKCT |  |  |  |  |
| 1 | Bulli-Appin Road, Appin near Kings Fall Bridge | Metropolitan Colliery <br> Traffic Assessment | 0.95 | 0.95 |
| 2 | Appin Road north of Princes Highway | PKCT Traffic Study ${ }^{2}$ | 2.1 | 2.8 |
| 3 | Mount Ousley Road at Mount Pleasant | AADT (2000-2005) | 4.0 | 4.0 |
| 4 | F6 Southern Freeway north of Princes Highway interchange at West Wollongong | SWCS ${ }^{3}$ | 2.0 | 2.7 |
| 5 | F6 Southern Freeway south of Princes Highway near footbridge |  | 2.0 | 2.7 |
| 6 | Masters Road at Mount St. Thomas | AADT (2000-2003) | 0 | 0 |
| 7 | Springhill Road at Coniston | AADT (2000-2005) | 0.5 | 0.5 |
| To/from BlueScope SteelWorks and Dendrobium Washery |  |  |  |  |
| 8 | Springhill Road north of Five Islands Road | AADT (2000-2005) | 0.9 | 0.9 |
| Route to/from Corrimal Coke Works |  |  |  |  |
| - | Northern Distributor, Towradgi, south of Towradgi Road | AADT (2000-2007) | 1.5 | 1.5 |
| Route to/from Coalcliff Coke Works |  |  |  |  |
| - | F6 Southern Freeway (North) | AADT (2000-2006) | 4.0 | 4.0 |
| 10^ | Lawrence Hargrave Drive east of Princes Highway | Metropolitan Colliery <br> Traffic Assessment ${ }^{1}$ | 5.0 | 5.0 |

table 9: assumed background traffic growth rates (continued)

| Site | Road and Location | Source | Percent per annum ${ }^{*}$ |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | Light Vehicles | Heavy Vehicles |
| Local Roads |  |  |  |  |
| 12 | Princes Highway at Bulli Pass | Metropolitan Colliery <br> Traffic Assessment |  |  |
| 13 | Princes Highway north of Bellambi <br> Lane, Russell Vale | Metropolitan Colliery <br> Traffic Assessment | 2.7 | 1.0 |
| 14 | Narellan-Appin Road north of <br> Appin township, south of Brian <br> Road | AADT(2000 - 2007) | 0.4 | 1.0 |
| 15 | Remembrance Driveway north of <br> Finns Road | AADT(2000 - 2006) | 3.0 | 0.4 |
| 16 | Menangle Road at Nepean River <br> Bridge | AADT(1999 - 2005) | 3.8 | 3.0 |
| 17 | Remembrance Driveway 0.5 km <br> north of Regreme Road | AADT(2000 - 2006) | 0 | 0 |
| 18 | Menangle Road east of Picton- <br> Oakdale Road | AADT(2000 - 2003) | 5.2 | 5.2 |
| 19 | Picton Road at Nepean River <br> Bridge | AADT(2000 - 2003) | 3.5 | 3.5 |
| 20 | Remembrance Driveway 0.8 km <br> south of Tahmoor Post Office | AADT(2000 - 2003) | 0 | 0 |
| 21 | Picton Road west of Mount Keira <br> Road | AADT(2000 - 2003) | 2.6 | 2.6 |
| 22 | Wilton Road at Clements Creek | AADT(2000 - 2003) | 0 | 0 |

* AADT data can show a decline in traffic. Where applicable, a 0\% growth rate was assumed.
${ }^{\wedge}$ Site 11 was removed from further assessment as Lawrence Hargrave Drive is covered under Site 10.
1 Masson Wilson Twiney (2008).
2 Cardno Eppell Olsen (2008).
3 Department of Transport and Regional Services (2007).

The time periods of particular interest to the Project with regard to traffic conditions are:

- Year 1 (i.e. approximately 2011), existing operations plus construction traffic;
- Year 3 (i.e. approximately 2013), being the indicative worst-case operational period which could include a shift of significant components of the workforce from West Cliff pit top to the Appin West pit top, combining maximum Project operational traffic, longwall machine relocation/upgrade traffic and growth in background traffic; and
- Year 10 (i.e. approximately 2019), combining Project operational traffic and growth in background traffic.

Given the high likelihood of changes to the road network beyond the next 10 years, it is not considered appropriate to forecast traffic conditions more than 10 years in advance. As the existing traffic contributions would remain static and background traffic would continue to rise, the relative contribution of the Appin Mine and West Cliff Colliery traffic would continue to fall over time. The operation of the road network is more appropriately assessed based on the performance of key intersections in the locality, as discussed in section 3.3.

Although a period of increased activity, only an additional 100 employees and minor movements associated with construction deliveries would be required for the construction phase of the Project. Therefore the construction period creates a lesser traffic impact when compared to Year 3 (i.e. 2013) when approximately 295 additional employees and an associated increase in deliveries of approximately $100 \%$ would be required (i.e. 975 employees including construction workforce against 1,170 employees at Project peak - Year 3). Also, coal haulage during the initial construction phase is much lower than during the later years when production increases. As such, it is considered that construction traffic impacts (including construction deliveries) would not result in impacts greater than those associated with Project Year 3 (2013) or Year 10 (2019) and therefore a construction scenario is not specifically assessed.

It should be noted that depending on the mine development sequence over the life of the Project (as described in Section 2 in the Main Report of the EA), there may be a period toward the end of the Project life (i.e. post 2030) when the Project underground workforce and deliveries are centred around the West Cliff pit top. As this scenario is not expected to occur within the next 10 years, a specific assessment has not been undertaken at this time given the variability in predicted future traffic growth. Consideration should be given to conducting a specific traffic assessment, should this scenario occur later in the Project life. Notwithstanding, improvements are recommended at the Appin Road and West Cliff pit top access road (section 4.4.1).

The future daily traffic volumes on key roads assuming that the Appin Mine and West Cliff Colliery would continue to operate as they do at present (i.e. no change in traffic generation) are shown in table 10 below.
table 10: existing and future average weekday traffic with existing operations (veh/day)

| Site | Road and Location | Average Weekday Two Way Traffic |  |  | Appin/West Cliff | Appin/West Cliff <br> Percent of Total |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Existing | 2013 | 2019 | ICHPL Vehicle movements | Existing | 2013 | 2019 |
| To/from PKCT |  |  |  |  |  |  |  |  |
| 1 | Bulli-Appin Road, Appin near Kings Fall Bridge | 8,330 | 8,651 | 9,156 | 948 | 11.4\% | 11.0\% | 10.4\% |
| 2 | Appin Road north of Princes Highway | 10,247 | 11,191 | 12,771 | 1,853 | 18.1\% | 16.6\% | 14.5\% |
| 3 | Mount Ousley Road at Mount Pleasant | 46,348 | 54,158 | 68,409 | 1,344 | 2.9\% | 2.5\% | 2.0\% |
| 4 | F6 Southern Freeway north of Princes Highway interchange at West | 78,964 | 85,727 | 96,979 | 1,335 | 1.7\% | 1.6\% | 1.4\% |
| 5 | F6 Southern Freeway south of Princes Highway near footbridge | 76,421 | 82,957 | 93,829 | 1,335 | 1.7\% | 1.6\% | 1.4\% |
| 6 | Masters Road at Mount St. Thomas | 26,539 | 26,539 | 26,539 | 950 | 3.6\% | 3.6\% | 3.6\% |
| 7 | Springhill Road at Coniston | 16,172 | 16,504 | 17,014 | 566 | 3.5\% | 3.4\% | 3.3\% |
| To/from BlueScope SteelWorks and Dendrobium Washery |  |  |  |  |  |  |  |  |
| 8 | Springhill Road north of Five Islands Road* | 42,025 | 43,581 | 46,025 | 384 | 0.9\% | 0.9\% | 0.8\% |
| Route to/from Corrimal Coke Works |  |  |  |  |  |  |  |  |
| - | Northern Distributor, Towradgi, south of Towradgi Road | 32,452 | 34,405 | 37,557 | 9 | 0.0\% | 0.0\% | 0.0\% |

table 10: existing and future average weekday traffic with existing operations (veh/day) (continued)

| Site | Road and Location | Average Weekday Two Way Traffic |  |  | Appin/West Cliff | Appin/West Cliff Percent of Total |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Existing | 2013 | 2019 | ICHPL Vehicle movements | Existing | 2013 | 2019 |
| Route to/from Coalcliff Coke Works |  |  |  |  |  |  |  |  |
| - | F6 Southern Freeway (North) | 42,709 | 50,012 | 63,373 | 56 | 0.1\% | 0.1\% | 0.1\% |
| 10 | Lawrence Hargrave Drive east of Princes Highway^ | 4,443 | 5,404 | 7,250 | 9 | 0.2\% | 0.2\% | 0.1\% |
| Local Roads |  |  |  |  |  |  |  |  |
| 12 | Princes Highway at Bulli Pass | 11,096 | 12,353 | 14,511 | 456 | 4.1\% | 3.7\% | 3.1\% |
| 13 | Princes Highway north of Bellambi Lane, Russell Vale | 26,288 | 27,333 | 28,981 | 456 | 1.7\% | 1.7\% | 1.6\% |
| 14 | Narellan-Appin Road north of Appin township, south of Brian Road | 10,003 | 10,181 | 10,453 | 150 | 1.5\% | 1.5\% | 1.4\% |
| 15 | Remembrance Driveway north of Finns Road | 11,191 | 12,590 | 15,025 | 43 | 0.4\% | 0.3\% | 0.3\% |
| 16 | Menangle Road at Nepean River Bridge | 6,631 | 7,684 | 9,586 | 43 | 0.6\% | 0.6\% | 0.4\% |
| 17 | Remembrance Driveway 0.5 km north of Regreme Road | 3,652 | 3,652 | 3,652 | 43 | 1.2\% | 1.2\% | 1.2\% |
| 18 | Menangle Road east of Picton-Oakdale Road | 2,991 | 3,667 | 4,979 | 43 | 1.4\% | 1.2\% | 0.9\% |
| 19 | Picton Road at Nepean River Bridge | 8,282 | 9,507 | 11,692 | 129 | 1.6\% | 1.4\% | 1.1\% |

table 10: existing and future average weekday traffic with existing operations (veh/day) (continued)

| Site | Road and Location | Average Weekday Two Way Traffic |  |  | Appin/West Cliff | Appin/West Cliff Percent of Total |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Existing | 2013 | 2019 | ICHPL Vehicle movements | Existing | 2013 | 2019 |
| 20 | Remembrance Driveway 0.8 km south of Tahmoor Post Office | 7,573 | 7,573 | 7,573 | 43 | 0.6\% | 0.6\% | 0.6\% |
| 21 | Picton Road west of Mount Keira Road | 11,213 | 12,426 | 14,494 | 348 | 3.1\% | 2.8\% | 2.4\% |
| 22 | Wilton Road at Clements Creek | 2,125 | 2,125 | 2,125 | 129 | 6.1\% | 6.1\% | 6.1\% |

* AADT data used to supplement survey data at Site 8.
$\wedge$ Site 11 was removed as Lawrence Hargrave Drive is covered under Site 10.


## 3.3 future peak hour traffic conditions

The performance of the road system is typically assessed based on a 10 year growth scenario in background traffic levels. This therefore establishes a 'base case' against which the potential impacts of the Project can then be measured. The growth rates discussed above in the context of daily volumes have therefore been applied to the existing surveyed intersection (turning movement) volumes to establish the future peak hour base case scenario for all intersections.

Application of these rates to the existing survey volumes results in future intersection performances as shown in table 11 below. It is emphasised that these results include existing traffic associated with the Appin Mine and West Cliff Colliery, but do not account for additional movements associated with the proposed Project.

The analysis excludes intersections that are not expected to be impacted by the Project. Therefore, intersections along the haulage route to Coal Cliff and Corrimal Coke Works, which would each be associated with an additional eight heavy vehicle movements per day due to the Project, are not considered further in this assessment due to the negligible increases in Project-related traffic. Although some minor employee traffic would make use of these intersections, they are more than approximately 30 km away from the nearest pit top and employee contributions to traffic volumes at this distance are expected to be negligible.

## table 11: future (2019) intersection performance: am and pm peak hour

 (without Project)| Site | Intersection Description | Control Type | Period | DOS | AVD (s/veh) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| HAULAGE ROUTES |  |  |  |  |  |  |
| A | Appin Road/George Sreet (Appin East pit top exit) | signals | AM | 0.613 | 16.5 | B |
|  |  |  | PM | 0.750 | 20.5 | B |
| B | Appin Road/Sheriff Road (Appin East pit top entry) | priority | AM | 0.196 | 14.5 | B |
|  |  |  | PM | 0.172 | 14.8 | B |
| C | Appin Road/West Cliff pit top access | priority | AM | 0.369 | 34.7 | C |
|  |  |  | PM | 0.346 | 28.6 | C |
| D | Appin Road/Princes Highway off-ramp | priority | AM | 0.343 | 19.0 | B |
|  |  |  | PM | 0.305 | 24.7 | B |
| E | Mount Ousley Road/F6 Southern Freeway | priority | Not relevant as existing intersection fails |  |  | F |
|  |  |  |  |  |  | F |
| F | Springhill Road/Masters Road | signals | AM | 0.847 | 31.3 | C |
|  |  |  | PM | 0.863 | 40.4 | C |
| G | Springhill Road/Recycling Road | signals | AM | 0.942 | 32.4 | C |
|  |  |  | PM | 0.911 | 27.0 | B |
| H | Springhill Road/Port Kembla Road | signals | AM | 0.280 | 7.2 | A |
|  |  |  | PM | 0.470 | 10.5 | A |
| LOCAL ROADS |  |  |  |  |  |  |
| K | Picton Road/Almond Street | priority | AM | 0.628 | 40.7 | C |
|  |  |  | PM | 0.833 | 63.3 | E |
| L | Wilton Road/Macarthur Road | priority | AM | 0.068 | 16.3 | B |
|  |  |  | PM | 0.075 | 15.9 | B |
| M | Wilton Rd/Douglas Park Drive | priority | AM | 0.168 | 14.7 | B |
|  |  |  | PM | 0.170 | 15.6 | B |
| N | Appin Road/Wilton Road | priority | AM | 0.372 | 25.3 | B |
|  |  |  | PM | 0.489 | 16.9 | B |

Note: No turning vehicles in PM peak at Corrimal Coke Works intersection.

It can be seen from table 11 above that all intersections will experience increased delays as a result of significant increases in traffic volumes predicted in the next 10 years. These delays will occur independent of the Project and in this regard, this scenario forms an appropriate 'base case' scenario with which to assess the relative impact of Project related traffic increases. The SIDRA output summaries are provided in attachment d2 for reference purposes. It is emphasised that the 2019 scenario discussed in the following sections represents a worst case for intersection performance due to background traffic growth; while conditions in 2013 relate to smaller, short term impacts.

### 3.3.1 picton road and almond street (wilton road)

The LOS at this intersection has decreased with background traffic growth from $C$ to $E$ in the PM peak hour. The existing operations contributions to this intersection are minor (i.e. 1.6\%). Picton Road and Almond Street would require additional capacity in the future as it is currently congested under existing conditions, particularly to safely cater for right turn movements from Almond Street onto Picton Road. The RTA has already committed to improving road safety along Picton Road and it is expected that this intersection will form part of those works in the future (RTA, 2009). The opportunity could also be taken to implement additional safety measures. This is considered highly beneficial to the surrounding road users considering the relative frequency of accidents at or in the vicinity of this intersection.

## 4. potential impacts of the project

## 4.1 additional traffic generation

### 4.1.1 project haulage

The annual rate of coal delivery to PKCT and BlueScope Steelworks would increase as a result of the Project. Deliveries to Corrimal and Coalcliff Coke Works, and Dendrobium Washery would also marginally increase above current levels. Delivery of coal to PKCT and BlueScope Steelworks would be up to approximately 7.5 Mtpa and 4 Mtpa , respectively. Coal deliveries to Corrimal and Coalcliff Coke Works would be up to 0.2 Mtpa (combined), and delivery of coal to Dendrobium Washery would be up to 0.5 Mtpa. While these are maximum anticipated deliveries to any destination, the peak Project coal delivery would not be more than 9.4 Mtpa. An average Project trucking capacity of 36.5 t has been taken for all haulage trucks after Cardno Eppell Olsen (2008).

### 4.1.2 visitors and deliveries

As shown in table 12, the Project is expected to generate additional vehicle movements to and from the pit tops each day, as a result of increased replacement/consumption of machine parts, fuel and other consumables. Delivery movements are expected to double as a result of the Project.
table 12: project courier and delivery movements

| Type of Vehicle | West Cliff pit top |  | Appin West pit top |  | Appin East pit top |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Movements | Percent of Total | Movements | Percent of Total | Movements | Percent of Total |
| Truck (machine parts/brine and sludge removal/other deliveries) | 36 /week | 4\% | 302 /week | 29\% | $6 /$ week | 2\% |
| Truck (fuel) | 40 /week | 4\% | 4 /week | 1\% | 1 /week | 1\% |
| Truck (oversize loads) | 12 /week | 1\% | $3 /$ week | 1\% | 1 /week | 1\% |
| Small truck/light vehicles (visitors/sale representatives) | $56 / w e e k d a y$ | 35\% | $32 /$ weekday | 19\% | $8 /$ weekday | 19\% |
| Small truck/vans (couriers/other deliveries) | $88 / w e e k d a y$ | 56\% | $83 /$ weekday | 50\% | $32 /$ weekday | 77\% |
| Total | 988 /week | 100 | 1,028 /week | 100 | 258 /week | 100 |

The source of traffic for deliveries is assumed not to have changed from those outlined in section 2. During Year 2013, it is likely that approximately 250 trucks would deliver longwall parts to the Appin West pit top over a six week period, to facilitate the movement of longwall mining operations from the West Cliff Area 5 domain to Appin West (Area 9) or an alternate domain (refer to Section 2 in the Main Report of the EA). This is a short-term increase and is therefore not considered appropriate for assessment purposes.

### 4.1.3 employee movements

Total movements generated by employees to/from the West Cliff pit top reduces during Year 2013 due to underground operations moving from the West Cliff Area 5 domain to Appin West (Area 9) or an alternate domain. For the purposes of this assessment, it is assumed approximately 343 underground employees would be moved from West Cliff pit top to Appin West pit top during Year 2013 (i.e. $80 \%$ of the West Cliff underground workforce moves to Appin West pit top and $20 \%$ moves to the Appin East pit top), bringing the total underground workforce accessing Appin West pit top to 866. The West Cliff Washery workforce is slightly increased to 88 . Approximately 216 employees would access underground operations from Appin East. A car pooling factor of 1.5 people per car at West Cliff pit top and 1.2 people per car at Appin East and Appin West pit tops was applied to the Project workforce numbers to calculate the total number of movements.

Total movements generated by employees, on-site contractors, and others were estimated to comprise approximately 118 movements per day to/from West Cliff pit top, approximately 360 movements per day to/from Appin East pit top and approximately 1,444 movements to/from Appin West pit top.

Details of the approximate distribution of these employee/contractor movements are provided in section 2.9.3 and are repeated below:
7. 50 percent to/from the south (i.e. Wollongong south of Fairy Meadow using F6 Southern Freeway and Appin Road).
7. 30 percent to/from the south (north of Fairy Meadow to Scarborough using Princes Highway up Bulli Pass and Appin Road).
„ 15 percent from local areas (i.e. Camden, Campbelltown, Douglas Park, Mittagong, Picton and Wilton) using Picton Road, Hume Highway, Remembrance Driveway, Menangle Road, Appin Road north of Appin and Wilton Road.
„ 5 percent from the north (i.e. Sydney), using the F5 South Western Freeway/Hume Highway and F6 Southern Freeway.

This traffic assessment assumes that the destinations of the courier and delivery vehicle movements when they leave the pit tops are the same as their origins as estimated by ICHPL. The assessment has also assumed that $1 / 3$ of the workforce leaves the site between 8am and 9am (i.e. night crew leaving for three shift option), and $1 / 2$ of the workforce arrives at the site between 5 pm and 6 pm (i.e. night crew arriving for two shift option).

Due to access for heavy vehicles travelling to Appin West pit top being restricted by Broughtons Pass Weir, it is assumed that all heavy vehicles travelling to Appin West pit top from Wollongong gain access via Picton Road west of Mount Keira Road, approaching Appin West pit top from south-east of Wilton. Heavy vehicles from the north (e.g. Campbelltown) would use the F5 South Western Freeway and Picton Road. Due to travel distances, light vehicles from Wollongong (south of Fairy Meadow) are also assumed to gain access to the Appin West pit top via Picton Road, west of Mount Keira Road.

Based on the above assumptions, the estimated Project traffic movements per weekday are provided in table 13.
table 13: project movements per weekday

| Site | Road and Location | Existing (refer to table 10) |  |  | Year 2013 |  |  | Year 2019 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total | ICHPL <br> Vehicle movements | ICHPL Percent of Total | Total | ICHPL <br> Vehicle Movements | ICHPL Percent of Total | Total | ICHPL <br> Vehicle Movements | ICHPL Percent of Total |
| To/from PKCT |  |  |  |  |  |  |  |  |  |  |
| 1 | Bulli-Appin Road, Appin near Kings Fall Bridge | 8,330 | 948 | 11.4\% | 9,516 | 1,813 | 19.1\% | 10,021 | 1,813 | 18.1\% |
| 2 | Appin Road north of Princes Highway | 10,247 | 1,853 | 18.1\% | 11,996 | 2,658 | 22.2\% | 13,576 | 2,658 | 19.6\% |
| 3 | Mount Ousley Road at Mount Pleasant | 46,348 | 1,344 | 2.9\% | 54,717 | 1,904 | 3.5\% | 68,968 | 1,904 | 2.8\% |
| 4 | F6 Southern Freeway north of Princes Highway interchange at West Wollongong | 78,964 | 1,335 | 1.7\% | 86,296 | 1,904 | 2.2\% | 97,547 | 1,904 | 2.0\% |
| 5 | F6 Southern Freeway south of Princes Highway near footbridge | 76,421 | 1,335 | 1.7\% | 83,525 | 1,904 | 2.3\% | 94,397 | 1,904 | 2.0\% |
| 6 | Masters Road at Mount St. Thomas | 26,539 | 950 | 3.6\% | 27,174 | 1,585 | 5.8\% | 27,174 | 1,585 | 5.8\% |
| 7 | Springhill Road at Coniston | 16,172 | 566 | 3.5\% | 17,202 | 1,264 | 7.4\% | 17,712 | 1,264 | 7.1\% |
| To/from BlueScope Steelworks and Dendrobium Washery |  |  |  |  |  |  |  |  |  |  |
| 8 | Springhill Road north of Five Islands Road | 42,025 | 384 | 0.9\% | 43,872 | 674 | 1.5\% | 46,315 | 674 | 1.5\% |
| Route to/from Corrimal Coke Works |  |  |  |  |  |  |  |  |  |  |
| - | Northern Distributor, Towradgi, south of Towradgi Road | 32,452 | 9 | 0.0\% | 34,413 | 17 | 0.0\% | 37,565 | 17 | 0.0\% |

table 13: project movements per weekday (continued)

| Site | Road and Location | Existing (refer to table 10) |  |  | Year 2013 |  |  | Year 2019 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total | ICHPL <br> Vehicle movements | ICHPL <br> Percent of Total | Total | ICHPL <br> Vehicle Movements | ICHPL <br> Percent of Total | Total | ICHPL <br> Vehicle Movements | ICHPL <br> Percent of Total |
| Route to/from Coalcliff Coke Works |  |  |  |  |  |  |  |  |  |  |
| - | F6 Southern Freeway (North) | 42,709 | 56 | 0.1\% | 50,047 | 91 | 0.2\% | 63,408 | 91 | 0.1\% |
| 10 | Lawrence Hargrave Drive east of Princes Highway | 4,443 | 9 | 0.2\% | 5,412 | 17 | 0.3\% | 7,258 | 17 | 0.2\% |
| Local Roads |  |  |  |  |  |  |  |  | 0 |  |
| 12 | Princes Highway at Bulli Pass | 11,096 | 456 | 4.1\% | 12,583 | 685 | 5.4\% | 14,741 | 685 | 4.6\% |
| 13 | Princes Highway north of Bellambi Lane, Russell Vale | 26,288 | 456 | 1.7\% | 27,563 | 685 | 2.5\% | 29,210 | 685 | 2.3\% |
| 14 | Narellan-Appin Road north of Appin township, south of Brian Road | 10,003 | 150 | 1.5\% | 10,256 | 224 | 2.2\% | 10,527 | 224 | 2.1\% |
| 15 | Remembrance Driveway north of Finns Road | 11,191 | 43 | 0.4\% | 12,615 | 67 | 0.5\% | 15,049 | 67 | 0.4\% |
| 16 | Menangle Road at Nepean River Bridge | 6,631 | 43 | 0.6\% | 7,708 | 67 | 0.9\% | 9,610 | 67 | 0.7\% |
| 17 | Remembrance Driveway 0.5 km north of Regreme Road | 3,652 | 43 | 1.2\% | 3,676 | 67 | 1.8\% | 3,676 | 67 | 1.8\% |
| 18 | Menangle Road east of Picton-Oakdale Road | 2,991 | 43 | 1.4\% | 3,691 | 67 | 1.8\% | 5,003 | 67 | 1.3\% |
| 19 | Picton Road at Nepean River Bridge | 8,282 | 129 | 1.6\% | 9,579 | 202 | 2.1\% | 11,765 | 202 | 1.7\% |

table 13: project movements per weekday (continued)

| Site | Road and Location | Existing (refer to table 10) |  |  | Year 2013 |  |  | Year 2019 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Total | ICHPL <br> Vehicle movements | ICHPL Percent of Tota | Total | ICHPL <br> Vehicle Movements | ICHPL <br> Percent of Total | Total | ICHPL <br> Vehicle Movements | ICHPL <br> Percent of Total |
| 20 | Remembrance Driveway 0.8 km south of Tahmoor Post Office | 7,573 | 43 | 0.6\% | 7,597 | 67 | 0.9\% | 7,597 | 67 | 0.9\% |
| 21 | Picton Road west of Mount Keira Road | 11,213 | 348 | 3.1\% | 12,854 | 777 | 6.0\% | 14,923 | 777 | 5.2\% |
| 22 | Wilton Road at Clements Creek | 2,125 | 129 | 6.1\% | 2,198 | 202 | 9.2\% | 2,198 | 202 | 9.2\% |

It is evident from table 13 that in most cases, the main contributor to future traffic levels is due to growth in background traffic volumes and not specifically the Project. The notable exception is Appin Road where the Project presently accounts for 11.4\% of existing traffic at King's Fall Bridge and 18.1\% of existing traffic north of the Princes Highway. Based on the estimates in table 13, the proportion of Project traffic would increase by $7.7 \%$ and $4.1 \%$ during Year 2013, respectively. At these traffic levels, the operation of the road network would be dictated by intersection performances rather than mid-block capacities.

The distribution of the additional vehicle movements associated with the Project relative to that of the existing operations are shown in the turning count spreadsheet included in attachment e. This assumes a distribution as discussed previously for each of the separate pit tops for employee and visitor arrivals/departures.

For the purposes of assessment it has been assumed that on average $65 \%$ of road haulage occurs during the day and $35 \%$ of road haulage occurs during the night (i.e. day is 7.00 am to 6.00 pm and nights are 6.00 pm to 7.00 am ) with an average truck load of 36.5 t per truck (Cardno Eppell Olsen, 2008).

## 4.2 impact on peak hour intersection performance

The potential impacts of the Project are evident by the relative increase in delays at critical intersections along key roads utilised by haulage trucks, employees and visitors/deliveries. These are summarised in table 14 below, and the SIDRA outputs are provided in attachment d3. For the purposes of assessment conservatism, all visitor and delivery movements have been assumed to be heavy vehicles. In practice however, a high proportion of these traffic movements are undertaken by smaller vehicles.
table 14: future (2019) plus project intersection performance: am and pm peak hour

| Site | Intersection Description | Control Type | Period | DOS | AVD (s/veh) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| haULAGE ROUTES |  |  |  |  |  |  |
| A | Appin Road/George Sreet (Appin East pit top exit) | signals | AM | 0.639 | 16.7 | B |
|  |  |  | PM | 0.827 | 21.9 | B |
| B | Appin Road/Sheriff Road (Appin East pit top entry) | priority | AM | 0.219 | 32.5 | C |
|  |  |  | PM | 0.218 | 21.7 | B |
| C | Appin Road/West Cliff pit top access | priority | AM | 0.697 | 65.8 | E |
|  |  |  | PM | 0.849 | 103.1 | F |
| D | Appin Road/Princes Highway off-ramp | priority | AM | 0.365 | 19.7 | B |
|  |  |  | PM | 0.328 | 26.8 | B |
| E | Mount Ousley Road/F6 Southern Freeway | priority | Not relevant as intersection fails |  |  | F |
|  |  |  |  |  |  | F |
| F | Springhill Road/Masters Road | signals | AM | 0.862 | 33.0 | C |
|  |  |  | PM | 0.885 | 41.5 | C |
| G | Springhill Road/Recycling Road | signals* | AM | 0.974 | 44.2 | D |
|  |  |  | PM | 0.918 | 29.4 | C |
| H | Springhill Road/Port Kembla Road | signals | AM | 0.280 | 8.5 | A |
|  |  |  | PM | 0.470 | 11.2 | A |
| LOCAL ROADS |  |  |  |  |  |  |
| K | Picton Road/Almond Street | priority | AM | 0.692 | 44.3 | D |
|  |  |  | PM | 0.938 | 85.6 | F |
| L | Wilton Road/Macarthur Road | priority | AM | 0.188 | 16.6 | B |
|  |  |  | PM | 0.285 | 18.4 | B |
| M | Wilton Rd/Douglas Park Drive | priority | AM | 0.388 | 16.4 | B |
|  |  |  | PM | 0.545 | 20.0 | B |
| N | Appin Road/Wilton Road | priority | AM | 0.394 | 27.5 | B |
|  |  |  | PM | 0.517 | 20.3 | B |

* LOS ' $D$ ' is considered satisfactory with a signals control type.

It can be seen that the majority of key intersections relied upon by the Project would continue to operate efficiently. However, some intersections would require improvements in order to provide sufficient capacity. Recommended improvements to these intersections are discussed further in section 4.4. It should be noted that the majority of this capacity is taken up by existing traffic volumes and future growth and not as a direct result of the Project. This is evident from a comparison between tables 11 and 14.

## 4.3 consideration of cumulative impacts

There are several proposals in the locality that will require the relevant Councils and the RTA to examine cumulative traffic impacts, in the context of their strategic planning responsibilities. These include the following:

$\pi \quad$ Wilton Quarry, Wilton Road;<br>$\pi \quad$ BlueScope Steel - Steel Injection Station Project, Port Kembla (major works to be completed 2010 [CH2MHILL, 2008]);<br>$\pi \quad$ other Southern Coalfield mining operations (e.g. Metropolitan Colliery, Tahmoor Colliery and NRE No. 1 Colliery); and<br>$\pi$ Leaf's Gully Power Station.

In so far as the Project is concerned, it is common to assess impacts generated by the Project itself, with accommodation for growth in background traffic over a 10 year period being the normal approach. This growth, which is substantial in relative terms, takes account of these projects to a significant extent and the inclusion of additional traffic to account for specific projects introduces a potential for 'double counting' which is inappropriate. In addition, the growth factors applied have been derived from strategic modelling based on several strategic reports as referenced in this report, which would reasonably be expected to take account of the above projects.

## 4.4 required capacity changes to the surrounding road network

The following intersections would require additional capacity in the future in order to accommodate both Project related traffic and background future traffic growth.

### 4.4.1 appin road and west cliff pit top access road

The right turn from the West Cliff pit top access westwards onto Appin Road would experience unacceptable delays during the PM peak period (table 14). As such, improvements would be required to safely accommodate the increased traffic associated with the Project. ICHPL have recently consulted with the RTA design team with regard to potential recommendations/improvements to the intersection.

It is recommended that the intersection upgrade should involve signals installation or an alternative design determined in consultation with the RTA. This upgrade should be consistent with the RTA's Road Design Guide (1996). The performance of the modified intersection arrangement at the Appin Road/West Cliff pit top access is shown in table 15 below. Roundabout control is not considered a suitable solution for the West Cliff pit top access as this would require significant land-take in order to accommodate movements by heavy vehicles.
table 15: future (2019) plus project intersection performance: am and pm peak hour

| Intersection <br> Description | Control Type | Period | Degree of <br> Saturation <br> (DOS) | Intersection <br> Delay <br> (s/veh) | Level of <br> Service <br> (LOS) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Road/West Cliff pit <br> top access | Signals | AM | 0.715 | 9.4 | A |
|  |  | PM | 0.629 | 9.8 | A |

The SIDRA output summaries are provided in attachment d4.

### 4.4.2 picton road and almond street (wilton road)

Both the existing operations and proposed Project contributions to this intersection are minor (i.e. 1.6\% and $2.1 \%$ in Year 2013, respectively). As described in section 3.3.1, Picton Road and Almond Street would require additional capacity in the future as it is currently congested under existing conditions, particularly to safely cater for right turn movements from Almond Street onto Picton Road. The RTA has already committed to improving road safety along Picton Road and it is expected that this intersection will form part of those works in the future (RTA, 2009). The opportunity could also be taken to implement additional safety measures. This is considered highly beneficial to the surrounding road users considering the relative frequency of accidents at or in the vicinity of this intersection.

### 4.4.3 mount ousley road and the F6 southern freeway

This intersection would require an alternate intersection treatment in the future and is currently congested under existing conditions. This is therefore considered a matter for the RTA to address separately, whether or not the Project proceeds. It is likely that a grade separated right turn movement turning onto Mount Ousley Road would be required in order to maintain the free flow conditions between Mount Ousley Road (west) and the F6 Southern Freeway.

Both the existing operations and proposed Project contributions to this intersection are minor (i.e. 2.9\% and $3.5 \%$ in Year 2013, respectively). This level of Project traffic is very minor in the context of existing traffic flows and would be accounted for within the typical background growth factor that would be adopted by the RTA when planning for future changes at this intersection.

## 5. consideration of alternative haulage routes

Current planning laws require consideration be given to the existing haulage routes used by the existing operations to determine if these routes are the most acceptable with regard to general environmental impacts, taking into consideration the distance travelled and amenity in built up areas. No changes to the existing routes are proposed as a result of the Project and these are discussed in more detail for each route below.

## 5.1 to/from pkct, dendrobrium washery \& bluescope steelworks

The existing route from the Appin Mine and West Cliff Colliery (i.e. Appin Road, F6 Southern Freeway and Mount Ousley Road) to the industrial facilities surrounding Port Kembla is considered the most appropriate route. Masters Road was specifically constructed in order to service traffic from the Port Kembla area and reliance on this route is appropriate.

In summary, the existing transport routes are considered the most appropriate for adoption given the majority of the route is high capacity and no reasonable alternative route exists. This could be reviewed periodically if required to take account of changes to the road network system that could influence the most appropriate route choice for haulage vehicles.

## 5.2 to/from coalcliff coke works

The existing route to the Coalcliff Coke Works is approximately 32 km in length. An alternative option includes using the Princes Highway (Bulli Pass) and Lawrence Hargrave Drive to access the Coalcliff Coke Works which would reduce the travelling distance to some 27 km . However, this is not considered appropriate due to reduced speed limits and the additional number of residential and built up areas through which this route would be required to traverse, given the increased safety and environmental amenity impacts.

## 5.3 to/from corrimal coke works

The existing route to the Corrimal Coke Works is also approximately 32 km in length. A number of shorter alternative routes are available including the use of the Princes Highway (Bulli Pass). The shorter travel distance is offset however by the reduced vehicle speeds along the alternative routes.

## 6. road safety review

## 6.1 pkct road safety audit

The PKCT Traffic Study (Cardno Eppell Olsen, 2008) conducted a road safety audit of key portions of the coal haulage route that would be used by the Project. The findings of the audit are summarised below. In general the PKCT Traffic Study road safety audit revealed that the road sections were in good condition with no significant safety issues. However, the following issues on various sections of the haulage routes were noted:

- Appin Road between the Appin East pit top access road and West Cliff pit top access road:
- Some linemarking is in poor condition or obliterated;
- The guardrail safety barrier at the Appin East pit top entry access road is too low and unsafe; and
- The pavement is badly rutted at the West Cliff pit top entry access road.
- Appin Road between West Cliff pit top and Mount Ousley Road:
- The major bridge over Lodden River does not meet the required safety standards for the volume of heavy vehicles now using the road.
- Mount Ousley Road between Appin Road and F6 Southern Freeway:
- The number of pit lids on the outer lane northbound, approximately 2.5 km south of Picton Road, which have settled causing unsafe conditions for traffic.

It is recommended that the deficiencies identified along Appin Road between the Appin East pit top and West Cliff pit top, be addressed as part of routine maintenance works.

The issues raised in the road safety audit conducted as part of the PKCT Traffic Study (Cardno Eppell Olsen, 2008) relating to the bridge over Lodden River and the settling of pit lids on Mount Ousley Road between Appin Road and F6 Southern Freeway raises general concerns that are largely independent of the Project and should be addressed in that context (i.e. as an existing deficiency).

## 6.2 project road safety

The road accident history of the haulage routes and local roads was undertaken by analysing validated data provided by the RTA for the period January 2003 to December 2007. The data is recorded in the event that a traffic incident/crash meets all of the following criteria:
$\pi$ was reported to the Police; and
$\pi$ occurred on a road open to the public; and
$\pi$ involved at least one moving vehicle; and
„ involved at least:

- one person being killed or injured; or
- one vehicle being towed away.

Over the five year period of data reviewed there have been 1,220 reported crashes along the roads used as haulage routes for the Appin Mine and West Cliff Colliery, including 16 fatal crashes, 460 injury crashes and 744 tow-away crashes. Picton Road, recently the beneficiary of funding to implement road safety measures, has recorded a total of 168 crashes including 6 fatal crashes, 78 injury crashes and 84 tow-away crashes.

The accident data was reviewed to determine whether any accidents in the period investigated involved trucks on the key haulage routes used by ICHPL haulage contractor trucks. Over the five year period investigated, all of the heavy vehicle accidents along the haulage routes used by the haulage contractor were either injury or tow-away accidents. Of the 236 heavy vehicle accidents along these haulage routes, 87 were injury crashes with the remaining 149 crashes involving the tow-away of one or more vehicles. The location of all accidents involving heavy vehicles are shown in figures $\mathbf{6 a}$ to $\mathbf{6 c}$. It is evident that only nine heavy vehicle crashes occurred along Appin Road between Wilton Road and the Princes Highway, which is considered moderate over a five year period having regard for the length of this road (approximately 18 km ).

Based on figures $\mathbf{6 a}$ to $\mathbf{6 c}$, the following areas have been identified as potential locations for further consideration by the RTA in the context of its ongoing road safety programme (it should be noted that these sections of road have been identified based on existing safety conditions):

त intersection of Masters Road with Drummond Street, particularly right turn from Drummond Street;
7. section of Lawrence Hargrave Drive between Chellow Dene Avenue and Otford Road;
$\pi$ intersection of Picton Road with Almond Street, particularly right turn from Almond Street; and


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road transport assessment: bulli seam operations project
7. Picton Road and the F5 South Western Freeway/Hume Highway (northbound) on-ramp.

It should also be noted that the Project results in minimal additional vehicle traffic along these sections of road and the safety of these roads is not expected to reduce by any significant extent as a result of the Project. Despite the larger proportion of Project-related traffic movements on Appin Road, there is no adverse crash history that warrants special attention, based either on the analysis of the accident data or the road safety audit undertaken separately by Cardno Eppell Olsen (2008).

Generally the identified areas are associated with right turn movements from minor roads onto roads with relatively high through-traffic volumes. It is expected that the above areas along Picton Road will be addressed within the current improvements planned for Picton Road by the RTA.

The 300 m section of Lawrence Hargrave Drive of potential concern primarily relates to a number of heavy vehicles travelling off the carriageway due to excessive speed. Five of the six accidents involving heavy vehicles in this location involved excessive speed factors. Additional recommended speed signage would assist in informing drivers of the conditions and increased shoulder width could be provided, subject to RTA funding and priority constraints, to reduce the potential for additional injury crashes along this length of road. It is emphasised that these accidents included all vehicles on the public road system of which the Project-related traffic is a minor proportion. For example, the existing operations traffic on Lawrence Hargrave Drive represents $0.2 \%$ of the existing traffic volumes, which increases to $0.3 \%$ in Year 2013 and reduces back to $0.2 \%$ (including additional Project traffic) in 10 years when growth in background traffic is taken into account (at $5 \%$ per annum based on the Metropolitan Colliery Traffic Assessment [Masson Wilson Twiney, 2008]).

## 7. conclusions

In summary:

7 The relative increase in road haulage volumes are minor particularly considering the total existing and future background traffic levels of roads along the Project haulage routes

7 The assessment is based on a 'worst case' scenario, where the Project traffic has been included as additional to the background growth volumes over a 10 year period
7. Some minor changes to three intersections would require additional treatment to ensure that sufficient capacity and adequate safety is provided. It should be noted that the majority of the capacity of these intersections is taken up by existing background traffic volumes and future growth over the next 10 years and not by Project-related traffic. These intersections are:

Appin Road and West Cliff pit top access road (an upgrade to this intersection is recommended and should involve a signals installation or an alternative design determined in consultation with the RTA). This upgrade should be consistent with the RTA's Road Design Guide (1996);

Picton Road and Almond Street (Wilton Road) (the RTA has already committed to improving road safety along Picton Road [RTA, 2009]); and

Mount Ousley Road and the F6 Southern Freeway (this intersection is considered a matter for the RTA to address separately, as the Project contributes minimal additional through traffic at this intersection).
7. Road haulage occurs primarily on arterial and other RTA controlled roads. This reduces the potential for other impacts on surrounding land owners and residents. Furthermore, this road network includes a number of roads that are provided primarily to facilitate access to the Port Kembla facilities.

त It is recommended that the deficiencies identified along Appin Road between the Appin East pit top and West Cliff pit top described in the PKCT Traffic Study road safety audit (Cardno Eppell Olsen, 2008) be addressed as part of routine maintenance works. Any upgrades undertaken should be consistent with the RTA's Road Design Guide (1996).
7. There have been a number of crashes in the past along roads that would continue to be used as the haulage routes for the Project in the future. However, the Project results in minimal additional vehicle traffic along these sections of road, and the safety of these roads is not expected to reduce by any significant extent as a result of the Project.

## 8. references

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## attachment a

summary of AADT data

Historic Annual Average Daily Traffic Data 1999 to 2007

| Road | Location | 1999 | 2000 | 2001 | 2002 | 2003 | 2004 | 2005 | 2006 | 2007 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Route from Appin East Pit Top to West Cliff Pit Top |  |  |  |  |  |  |  |  |  |  |
| Appin Road | Appin, West of Princes Highway |  | 8463 | 7911 | 8969 | 9008 | 9081 | 10991 | 9528 | 9,030 |
| Route to/from PKCT |  |  |  |  |  |  |  |  |  |  |
| Appin Road | Appin, West of Princes Highway |  | 8463 | 7911 | 8969 | 9008 | 9081 | 10991 | 9528 | 9,030 |
| Mt Ousley Road | Mt Pleasant, South of Clive Bissell Drive |  | 36822 | 33940 | 38941 | 40285 | 41500 | 39799 | 39881 | 29571 |
|  | Mount Ousley Road south of freeway on-ramp |  | 31627 |  |  | 34570 |  | 31958 |  |  |
| F6 Freeway (South) | Mt Ousley, South of Mt Ousley Road |  | 34759 |  |  | 35642 |  | 42220 |  |  |
|  | West Wollongong at Gipps Road overpass |  | 58758 | 59211 | 64168 | 68681 | 70269 | 72310 | 68945 | 69771 |
| Masters Road | Mt St Thomas, West of Springhill Road |  | 25317 |  |  | 25226 |  |  |  |  |
| Springhill Road | Mt St Thomas, North of Masters Road |  | 35226 |  |  | 31147 |  | 35179 |  |  |
|  | Coniston, North of Kiera Street |  | 16184 |  |  | 15582 |  | 16600 |  |  |
| Route to/from BlueScope Steelworks and Dendrobium CPP |  |  |  |  |  |  |  |  |  |  |
| Appin Road | Appin, West of Princes Highway |  | 8463 | 7911 | 8969 | 9008 | 9081 | 10991 | 9528 | 9,030 |
| Mt Ousley Road | Mt Pleasant, South of Clive Bissell Drive |  | 36822 | 33940 | 38941 | 40285 | 41500 | 39799 | 39881 |  |
|  | Mount Ousley Road south of freeway on-ramp |  | 31627 |  |  | 34570 |  | 31958 |  |  |
| F6 Freeway (South) | Mt Ousley, South of Mt Ousley Road |  | 34759 |  |  | 35642 |  | 42220 |  |  |
|  | West Wollongong at Gipps Road overpass |  | 58758 | 59211 | 64168 | 68681 | 70269 | 72310 | 68945 | 69771 |
| Masters Road | Mt St Thomas, West of Springhill Road |  | 25317 |  |  | 25226 |  |  |  |  |
| Springhill Road | Mt St Thomas, North of Five Islands Road |  | 38723 |  |  | 35931 |  | 40524 |  |  |
| Route to/from Corrimal |  |  |  |  |  |  |  |  |  |  |
| Appin Road | Appin, West of Princes Highway |  | 8463 | 7911 | 8969 | 9008 | 9081 | 10991 | 9528 | 9,030 |
| Mt Ousley Road | Mt Pleasant, South of Clive Bissell Drive |  | 36822 | 33940 | 38941 | 40285 | 41500 | 39799 | 39881 |  |
|  | Mount Ousley Road south of freeway on-ramp |  | 31627 |  |  | 34570 |  | 31958 |  |  |
| F6 Freeway (South) | Mt Ousley, South of Mt Ousley Road |  | 34759 |  |  | 35642 |  | 42220 |  |  |
| University Avenue/Graham Avenue | Gwynneville, West of F6 Freeway (South) |  |  |  |  |  |  |  |  |  |

Historic Annual Average Daily Traffic Data 1999 to 2007

| Road | Location | 1999 | 2000 | 2001 | 2002 | 2003 | 2004 | 2005 | 2006 | 2007 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Northern Distributor | Towradgi, South of Towradgi Road |  | 28453 | 29114 | 30033 | 30260 | 30619 | 30901 | 27909 | 31517 |
|  | Northern Distributor south of Old Princes Highway |  | 43108 |  |  | 38314 |  |  |  |  |
| Route to/from Coalcliff |  |  |  |  |  |  |  |  |  |  |
| Appin Road | Appin, West of Princes Highway |  | 8463 | 7911 | 8969 | 9008 | 9081 | 10991 | 9528 | 9,030 |
| F6 Freeway (North) | Bulli Tops, North of Appin Road |  | 29942 | 34202 | 35370 | 36459 | 38098 | 36897 | 37941 |  |
| Lawrence Hargrave Dr | Stanwell Park, West of Bald Hill Road |  | 4620 |  |  | 5,546 |  |  |  |  |
| OTHER ROADS |  |  |  |  |  |  |  |  |  |  |
| Remembrance Driveway | Camden, 500 m south of Burragorang Road | 14289 |  |  | 15247 |  |  | 17377 |  |  |
|  | North of Finns Road |  | 8912 | 9694 |  | 11084 |  |  | 10635 |  |
|  | Picton, 0.5 km north of Regreme Road |  | 7704 | 7317 |  | 8564 |  |  | 7460 |  |
|  | 0.8 km south of Tahmoor Post Office |  | 10705 | 6260 |  | 10121 |  |  |  |  |
| Menangle Road | Menangle Park at Nepean River Bridge | 4571 |  |  | 5041 |  |  | 5858 |  |  |
|  | Nepean River Bridge |  | 4764 | 4928 |  | 5604 |  |  | 5943 |  |
|  | East of Picton-Oakdale Road |  | 2421 | 2475 |  | 2821 |  |  |  |  |
|  | Menangle Road east of Gilchrist Drive |  |  |  | 5173 |  |  | 7192 |  |  |
|  | Menangle Road East of Tailby Street | 10118 |  |  | 10236 |  |  | 8983 |  |  |
| Menangle Street | Picton, south of Prince Street |  | 7167 |  |  | 8149 |  |  |  |  |
| Argyle Street | South of Menangle Street |  | 12019 | 12912 |  | 14153 |  |  |  |  |
| Picton Road | West of Mt Keira Rd |  | 12430 |  |  | 12145 |  | 10953 |  | 11084 |
|  | East of Mount Keira Road |  | 11428 |  |  | 11705 |  | 13639 |  |  |
|  | At Nepean River Bridge |  | 7074 | 6991 |  | 7845 |  |  |  |  |
|  | East of Hume Highway |  | 10674 | 11082 | 11781 | 12068 | 12818 | 13015 | 11659 | 14426 |
|  | South of Appin Road |  | 11493 |  |  | 11172 |  |  |  |  |
|  | East of Cordeaux Dam Road |  | 11296 |  |  | 11699 |  |  |  |  |
| Hume Highway | Hume Highway at Menangle |  | 31324 | 31877 | 33787 | 35359 | 38080 | 36887 | 35531 | 37594 |
|  | Hume Highway at Pheasants Nest |  | 27040 | 27098 | 29284 | 29798 | 26595 | 30042 | 29912 | 31110 |
|  | North of Narellan Road Interchange | 43514 |  |  | 47641 |  |  | 50514 |  |  |

Historic Annual Average Daily Traffic Data 1999 to 2007

| Road | Location | 1999 | 2000 | 2001 | 2002 | 2003 | 2004 | 2005 | 2006 | 2007 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Road | Wilton Road at Broughtons Pass |  | 1835 | 1670 |  | 1775 |  |  |  |  |
| Appin Road | Appin Road north of Maldon Road |  | 8906 | 9144 | 9288 | 9255 | 9435 | 9243 | 9121 | 9184 |
| Mt Keira Road | South of Picton Road |  | 1576 |  |  | 682** |  | 701 |  |  |
|  | West of Abercrombie Road |  | 6840 |  |  | 7108 |  | 6589 |  |  |
|  | West Wollongong, West of F6 Freeway |  | 12548 |  |  | 13078 |  | 12192 |  |  |
| Princes Highway | Bulli Pass, east of Mt Ousley Rd |  | 10138 | 10017 | 10525 | 11135 | 12340 |  | 10024 | 10282 |
| Mt Ousley Road | Mount Ousley Road west of Old Princes Highway |  | 10691 |  |  | 10719 |  | 10987 |  |  |
| Lawrence | East of Old Princes Highway |  | 3559 |  |  | 5070 |  | 4166 |  | 4602 |
|  | West of Bald Hill Road |  | 4620 |  |  | 5546 |  |  |  |  |

Source: RTA Traffic Volume Data (2003, 2005, 2006, 2007, 2008). PKCT Traffic Assessment (Cardno, 2008).
** Coal trucks not using this road during survey.

## attachment b

photographic record


View looking south along Douglas Park Drive towards its intersection with the Appin West pit top access.


View looking north along Wilton Road at its intersection with Douglas Park Drive.



View looking south along Macarthur Road from its intersection with Wilton Road.


View looking south along Almond Street towards its intersection with Picton Road.


View looking east along Picton Road at its intersection with Almond Street.


View looking east along Wilton Road on approach to the bridge at Broughtons Pass



View looking east north along Wilton Road on approach to Appin Road.


View looking east along Bulli-Appin Road from its intersection with Wilton Road.



View looking east along Bulli-Appin Road towards its intersection with the Appin East pit top egress road.


View looking west along Bulli-Appin Road towards the Appin East pit top entry.


View looking south along Bulli-Appin Road towards its interchange with Mount Ousley Road and the Princes Highway.


View looking south at the interchange of southbound lanes of Appin Road and Mount Ousley Road.



View looking south along Mount Ousley Road at the Bulli Pass off-ramp.


View looking south along Mount Ousley Road at the Picton Road off-ramp.


View looking south along Mount Ousley Road that indicates the bus and truck speed limit on this section of road.


View looking west along Mount Ousley Road towards its interchange with the F6 Southern Freeway.


View looking east along Masters Road on approach to Springhill Road.


View looking north along Springhill Road at its intersection with Recycling Road.



View looking west along Railway Street towards its intersection with the Northern Distributor.


View looking east along Railway Street in the vicinity of the Corrimal Coke Works access.

## attachment C

survey results

R.O.A.R. DATA PTY. LTD.

Reliable, Original \& Authentic Results

83 Church St, Ryde NSW 2112.
Ph.88196847, Fax 88196849, Mob.0418-239019
Email: roardata@optusnet.com.au, Website: www.roardata.com.au

## FIELD SURVEY REPORT

| $\underline{\text { Day / date }}$ | $:$ | Thursday $9^{\text {th }}$ April 2009 |
| :--- | :--- | :--- |
| $\underline{\text { Client }}$ | $:$ | Traffix |
| $\underline{\text { Job No/Name }}$ | $:$ | 2654 Wollongong Area Traffic Counts |
| $\underline{\text { ATTENTION }}$ | $:$ | Tim Lewis |

Firstly the counts went well with the exception of TC 12 it was counted but we could only get 1 surveyor there in the afternoon. This in itself apparently was not enough it needed 2 I am informed. We prioritised it and counted the Bluescope Steel access and not worry about the Colourbond site across the road. I am told no heavy vehicles accessed the Colourbond site and there was a minimum number of cars. I think this site has offices only my apologies as you know it was short notice but we did manage to get data. The Mt. Ousley Rd and Southern Freeway site in the afternoon had longish queues westbound towards the freeway. These lasted from 4pm to 5pm and I am not sure if this added to the left turn or not. All other sites went well and we were only able to get the turning movements for the eastern access at the un signalised location. TC 04 to difficult to do through on both for 1 person.

I do hope that the results are useful to you and if you have any questions please contact me.


Appin Rd \& East Mine Access

To
Tim Lewis

## at <br> Traffix

your results for

## Wollongong Area Traffic Counts

supplied by
R.O.A.R. DATA Pty. Ltd.
www.roardata.com.au

Reliable, Original \& Authentic Results Ph.88196847, Fax 88196849.
Mobile. 0418239019

Client
Job No/Name Day/Date

Traffix
2654 Wollongong Area Traffic Surveys : Thursday 9th April 2009

Unsignalised

| PEDS | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Appin Rd | East Mine | Appin Rd | TOT |
| $0700-0715$ |  |  |  | $\mathbf{0}$ |
| $0715-0730$ |  |  |  | $\mathbf{0}$ |
| $0730-0745$ |  | NOT |  | $\mathbf{0}$ |
| $0745-0800$ |  | REQUIRED |  | $\mathbf{0}$ |
| $0800-0815$ |  |  |  | $\mathbf{0}$ |
| $0815-0830$ |  |  |  | $\mathbf{0}$ |
| $0830-0845$ |  |  |  | $\mathbf{0}$ |
| $0845-0900$ |  |  |  | $\mathbf{0}$ |
| $0900-0915$ |  |  |  | $\mathbf{0}$ |
| $0915-0930$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


| PEDS | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Appin Rd | East Mine | Appin Rd | TOT |
| $0700-0800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0715-0815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0730-0830$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0745-0845$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0800-0900$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0815-0915$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0830-0930$ | 0 | 0 | 0 | $\mathbf{0}$ |


| PEAK HR | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | I | R | $\underline{L}$ | R | $\underline{\underline{L}}$ | I | TOT |
| 0630-0645 |  | 0 | 0 | 0 | 0 |  | 0 |
| 0645-0700 |  | 0 | 0 | 0 | 0 |  | 0 |
| 0700-0715 |  | 0 | 0 | 0 | 2 |  | 2 |
| 0715-0730 |  | 0 | 0 | 0 | 0 |  | 0 |
| 0730-0745 |  | 0 | 0 | 0 | 0 |  | 0 |
| 0745-0800 |  | 0 | 0 | 0 | 0 |  | 0 |
| 0800-0815 |  | 0 | 0 | 0 | 0 |  | 0 |
| 0815-0830 |  | 0 | 0 |  | 0 |  | 0 |
| 0900-0915 |  | 0 | 0 | 0 | 0 |  | 0 |
| 0915-0930 |  | 0 | 0 | 0 | 0 |  | 0 |
| Per End | 0 | 0 | 0 | 0 | 2 | 0 | 2 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | I | $\underline{R}$ | $\underline{L}$ | $\underline{\text { R }}$ | $\underline{\text { L }}$ | I | TOT |
| 0630-0645 |  | 0 | 0 | 0 | 12 |  | 12 |
| 0645-0700 |  | 0 | 0 | 0 | 7 |  | 7 |
| 0700-0715 |  | 0 | 0 | 0 | 8 |  | 8 |
| 0715-0730 |  | 0 | 0 | 0 | 7 |  | 7 |
| 0730-0745 |  | 0 | 0 | 0 | 5 |  | 5 |
| 0745-0800 |  | 1 | 0 | 0 | 7 |  | 8 |
| 0800-0815 |  | 0 | 0 | 0 | 0 |  | 0 |
| 0815-0830 |  | 1 | 0 | 0 | 0 |  | 1 |
| 0900-0915 |  | 0 | 0 | 0 | 0 |  | 0 |
| 0915-0930 |  | 0 | 0 | 0 | 0 |  | 0 |
| Per End | 0 | 2 | 0 | 0 | 46 | 0 | 48 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | I | $\underline{R}$ | $\underline{\text { L }}$ | $\underline{R}$ | $\underline{\underline{L}}$ | I | TOT |
| 0630-0645 | 0 | 0 | 0 | 0 | 12 | 0 | 12 |
| 0645-0700 | 0 | 0 | 0 | 0 | 7 | 0 | 7 |
| 0700-0715 | 0 | 0 | 0 | 0 | 10 | 0 | 10 |
| 0715-0730 | 0 | 0 | 0 | 0 | 7 | 0 | 7 |
| 0730-0745 | 0 | 0 | 0 | 0 | 5 | 0 | 5 |
| 0745-0800 | 0 | 1 | 0 | 0 | 7 | 0 | 8 |
| 0800-0815 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0815-0830 | 0 | 1 | 0 | 0 | 0 | 0 | 1 |
| 0900-0915 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0915-0930 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Per End | 0 | 2 | 0 | 0 | 48 | 0 | 50 |


| Lights | $\frac{\text { WEST }}{\text { Appin } \text { Rd }}$ |  | $\begin{gathered} \hline \text { SOUTH } \\ \hline \text { East Mine } \end{gathered}$ |  | $\begin{gathered} \hline \text { EAST } \\ \hline \text { Appin Rd } \end{gathered}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Per | I | R | L | R | L | T | TOT |
| 0700-0800 | 0 | 0 | 0 | 0 | 2 | 0 | 2 |
| 0715-0815 | 0 | 0 | 0 | 0 | 2 | 0 | 2 |
| 0730-0830 | 0 | 0 | 0 | 0 | 2 | 0 | 2 |
| 0745-0845 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0800-0900 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0815-0915 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0830-0930 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | I | R | $\underline{L}$ | R | $\underline{\underline{L}}$ | I | TOT |
| 0700-0800 | 0 | 0 | 0 | 0 | 34 | 0 | 34 |
| 0715-0815 | 0 | 0 | 0 | 0 | 27 | 0 | 27 |
| 0730-0830 | 0 | 1 | 0 | 0 | 27 | 0 | 28 |
| 0745-0845 | 0 | 1 | 0 | 0 | 19 | 0 | 20 |
| 0800-0900 | 0 | 2 | 0 | 0 | 12 | 0 | 14 |
| 0815-0915 | 0 | 2 | 0 | 0 | 7 | 0 | 9 |
| 0830-0930 | 0 | 1 | 0 | 0 | 0 | 0 | 1 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | I | R | $\underline{\square}$ | R | $\underline{L}$ | I | TOT |
| 0700-0800 | 0 | 0 | 0 | 0 | 36 | 0 | 36 |
| 0715-0815 | 0 | 0 | 0 | 0 | 29 | 0 | 29 |
| 0730-0830 | 0 | 1 | 0 | 0 | 29 | 0 | 30 |
| 0745-0845 | 0 | 1 | 0 | 0 | 19 | 0 | 20 |
| 0800-0900 | 0 | 2 | 0 | 0 | 12 | 0 | 14 |
| 0815-0915 | 0 | 2 | 0 | 0 | 7 | 0 | 9 |
| 0830-0930 | 0 | 1 | 0 | 0 | 0 | 0 | 1 |




## East Mine Access



| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | $\underline{\mathbf{I}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{I}}$ | TOT |
| $1600-1615$ |  | 0 | 0 | 0 | 0 |  | $\mathbf{0}$ |
| $1615-1630$ |  | 0 | 0 | 0 | 0 |  | $\mathbf{0}$ |
| $1630-1645$ |  | 0 | 0 | 0 | 0 |  | $\mathbf{0}$ |
| $1645-1700$ |  | 0 | 0 | 1 | 0 |  | $\mathbf{1}$ |
| $1700-1715$ |  | 0 | 0 | 0 | 0 |  | $\mathbf{0}$ |
| $1715-1730$ |  | 0 | 0 | 0 | 0 |  | $\mathbf{0}$ |
| $1730-1745$ |  | 0 | 0 | 0 | 0 |  | $\mathbf{0}$ |
| $1745-1800$ |  | 0 | 0 | 0 | 0 |  | $\mathbf{0}$ |
| $1800-1815$ |  | 0 | 0 | 0 | 0 |  | $\mathbf{0}$ |
| $1815-1830$ |  | 0 | 0 | 0 | 0 |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{1}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{1}$ |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | I | R | $\underline{L}$ | R | $\underline{L}$ | I | TOT |
| 1600-1615 |  | 0 | 0 | 0 | 3 |  | 3 |
| 1615-1630 |  | 0 | 0 | 0 | 3 |  | 3 |
| 1630-1645 |  | 0 | 0 | 0 | 3 |  | 3 |
| 1645-1700 |  | 0 | 0 | 0 | 0 |  | 0 |
| 1700-1715 |  | 0 | 0 | 0 | 0 |  | 0 |
| 1715-1730 |  | 0 | 0 | 0 | 0 |  | 0 |
| 1730-1745 |  | 0 | 0 | 0 | 0 |  | 0 |
| 1745-1800 |  | 0 | 0 | 0 | 0 |  | 0 |
| 1800-1815 |  | 0 | 0 | 0 | 1 |  | 1 |
| 1815-1830 |  | 0 | 0 | 0 | 0 |  | 0 |
| Per End | 0 | 0 | 0 | 0 | 10 | 0 | 10 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | I | R | $\underline{L}$ | R | $\underline{L}$ | I | TOT |
| 1600-1615 | 0 | 0 | 0 | 0 | 3 | 0 | 3 |
| 1615-1630 | 0 | 0 | 0 | 0 | 3 | 0 | 3 |
| 1630-1645 | 0 | 0 | 0 | 0 | 3 | 0 | 3 |
| 1645-1700 | 0 | 0 | 0 | 1 | 0 | 0 | 1 |
| 1700-1715 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1715-1730 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1730-1745 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1745-1800 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1800-1815 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| 1815-1830 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Per End | 0 | 0 | 0 | 1 | 10 | 0 | 11 |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | $\underline{\mathbf{T}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{T}}$ | TOT |
| $1600-1700$ | 0 | 0 | 0 | 1 | 0 | 0 | $\mathbf{1}$ |
| $1615-1715$ | 0 | 0 | 0 | 1 | 0 | 0 | $\mathbf{1}$ |
| $1630-1730$ | 0 | 0 | 0 | 1 | 0 | 0 | $\mathbf{1}$ |
| $1645-1745$ | 0 | 0 | 0 | 1 | 0 | 0 | $\mathbf{1}$ |
| $1700-1800$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |
| $1715-1815$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |
| $1730-1830$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | $\underline{\text { I }}$ | R | $\underline{L}$ | R | $\underline{\text { L }}$ | $\underline{\text { I }}$ | TOT |
| 1600-1700 | 0 | 0 | 0 | 0 | 9 | 0 | 9 |
| 1615-1715 | 0 | 0 | 0 | 0 | 6 | 0 | 6 |
| 1630-1730 | 0 | 0 | 0 | 0 | 3 | 0 | 3 |
| 1645-1745 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1700-1800 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1715-1815 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |
| 1730-1830 | 0 | 0 | 0 | 0 | 1 | 0 | 1 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | $\underline{\mathbf{T}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathrm{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathrm{L}}$ | $\underline{\mathbf{T}}$ | TOT |
| $1600-1700$ | 0 | 0 | 0 | 1 | 9 | 0 | $\mathbf{1 0}$ |
| $1615-1715$ | 0 | 0 | 0 | 1 | 6 | 0 | $\mathbf{7}$ |
| $1630-1730$ | 0 | 0 | 0 | 1 | 3 | 0 | $\mathbf{4}$ |
| $1645-1745$ | 0 | 0 | 0 | 1 | 0 | 0 | $\mathbf{1}$ |
| $1700-1800$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |
| $1715-1815$ | 0 | 0 | 0 | 0 | 1 | 0 | $\mathbf{1}$ |
| $1730-1830$ | 0 | 0 | 0 | 0 | 1 | 0 | $\mathbf{1}$ |



## R.O.A.R. DATA



PM PEAK
1600-1700


East Mine Access

## East Mine Access

## R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019

Intersection Details
Obtained via satellite
May be incorrect
$\square$
TC-04


PM PEAK HOUR 1600-1700

Weather >>>


East Mine Access


Appin Rd \& East Mine Access
TC-04
Signals

To
Tim Lewis

## at <br> Traffix

your results for

## Wollongong Area Traffic Counts

supplied by

R.O.A.R. DATA Pty. Ltd.

www.roardata.com.au

| PEDS | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Appin Rd | East Mine | Appin Rd | TOT |
| $0700-0715$ |  |  |  | $\mathbf{0}$ |
| $0715-0730$ |  |  |  | $\mathbf{0}$ |
| $0730-0745$ |  | NOT |  | $\mathbf{0}$ |
| $0745-0800$ |  | REQUIRED |  | $\mathbf{0}$ |
| $0800-0815$ |  |  |  | $\mathbf{0}$ |
| $0815-0830$ |  |  |  | $\mathbf{0}$ |
| $0830-0845$ |  |  |  | $\mathbf{0}$ |
| $0845-0900$ |  |  |  | $\mathbf{0}$ |
| $0900-0915$ |  |  |  | $\mathbf{0}$ |
| $0915-0930$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


|  | PEDS | WEST | SOUTH | EAST |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |
| Peak Per | Appin Rd | East Mine | Appin Rd | TOT |
| $0700-0800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0715-0815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0730-0830$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0745-0845$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0800-0900$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0815-0915$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0830-0930$ | 0 | 0 | 0 | $\mathbf{0}$ |

TC-04-Signals

| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | $\underline{\mathbf{T}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{T}}$ | TOT |
| $0700-0715$ | 3 | 0 | 0 | 13 | 0 | 13 | $\mathbf{2 9}$ |
| $0715-0730$ | 2 | 0 | 0 | 11 | 0 | 11 | $\mathbf{2 4}$ |
| $0730-0745$ | 4 | 0 | 0 | 9 | 0 | 9 | $\mathbf{2 2}$ |
| $0745-0800$ | 3 | 0 | 0 | 7 | 0 | 7 | $\mathbf{1 7}$ |
| $0800-0815$ | 4 | 0 | 0 | 5 | 0 | 5 | $\mathbf{1 4}$ |
| $0815-0830$ | 7 | 0 | 0 | 9 | 0 | 9 | $\mathbf{2 5}$ |
| $0830-0845$ | 0 | 0 | 0 | 4 | 0 | 4 | $\mathbf{8}$ |
| $0845-0900$ | 4 | 0 | 0 | 1 | 0 | 1 | $\mathbf{6}$ |
| $0900-0915$ | $\mathbf{1}$ | 0 | 0 | 1 | 0 | 1 | $\mathbf{3}$ |
| $0915-0930$ | 4 | 0 | 0 | 0 | 0 | 0 | $\mathbf{4}$ |
| Per End | $\mathbf{3 2}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{6 0}$ | $\mathbf{0}$ | $\mathbf{6 0}$ | $\mathbf{1 5 2}$ |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | I | R | $\underline{\text { L }}$ | R | $\underline{L}$ | I | TOT |
| 0700-0715 | 58 | 0 | 0 | 13 | 0 | 98 | 169 |
| 0715-0730 | 79 | 0 | 0 | 11 | 0 | 133 | 223 |
| 0730-0745 | 104 | 0 | 0 | 15 | 0 | 149 | 268 |
| 0745-0800 | 79 | 0 | 0 | 15 | 0 | 156 | 250 |
| 0800-0815 | 82 | 0 | 0 | 5 | 0 | 144 | 231 |
| 0815-0830 | 73 | 0 | 0 | 9 | 0 | 119 | 201 |
| 0830-0845 | 41 | 0 | 0 | 4 | 0 | 68 | 113 |
| 0845-0900 | 64 | 0 | 0 | 2 | 0 | 74 | 140 |
| 0900-0915 | 46 | 0 | 0 | 2 | 0 | 50 | 98 |
| 0915-0930 | 58 | 0 | 1 | 1 | 0 | 55 | 115 |
| Per End | 684 | 0 | 1 | 77 | 0 | 1046 | 1808 |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Per | T | R | $\underline{L}$ | R | L | I | TOT |
| 0700-0800 | 308 | 0 | 0 | 14 | 0 | 496 | 818 |
| 0715-0815 | 331 | 0 | 0 | 14 | 0 | 550 | 895 |
| 0730-0830 | 320 | 0 | 0 | 14 | 0 | 538 | 872 |
| 0745-0845 | 261 | 0 | 0 | 8 | 0 | 462 | 731 |
| 0800-0900 | 245 | 0 | 0 | 1 | 0 | 386 | 632 |
| 0815-0915 | 212 | 0 | 0 | 2 | 0 | 296 | 510 |
| 0830-0930 | 200 | 0 | 1 | 3 | 0 | 241 | 445 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | $\underline{\text { I }}$ | R | $\underline{L}$ | R | $\underline{L}$ | I | TOT |
| 0700-0800 | 12 | 0 | 0 | 40 | 0 | 40 | 92 |
| 0715-0815 | 13 | 0 | 0 | 32 | 0 | 32 | 77 |
| 0730-0830 | 18 | 0 | 0 | 30 | 0 | 30 | 78 |
| 0745-0845 | 14 | 0 | 0 | 25 | 0 | 25 | 64 |
| 0800-0900 | 15 | 0 | 0 | 19 | 0 | 19 | 53 |
| 0815-0915 | 12 | 0 | 0 | 15 | 0 | 15 | 42 |
| 0830-0930 | 9 | 0 | 0 | 6 | 0 | 6 | 21 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | I | R | $\underline{L}$ | R | L | I | TOT |
| 0700-0800 | 320 | 0 | 0 | 54 | 0 | 536 | 910 |
| 0715-0815 | 344 | 0 | 0 | 46 | 0 | 582 | 972 |
| 0730-0830 | 338 | 0 | 0 | 44 | 0 | 568 | 950 |
| 0745-0845 | 275 | 0 | 0 | 33 | 0 | 487 | 795 |
| 0800-0900 | 260 | 0 | 0 | 20 | 0 | 405 | 685 |
| 0815-0915 | 224 | 0 | 0 | 17 | 0 | 311 | 552 |
| 0830-0930 | 209 | 0 | 1 | 9 | 0 | 247 | 466 |





## East Mine Access

## TC-04-Signals

|  | PEDS | WEST | SOUTH | EAST |
| :---: | :---: | :---: | :---: | :---: |


| PEDS | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Appin Rd | East Mine | Appin Rd | TOT |
| $0700-0800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0715-0815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0730-0830$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0745-0845$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0800-0900$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0815-0915$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0830-0930$ | 0 | 0 | 0 | $\mathbf{0}$ |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | I | R | $\underline{L}$ | R | $\underline{L}$ | I | TOT |
| 1600-1615 | 143 | 0 | 0 | 4 | 0 | 88 | 235 |
| 1615-1630 | 121 | 0 | 0 | 3 | 0 | 81 | 205 |
| 1630-1645 | 102 | 0 | 0 | 2 | 0 | 101 | 205 |
| 1645-1700 | 133 | 0 | 0 | 1 | 0 | 107 | 241 |
| 1700-1715 | 125 | 0 | 0 | 1 | 0 | 77 | 203 |
| 1715-1730 | 99 | 0 | 0 | 1 | 0 | 89 | 189 |
| 1730-1745 | 131 | 0 | 0 | 0 | 0 | 81 | 212 |
| 1745-1800 | 104 | 0 | 0 | 0 | 0 | 76 | 180 |
| 1800-1815 | 91 | 0 | 0 | 0 | 0 | 79 | 170 |
| 1815-1830 | 117 | 0 | 0 | 0 | 0 | 53 | 170 |
| Per End | 1166 | 0 | 0 | 12 | 0 | 832 | 2010 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Time Per | I | R | $\underline{\text { L }}$ | R | $\underline{\text { L }}$ | I | TOT |
| 1600-1615 | 1 | 0 | 0 | 5 | 0 | 5 | 11 |
| 1615-1630 | 2 | 0 | 0 | 4 | 0 | 1 | 7 |
| 1630-1645 | 1 | 0 | 0 | 6 | 0 | 2 | 9 |
| 1645-1700 | 1 | 0 | 0 | 1 | 0 | 1 | 3 |
| 1700-1715 | 0 | 0 | 0 | 0 | 0 | 2 | 2 |
| 1715-1730 | 1 | 0 | 0 | 0 | 0 | 1 | 2 |
| 1730-1745 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 1745-1800 | 3 | 0 | 0 | 0 | 0 | 1 | 4 |
| 1800-1815 | 1 | 0 | 0 | 0 | 0 | 2 | 3 |
| 1815-1830 | 0 | 0 | 0 | 1 | 0 | 2 | 3 |
| Per End | 10 | 0 | 0 | 17 | 0 | 17 | 44 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd | East Mine |  | Appin Rd |  |  |  |  |
| Time Per | $\underline{\mathbf{I}}$ | $\underline{\mathbf{R}}$ | $\underline{\underline{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\underline{L}}$ | $\underline{\mathrm{I}}$ | TOT |
| $1600-1615$ | 144 | 0 | 0 | 9 | 0 | 93 | $\mathbf{2 4 6}$ |
| $1615-1630$ | 123 | 0 | 0 | 7 | 0 | 82 | $\mathbf{2 1 2}$ |
| $1630-1645$ | 103 | 0 | 0 | 8 | 0 | 103 | $\mathbf{2 1 4}$ |
| $1645-1700$ | 134 | 0 | 0 | 2 | 0 | 108 | $\mathbf{2 4 4}$ |
| $1700-1715$ | 125 | 0 | 0 | 1 | 0 | 79 | $\mathbf{2 0 5}$ |
| $1715-1730$ | 100 | 0 | 0 | 1 | 0 | 90 | $\mathbf{1 9 1}$ |
| $1730-1745$ | 131 | 0 | 0 | 0 | 0 | 81 | $\mathbf{2 1 2}$ |
| $1745-1800$ | 107 | 0 | 0 | 0 | 0 | 77 | $\mathbf{1 8 4}$ |
| $1800-1815$ | 92 | 0 | 0 | 0 | 0 | 81 | $\mathbf{1 7 3}$ |
| $1815-1830$ | 117 | 0 | 0 | 1 | 0 | 55 | $\mathbf{1 7 3}$ |
| Per End | $\mathbf{1 1 7 6}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{2 9}$ | $\mathbf{0}$ | $\mathbf{8 4 9}$ | $\mathbf{2 0 5 4}$ |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | $\underline{\mathbf{T}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{T}}$ | TOT |
| $1600-1700$ | 499 | 0 | 0 | 10 | 0 | 377 | $\mathbf{8 8 6}$ |
| $1615-1715$ | 481 | 0 | 0 | 7 | 0 | 366 | $\mathbf{8 5 4}$ |
| $1630-1730$ | 459 | 0 | 0 | 5 | 0 | 374 | $\mathbf{8 3 8}$ |
| $1645-1745$ | 488 | 0 | 0 | 3 | 0 | 354 | $\mathbf{8 4 5}$ |
| $1700-1800$ | 459 | 0 | 0 | 2 | 0 | 323 | $\mathbf{7 8 4}$ |
| $1715-1815$ | 425 | 0 | 0 | 1 | 0 | 325 | $\mathbf{7 5 1}$ |
| $1730-1830$ | 443 | 0 | 0 | 0 | 0 | 289 | $\mathbf{7 3 2}$ |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | $\underline{\text { I }}$ | R | $\underline{L}$ | R | $\underline{\text { L }}$ | $\underline{\text { I }}$ | TOT |
| 1600-1700 | 5 | 0 | 0 | 16 | 0 | 9 | 30 |
| 1615-1715 | 4 | 0 | 0 | 11 | 0 | 6 | 21 |
| 1630-1730 | 3 | 0 | 0 | 7 | 0 | 6 | 16 |
| 1645-1745 | 2 | 0 | 0 | 1 | 0 | 4 | 7 |
| 1700-1800 | 4 | 0 | 0 | 0 | 0 | 4 | 8 |
| 1715-1815 | 5 | 0 | 0 | 0 | 0 | 4 | 9 |
| 1730-1830 | 4 | 0 | 0 | 1 | 0 | 5 | 10 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | East Mine |  | Appin Rd |  |  |
| Peak Per | $\underline{\text { I }}$ | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | $\underline{\text { L }}$ | I | TOT |
| 1600-1700 | 504 | 0 | 0 | 26 | 0 | 386 | 916 |
| 1615-1715 | 485 | 0 | 0 | 18 | 0 | 372 | 875 |
| 1630-1730 | 462 | 0 | 0 | 12 | 0 | 380 | 854 |
| 1645-1745 | 490 | 0 | 0 | 4 | 0 | 358 | 852 |
| 1700-1800 | 463 | 0 | 0 | 2 | 0 | 327 | 792 |
| 1715-1815 | 430 | 0 | 0 | 1 | 0 | 329 | 760 |
| 1730-1830 | 447 | 0 | 0 | 1 | 0 | 294 | 742 |




PM PEAK
1600-1700


East Mine Access
East Mine Access

## R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019

Intersection Details
Obtained via satellite
May be incorrect


East Mine Access

To


Appin Rd \& West Cliff Mine Access

R.O.A.R. DATA Pty. Ltd.

www.roardata.com.au
R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849
Mobile. 0418239019

Client
Job No/Name Day/Date

Traffix
2654 Wollongong Area Traffic Surveys
Thursday 9th April 2009

| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Appin Rd | West Cliff | Appin Rd | TOT |
| $0700-0715$ |  |  |  | 0 |
| $0715-0730$ |  |  |  | 0 |
| $0730-0745$ |  | NOT |  | $\mathbf{0}$ |
| $0745-0800$ |  | REQUIRED |  | 0 |
| $0800-0815$ |  |  |  | $\mathbf{0}$ |
| $0815-0830$ |  |  |  | 0 |
| $0830-0845$ |  |  |  | $\mathbf{0}$ |
| $0845-0900$ |  |  |  | 0 |
| $0900-0915$ |  |  |  | $\mathbf{0}$ |
| $0915-0930$ |  |  | $\mathbf{0}$ | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ |  |  |


| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Peak Per | Appin Rd | West Cliff | Appin $\boldsymbol{R} \boldsymbol{d}$ |
| TOT |  |  |  |  |
| $0700-0800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0715-0815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0730-0830$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0745-0845$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0800-0900$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0815-0915$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0830-0930$ | 0 | 0 | 0 | $\mathbf{0}$ |


| PEAK HR | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- |


| Lights | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Time Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | R | I | TOT |
| 0700-0715 | 58 | 4 | 0 | 4 | 7 | 72 | 145 |
| 0715-0730 | 73 | 7 | 0 | 6 | 9 | 141 | 236 |
| 0730-0745 | 88 | 0 | 0 | 5 | 0 | 148 | 241 |
| 0745-0800 | 84 | 1 | 4 | 4 | 2 | 154 | 249 |
| 0800-0815 | 86 | 1 | 1 | 1 | 3 | 145 | 237 |
| 0815-0830 | 54 | 4 | 1 | 19 | 5 | 94 | 177 |
| 0830-0845 | 48 | 2 | 4 | 5 | 4 | 63 | 126 |
| 0845-0900 | 59 | 1 | 2 | 5 | 8 | 67 | 142 |
| 0900-0915 | 61 | 2 | 2 | 4 | 6 | 65 | 140 |
| 0915-0930 | 40 | 1 | 2 | 0 | 3 | 55 | 101 |
| Per End | 651 | 23 | 16 | 53 | 47 | 1004 | 1794 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Time Per | I | $\underline{\text { L }}$ | $\underline{\text { R }}$ | $\underline{\square}$ | R | I | TOT |
| 0700-0715 | 0 | 11 | 11 | 6 | 5 | 0 | 33 |
| 0715-0730 | 3 | 15 | 10 | 7 | 7 | 0 | 42 |
| 0730-0745 | 1 | 8 | 6 | 4 | 11 | 3 | 33 |
| 0745-0800 | 1 | 8 | 8 | 11 | 7 | 1 | 36 |
| 0800-0815 | 5 | 6 | 7 | 10 | 4 | 4 | 36 |
| 0815-0830 | 2 | 8 | 7 | 6 | 7 | 3 | 33 |
| 0830-0845 | 2 | 11 | 0 | 8 | 8 | 1 | 30 |
| 0845-0900 | 1 | 2 | 1 | 17 | 4 | 3 | 28 |
| 0900-0915 | 1 | 3 | 0 | 13 | 6 | 2 | 25 |
| 0915-0930 | 1 | 3 | 0 | 11 | 2 | 3 | 20 |
| Per End | 17 | 75 | 50 | 93 | 61 | 20 | 316 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Time Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 0700-0715 | 58 | 15 | 11 | 10 | 12 | 72 | 178 |
| 0715-0730 | 76 | 22 | 10 | 13 | 16 | 141 | 278 |
| 0730-0745 | 89 | 8 | 6 | 9 | 11 | 151 | 274 |
| 0745-0800 | 85 | 9 | 12 | 15 | 9 | 155 | 285 |
| 0800-0815 | 91 | 7 | 8 | 11 | 7 | 149 | 273 |
| 0815-0830 | 56 | 12 | 8 | 25 | 12 | 97 | 210 |
| 0830-0845 | 50 | 13 | 4 | 13 | 12 | 64 | 156 |
| 0845-0900 | 60 | 3 | 3 | 22 | 12 | 70 | 170 |
| 0900-0915 | 62 | 5 | 2 | 17 | 12 | 67 | 165 |
| 0915-0930 | 41 | 4 | 2 | 11 | 5 | 58 | 121 |
| Per End | 668 | 98 | 66 | 146 | 108 | 1024 | 2110 |


| Lights | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | $\underline{\text { L }}$ | R | I | TOT |
| 0700-0800 | 303 | 12 | 4 | 19 | 18 | 515 | 871 |
| 0715-0815 | 331 | 9 | 5 | 16 | 14 | 588 | 963 |
| 0730-0830 | 312 | 6 | 6 | 29 | 10 | 541 | 904 |
| 0745-0845 | 272 | 8 | 10 | 29 | 14 | 456 | 789 |
| 0800-0900 | 247 | 8 | 8 | 30 | 20 | 369 | 682 |
| 0815-0915 | 222 | 9 | 9 | 33 | 23 | 289 | 585 |
| 0830-0930 | 208 | 6 | 10 | 14 | 21 | 250 | 509 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | L | $\underline{R}$ | T | TOT |
| 0700-0800 | 5 | 42 | 35 | 28 | 30 | 4 | 144 |
| 0715-0815 | 10 | 37 | 31 | 32 | 29 | 8 | 147 |
| 0730-0830 | 9 | 30 | 28 | 31 | 29 | 11 | 138 |
| 0745-0845 | 10 | 33 | 22 | 35 | 26 | 9 | 135 |
| 0800-0900 | 10 | 27 | 15 | 41 | 23 | 11 | 127 |
| 0815-0915 | 6 | 24 | 8 | 44 | 25 | 9 | 116 |
| 0830-0930 | 5 | 19 | 1 | 49 | 20 | 9 | 103 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Per | I | L | R | L | R | I | TOT |
| 0700-0800 | 308 | 54 | 39 | 47 | 48 | 519 | 1015 |
| 0715-0815 | 341 | 46 | 36 | 48 | 43 | 596 | 1110 |
| 0730-0830 | 321 | 36 | 34 | 60 | 39 | 552 | 1042 |
| 0745-0845 | 282 | 41 | 32 | 64 | 40 | 465 | 924 |
| 0800-0900 | 257 | 35 | 23 | 71 | 43 | 380 | 809 |
| 0815-0915 | 228 | 33 | 17 | 77 | 48 | 298 | 701 |
| 0830-0930 | 213 | 25 | 11 | 63 | 41 | 259 | 612 |




West Cliff Mine Access


# Reliable, Original \& Authentic Results <br> Ph.88196847, Fax 88196849. <br> Mobile. 0418239019 <br> Client <br> Job No/Name <br> Day/Date <br> : Traffix <br> : 2654 Wollongong Area Traffic Surveys : Thursday 9th April 2009 

TC-05

| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Appin Rd | West Cliff | Appin Rd | TOT |
| $1600-1615$ |  |  |  | $\mathbf{0}$ |
| $1615-1630$ |  |  |  | $\mathbf{0}$ |
| $1630-1645$ |  | NOT |  | $\mathbf{0}$ |
| $1645-1700$ |  | REQUIRED |  | $\mathbf{0}$ |
| $1700-1715$ |  |  |  | $\mathbf{0}$ |
| $1715-1730$ |  |  |  | $\mathbf{0}$ |
| $1730-1745$ |  |  |  | $\mathbf{0}$ |
| $1745-1800$ |  |  |  | $\mathbf{0}$ |
| $1800-1815$ |  |  |  | $\mathbf{0}$ |
| $1815-1830$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Appin Rd | West Cliff | Appin Rd | TOT |
| $1600-1700$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1615-1715$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1630-1730$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1645-1745$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1700-1800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1715-1815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1730-1830$ | 0 | 0 | 0 | $\mathbf{0}$ |


| PEAKHR | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- |


| Lights | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Time Per | I | $\underline{L}$ | R | $\underline{L}$ | $\underline{\text { R }}$ | I | TOT |
| 1600-1615 | 169 | 3 | 1 | 4 | 0 | 81 | 258 |
| 1615-1630 | 115 | 0 | 2 | 6 | 1 | 68 | 192 |
| 1630-1645 | 97 | 0 | 14 | 14 | 0 | 84 | 209 |
| 1645-1700 | 103 | 0 | 9 | 21 | 1 | 89 | 223 |
| 1700-1715 | 128 | 1 | 1 | 8 | 0 | 75 | 213 |
| 1715-1730 | 101 | 1 | 0 | 4 | 2 | 84 | 192 |
| 1730-1745 | 154 | 0 | 3 | 4 | 2 | 89 | 252 |
| 1745-1800 | 102 | 0 | 0 | 1 | 2 | 74 | 179 |
| 1800-1815 | 106 | 0 | 1 | 3 | 1 | 75 | 186 |
| 1815-1830 | 91 | 4 | 2 | 6 | 5 | 61 | 169 |
| Per End | 1166 | 9 | 33 | 71 | 14 | 780 | 2073 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Time Per | I | $\underline{L}$ | R | $\underline{\text { L }}$ | R | I | TOT |
| 1600-1615 | 1 | 6 | 8 | 8 | 7 | 2 | 32 |
| 1615-1630 | 0 | 3 | 6 | 4 | 1 | 0 | 14 |
| 1630-1645 | 0 | 7 | 3 | 4 | 1 | 3 | 18 |
| 1645-1700 | 1 | 4 | 0 | 7 | 3 | 1 | 16 |
| 1700-1715 | 0 | 1 | 1 | 11 | 2 | 1 | 16 |
| 1715-1730 | 0 | 0 | 0 | 7 | 6 | 1 | 14 |
| 1730-1745 | 1 | 0 | 0 | 5 | 4 | 0 | 10 |
| 1745-1800 | 3 | 0 | 0 | 3 | 4 | 1 | 11 |
| 1800-1815 | 0 | 0 | 1 | 5 | 3 | 2 | 11 |
| 1815-1830 | 0 | 1 | 1 | 6 | 11 | 1 | 20 |
| Per End | 6 | 22 | 20 | 60 | 42 | 12 | 162 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Time Per | I | $\underline{\square}$ | R | $\underline{L}$ | R | I | TOT |
| 1600-1615 | 170 | 9 | 9 | 12 | 7 | 83 | 290 |
| 1615-1630 | 115 | 3 | 8 | 10 | 2 | 68 | 206 |
| 1630-1645 | 97 | 7 | 17 | 18 | 1 | 87 | 227 |
| 1645-1700 | 104 | 4 | 9 | 28 | 4 | 90 | 239 |
| 1700-1715 | 128 | 2 | 2 | 19 | 2 | 76 | 229 |
| 1715-1730 | 101 | 1 | 0 | 11 | 8 | 85 | 206 |
| 1730-1745 | 155 | 0 | 3 | 9 | 6 | 89 | 262 |
| 1745-1800 | 105 | 0 | 0 | 4 | 6 | 75 | 190 |
| 1800-1815 | 106 | 0 | 2 | 8 | 4 | 77 | 197 |
| 1815-1830 | 91 | 5 | 3 | 12 | 16 | 62 | 189 |
| Per End | 1172 | 31 | 53 | 131 | 56 | 792 | 2235 |


| Lights | WEST |  | NORTH |  | EAST |  | TOT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | $\underline{\square}$ | R | I |  |
| 1600-1700 | 484 | 3 | 26 | 45 | 2 | 322 | 882 |
| 1615-1715 | 443 | 1 | 26 | 49 | 2 | 316 | 837 |
| 1630-1730 | 429 | 2 | 24 | 47 | 3 | 332 | 837 |
| 1645-1745 | 486 | 2 | 13 | 37 | 5 | 337 | 880 |
| 1700-1800 | 485 | 2 | 4 | 17 | 6 | 322 | 836 |
| 1715-1815 | 463 | 1 | 4 | 12 | 7 | 322 | 809 |
| 1730-1830 | 453 | 4 | 6 | 14 | 10 | 299 | 786 |


| Heavies | WEST |  | NORTH |  | EAST |  | TOT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | L | R | I |  |
| 1600-1700 | 2 | 20 | 17 | 23 | 12 | 6 | 80 |
| 1615-1715 | 1 | 15 | 10 | 26 | 7 | 5 | 64 |
| 1630-1730 | 1 | 12 | 4 | 29 | 12 | 6 | 64 |
| 1645-1745 | 2 | 5 | 1 | 30 | 15 | 3 | 56 |
| 1700-1800 | 4 | 1 | 1 | 26 | 16 | 3 | 51 |
| 1715-1815 | 4 | 0 | 1 | 20 | 17 | 4 | 46 |
| 1730-1830 | 4 |  | 2 | 19 | 22 | 4 | 52 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | West Cliff |  | Appin Rd |  |  |
| Peak Per | I | $\underline{\square}$ | $\underline{R}$ | $\underline{\text { L }}$ | $\underline{\text { R }}$ | I | TOT |
| 1600-1700 | 486 | 23 | 43 | 68 | 14 | 328 | 962 |
| 1615-1715 | 444 | 16 | 36 | 75 | 9 | 321 | 901 |
| 1630-1730 | 430 | 14 | 28 | 76 | 15 | 338 | 901 |
| 1645-1745 | 488 | 7 | 14 | 67 | 20 | 340 | 936 |
| 1700-1800 | 489 | 3 | 5 | 43 | 22 | 325 | 887 |
| 1715-1815 | 467 | 1 | 5 | 32 | 24 | 326 | 855 |
| 1730-1830 | 457 | 5 | 8 | 33 | 32 | 303 | 838 |

[^5]R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019

Job No/Name
$N$
40
40

## West Cliff Mine Access



## R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019

$$
\text { : Thursday 9th April } 2009
$$

ntersection Details
Obtained via satellite
May be incorrect


Appin Rd

| (27 ) ${ }^{4}$ Reliable, Original \& Authentic Results | PEDS | WEST | SOUTH | EAST |  | PEDS | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| (1) Ph- ${ }_{\text {d }}$ | Time Per | Mt. Ousley Rd | Southern | Mt. Ousley Rd | TOT | Peak Per | Mt. Ousley Rd | Eraomav | Mt. Ousley Rd | TOT |
| Mobile. 0418239019 | 0700-0715 |  |  |  | 0 | 0700-0800 | 0 | 0 | 0 | 0 |
|  | 0715-0730 |  |  |  | 0 | 0715-0815 | 0 | 0 | 0 | 0 |
| Client : Traffix | 0730-0745 |  | NOT |  | 0 | 0730-0830 | 0 | 0 | 0 | 0 |
| Job No/Name : 2654 Wollongong Area Traffic Surveys | 0745-0800 |  | REQUIRED |  | 0 | 0745-0845 | 0 | 0 | 0 | 0 |
| Day/Date : Thursday 9th April 2009 | 0800-0815 |  |  |  | 0 | 0800-0900 | 0 | 0 | 0 | 0 |
|  | 0815-0830 |  |  |  | 0 | 0815-0915 | 0 | 0 | 0 | 0 |
|  | 0830-0845 |  |  |  | 0 | 0830-0930 | 0 | 0 | 0 | 0 |
| TC-06 | 0845-0900 |  |  |  | 0 |  |  |  |  |  |
|  | 0900-0915 |  |  |  | 0 |  |  |  |  |  |
|  | 0915-0930 |  |  |  | 0 |  |  |  |  |  |
|  | Per End | 0 | 0 | 0 | 0 | PEAK HR | 0 | 0 | 0 | 0 |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley Rd |  | Southern |  | Mt. Ousley Rd |  |  |
| Time Per | I | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | TOT |
| 0700-0715 | 50 | 228 | 363 | 0 | 17 | 82 | 740 |
| 0715-0730 | 69 | 225 | 345 | 0 | 21 | 70 | 730 |
| 0730-0745 | 75 | 340 | 359 | 0 | 32 | 64 | 870 |
| 0745-0800 | 80 | 420 | 293 | 0 | 33 | 38 | 864 |
| 0800-0815 | 98 | 471 | 311 | 0 | 23 | 42 | 945 |
| 0815-0830 | 107 | 409 | 247 | 0 | 39 | 48 | 850 |
| 0830-0845 | 92 | 377 | 219 | 0 | 58 | 44 | 790 |
| 0845-0900 | 99 | 355 | 245 | 0 | 48 | 43 | 790 |
| 0900-0915 | 53 | 351 | 207 | 0 | 25 | 42 | 678 |
| 0915-0930 | 64 | 282 | 249 | 0 | 37 | 50 | 682 |
| Per End | 787 | 3458 | 2838 | 0 | 333 | 523 | 7939 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley Rd |  | Southern |  | Mt. Ousley Rd |  |  |
| Time Per | $\underline{\mathbf{I}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | TOT |
| $0700-0715$ | 3 | 32 | 29 | 0 | 0 | 2 | $\mathbf{6 6}$ |
| $0715-0730$ | 1 | 28 | 35 | 0 | 0 | 1 | $\mathbf{6 5}$ |
| $0730-0745$ | 2 | 31 | 51 | 0 | 0 | 0 | $\mathbf{8 4}$ |
| $0745-0800$ | 3 | 29 | 30 | 0 | 0 | 1 | $\mathbf{6 3}$ |
| $0800-0815$ | 3 | 48 | 42 | 0 | 0 | 0 | $\mathbf{9 3}$ |
| $0815-0830$ | 2 | 34 | 47 | 0 | 1 | 3 | $\mathbf{8 7}$ |
| $0830-0845$ | 2 | 35 | 49 | 0 | 3 | 2 | $\mathbf{9 1}$ |
| $0845-0900$ | 4 | 42 | 38 | 0 | 4 | 0 | $\mathbf{8 8}$ |
| $0900-0915$ | 2 | 42 | 37 | 0 | 0 | 2 | $\mathbf{8 3}$ |
| $0915-0930$ | 2 | 31 | 50 | 0 | 0 | 1 | $\mathbf{8 4}$ |
| Per End | $\mathbf{2 4}$ | $\mathbf{3 5 2}$ | $\mathbf{4 0 8}$ | $\mathbf{0}$ | $\mathbf{8}$ | $\mathbf{1 2}$ | $\mathbf{8 0 4}$ |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley Rd |  | Southern |  | Mt. Ousley Rd |  |  |
| Time Per | T | $\underline{R}$ | $\underline{L}$ | R | $\underline{L}$ | R | TOT |
| 0700-0715 | 53 | 260 | 392 | 0 | 17 | 84 | 806 |
| 0715-0730 | 70 | 253 | 380 | 0 | 21 | 71 | 795 |
| 0730-0745 | 77 | 371 | 410 | 0 | 32 | 64 | 954 |
| 0745-0800 | 83 | 449 | 323 | 0 | 33 | 39 | 927 |
| 0800-0815 | 101 | 519 | 353 | 0 | 23 | 42 | 1038 |
| 0815-0830 | 109 | 443 | 294 | 0 | 40 | 51 | 937 |
| 0830-0845 | 94 | 412 | 268 | 0 | 61 | 46 | 881 |
| 0845-0900 | 103 | 397 | 283 | 0 | 52 | 43 | 878 |
| 0900-0915 | 55 | 393 | 244 | 0 | 25 | 44 | 761 |
| 0915-0930 | 66 | 313 | 299 | 0 | 37 | 51 | 766 |
| Per End | 811 | 3810 | 3246 | 0 | 341 | 535 | 8743 |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley Rd |  | Southern |  | Mt. Ousley Rd |  |  |
| Peak Per | T | R | $\underline{L}$ | R | $\underline{L}$ | R | TOT |
| 0700-0800 | 274 | 1213 | 1360 | 0 | 103 | 254 | 3204 |
| 0715-0815 | 322 | 1456 | 1308 | 0 | 109 | 214 | 3409 |
| 0730-0830 | 360 | 1640 | 1210 | 0 | 127 | 192 | 3529 |
| 0745-0845 | 377 | 1677 | 1070 | 0 | 153 | 172 | 3449 |
| 0800-0900 | 396 | 1612 | 1022 | 0 | 168 | 177 | 3375 |
| 0815-0915 | 351 | 1492 | 918 | 0 | 170 | 177 | 3108 |
| 0830-0930 | 308 | 1365 | 920 | 0 | 168 | 179 | 2940 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley Rd |  | Southern |  | Mt. Ousley Rd |  |  |
| Peak Per | $\underline{\mathbf{T}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | TOT |
| $0700-0800$ | 9 | 120 | 145 | 0 | 0 | 4 | $\mathbf{2 7 8}$ |
| $0715-0815$ | 9 | 136 | 158 | 0 | 0 | 2 | $\mathbf{3 0 5}$ |
| $\mathbf{0 7 3 0 - 0 8 3 0}$ | 10 | 142 | 170 | 0 | 1 | 4 | $\mathbf{3 2 7}$ |
| $0745-0845$ | 10 | 146 | 168 | 0 | 4 | 6 | $\mathbf{3 3 4}$ |
| $0800-0900$ | 11 | 159 | 176 | 0 | 8 | 5 | $\mathbf{3 5 9}$ |
| $0815-0915$ | 10 | 153 | 171 | 0 | 8 | 7 | $\mathbf{3 4 9}$ |
| $0830-0930$ | 10 | 150 | 174 | 0 | 7 | 5 | $\mathbf{3 4 6}$ |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley Rd |  | Southern |  | Mt. Ousley Rd |  |  |
| Peak Per | I | $\underline{R}$ | $\underline{L}$ | R | $\underline{L}$ | R | TOT |
| 0700-0800 | 283 | 1333 | 1505 | 0 | 103 | 258 | 3482 |
| 0715-0815 | 331 | 1592 | 1466 | 0 | 109 | 216 | 3714 |
| 0730-0830 | 370 | 1782 | 1380 | 0 | 128 | 196 | 3856 |
| 0745-0845 | 387 | 1823 | 1238 | 0 | 157 | 178 | 3783 |
| 0800-0900 | 407 | 1771 | 1198 | 0 | 176 | 182 | 3734 |
| 0815-0915 | 361 | 1645 | 1089 | 0 | 178 | 184 | 3457 |
| 0830-0930 | 318 | 1515 | 1094 | 0 | 175 | 184 | 3286 |



Southern Freeway

| [4) Reliable, Original \& Authentic Results | PEDS | WEST | SOUTH | EAST |  | PEDS | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| L-A Ph.88196847, Fax 88196849. | Time Per | Mt. Ousley | Southern | Mt. Ousley | TOT | Peak Per | Mt. Ousley | Southern | Mt. Ousley | TOT |
| Mobile. 0418239019 | 1600-1615 |  |  |  | 0 | 0700-0800 | 0 | 0 | 0 | 0 |
|  | 1615-1630 |  |  |  | 0 | 0715-0815 | 0 | 0 | 0 | 0 |
| Client : Traffix | 1630-1645 |  | NOT |  | 0 | 0730-0830 | 0 | 0 | 0 | 0 |
| Job No/Name : 2654 Wollongong Area Traffic Surveys | 1645-1700 |  | REQUIRED |  | 0 | 0745-0845 | 0 | 0 | 0 | 0 |
| Day/Date : Thursday 9th April 2009 | 1700-1715 |  |  |  | 0 | 0800-0900 | 0 | 0 | 0 | 0 |
|  | 1715-1730 |  |  |  | 0 | 0815-0915 | 0 | 0 | 0 | 0 |
|  | 1730-1745 |  |  |  | 0 | 0830-0930 | 0 | 0 | 0 | 0 |
| TC-06 | 1745-1800 |  |  |  | 0 |  |  |  |  |  |
|  | 1800-1815 |  |  |  | 0 |  |  |  |  |  |
|  | 1815-1830 |  |  |  | 0 |  |  |  |  |  |
|  | Per End | 0 | 0 | 0 | 0 | PEAK HR | 0 | 0 | 0 | 0 |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley |  | Southern |  | Mt. Ousley |  |  |
| Time Per | I | R | $\underline{L}$ | R | $\underline{L}$ | R | TOT |
| 1600-1615 | 71 | 576 | 405 | 0 | 48 | 17 | 1117 |
| 1615-1630 | 87 | 548 | 384 | 0 | 55 | 32 | 1106 |
| 1630-1645 | 58 | 477 | 374 | 0 | 54 | 36 | 999 |
| 1645-1700 | 86 | 552 | 358 | 0 | 45 | 47 | 1088 |
| 1700-1715 | 55 | 529 | 348 | 0 | 54 | 36 | 1022 |
| 1715-1730 | 71 | 519 | 342 | 0 | 48 | 20 | 1000 |
| 1730-1745 | 75 | 501 | 342 | 0 | 69 | 49 | 1036 |
| 1745-1800 | 75 | 512 | 287 | 0 | 28 | 36 | 938 |
| 1800-1815 | 72 | 520 | 247 | 0 | 32 | 31 | 902 |
| 1815-1830 | 68 | 523 | 201 | 0 | 38 | 32 | 862 |
| Per End | 718 | 5257 | 3288 | 0 | 471 | 336 | 10070 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley |  | Southern |  | Mt. Ousley |  |  |
| Time Per | $\underline{\underline{I}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | TOT |
| $1600-1615$ | 0 | 22 | 17 | 0 | 0 | 0 | $\mathbf{3 9}$ |
| $1615-1630$ | 2 | 20 | 16 | 0 | 0 | 0 | $\mathbf{3 8}$ |
| $1630-1645$ | 0 | 16 | 13 | 0 | 0 | 1 | $\mathbf{3 0}$ |
| $1645-1700$ | 0 | 10 | 11 | 0 | 0 | 0 | $\mathbf{2 1}$ |
| $1700-1715$ | 1 | 14 | 16 | 0 | 0 | 0 | $\mathbf{3 1}$ |
| $1715-1730$ | 0 | 21 | 14 | 0 | 0 | 0 | $\mathbf{3 5}$ |
| $1730-1745$ | 1 | 24 | 8 | 0 | 1 | 0 | $\mathbf{3 4}$ |
| $1745-1800$ | 1 | 12 | 15 | 0 | 0 | 0 | $\mathbf{2 8}$ |
| $1800-1815$ | 0 | 15 | 14 | 0 | 0 | 0 | $\mathbf{2 9}$ |
| $1815-1830$ | 1 | 17 | 15 | 0 | 0 | 0 | $\mathbf{3 3}$ |
| Per End | $\mathbf{6}$ | $\mathbf{1 7 1}$ | $\mathbf{1 3 9}$ | $\mathbf{0}$ | $\mathbf{1}$ | $\mathbf{1}$ | $\mathbf{3 1 8}$ |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley |  | Southern |  | Mt. Ousley |  |  |
| Time Per | I | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | TOT |
| 1600-1615 | 71 | 598 | 422 | 0 | 48 | 17 | 1156 |
| 1615-1630 | 89 | 568 | 400 | 0 | 55 | 32 | 1144 |
| 1630-1645 | 58 | 493 | 387 | 0 | 54 | 37 | 1029 |
| 1645-1700 | 86 | 562 | 369 | 0 | 45 | 47 | 1109 |
| 1700-1715 | 56 | 543 | 364 | 0 | 54 | 36 | 1053 |
| 1715-1730 | 71 | 540 | 356 | 0 | 48 | 20 | 1035 |
| 1730-1745 | 76 | 525 | 350 | 0 | 70 | 49 | 1070 |
| 1745-1800 | 76 | 524 | 302 | 0 | 28 | 36 | 966 |
| 1800-1815 | 72 | 535 | 261 | 0 | 32 | 31 | 931 |
| 1815-1830 | 69 | 540 | 216 | 0 | 38 | 32 | 895 |
| Per End | 724 | 5428 | 3427 | 0 | 472 | 337 | 10388 |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley |  | Southern |  | Mt. Ousley |  |  |
| Peak Per | I | R | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | TOT |
| 1600-1700 | 302 | 2153 | 1521 | 0 | 202 | 132 | 4310 |
| 1615-1715 | 286 | 2106 | 1464 | 0 | 208 | 151 | 4215 |
| 1630-1730 | 270 | 2077 | 1422 | 0 | 201 | 139 | 4109 |
| 1645-1745 | 287 | 2101 | 1390 | 0 | 216 | 152 | 4146 |
| 1700-1800 | 276 | 2061 | 1319 | 0 | 199 | 141 | 3996 |
| 1715-1815 | 293 | 2052 | 1218 | 0 | 177 | 136 | 3876 |
| 1730-1830 | 290 | 2056 | 1077 | 0 | 167 | 148 | 3738 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley |  | Southern |  | Mt. Ousley |  |  |
| Peak Per | I | R | L | R | $\underline{L}$ | R | TOT |
| 1600-1700 | 2 | 68 | 57 | 0 | 0 | 1 | 128 |
| 1615-1715 | 3 | 60 | 56 | 0 | 0 | 1 | 120 |
| 1630-1730 | 1 | 61 | 54 | 0 | 0 | 1 | 117 |
| 1645-1745 | 2 | 69 | 49 | 0 | 1 | 0 | 121 |
| 1700-1800 | 3 | 71 | 53 | 0 | 1 | 0 | 128 |
| 1715-1815 | 2 | 72 | 51 | 0 | 1 | 0 | 126 |
| 1730-1830 | 3 | 68 | 52 | 0 | 1 | 0 | 124 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Mt. Ousley |  | Southern |  | Mt. Ousley |  |  |
| Peak Per | $\underline{\mathbf{T}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | TOT |
| $1600-1700$ | 304 | 2221 | 1578 | 0 | 202 | 133 | 4438 |
| $1615-1715$ | 289 | 2166 | 1520 | 0 | 208 | 152 | 4335 |
| $1630-1730$ | 271 | 2138 | 1476 | 0 | 201 | 140 | 4226 |
| $1645-1745$ | 289 | 2170 | 1439 | 0 | 217 | 152 | 4267 |
| $1700-1800$ | 279 | 2132 | 1372 | 0 | 200 | 141 | 4124 |
| $1715-1815$ | 295 | 2124 | 1269 | 0 | 178 | 136 | 4002 |
| $1730-1830$ | 293 | 2124 | 1129 | 0 | 168 | 148 | $\mathbf{3 8 6 2}$ |




## R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019
$\square$
TC-06


Springhill Rd \& Bluescope

To
Tim Lewis at Traffix your results for

## Wollongong Area Traffic Counts

supplied by
R.O.A.R. DATA Pty. Ltd.
www.roardata.com.au

| Lights | R.O.A.R. DATA <br> Reliable, Original \& Authentic Results Ph.88196847, Fax 88196849, Mob.0418-239019 |  |  |  |  |  |  |  |  | TC-12 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | NORTH |  |  | WEST |  |  | SOUTH |  |  | EAST |  |  |  |
|  | Springhill Rd |  |  | Bluescope Steel |  |  | Springhill Rd |  |  | Colourbond |  |  |  |
| Time Per | $\underline{L}$ | I | $\underline{R}$ | $\underline{L}$ | I | $\underline{\text { R }}$ | $\underline{L}$ | I | $\underline{R}$ | $\underline{L}$ | I | $\underline{R}$ | TOT |
| 1600-1615 |  | 456 | 2 | 9 | 0 | 11 | 0 | 394 |  |  |  |  | 872 |
| 1615-1630 |  | 462 | 3 | 12 | 0 | 13 | 1 | 448 |  |  |  |  | 939 |
| 1630-1645 |  | 447 | 2 | 6 | 0 | 11 | 3 | 419 |  |  |  |  | 888 |
| 1645-1700 |  | 511 | 3 | 18 | 1 | 11 | 3 | 378 |  |  |  |  | 925 |
| 1700-1715 |  | 541 | 2 | 21 | 0 | 14 | 0 | 392 |  |  |  |  | 970 |
| 1715-1730 |  | 546 | 0 | 9 | 0 | 4 | 4 | 410 |  |  |  |  | 973 |
| 1730-1745 |  | 522 | 2 | 19 | 0 | 14 | 1 | 382 |  |  |  |  | 940 |
| 1745-1800 |  | 457 | 0 | 9 | 0 | 3 | 4 | 352 |  |  |  |  | 825 |
| 1800-1815 |  | 372 | 5 | 1 | 0 | 1 | 1 | 331 |  |  |  |  | 711 |
| 1815-1830 |  | 321 | 3 | 3 | 0 | 3 | 4 | 308 |  |  |  |  | 642 |
| Period End | 0 | 4635 | 22 | 107 | 1 | 85 | 21 | 3814 | 0 | 0 | 0 | 0 | 8685 |



\section*{| PEAK HOUR | 0 | 2120 | 7 | 67 | 1 | 43 | 8 | 1562 | 0 | 0 | 0 | 0 | 3808 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |}



| Combined | NORTH |  |  | WEST |  |  | SOUTH |  |  | EAST |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Springhill Rd |  |  | Bluescope Steel |  |  | Springhill Rd |  |  | Colourbond |  |  |  |
| Peak Per | $\underline{L}$ | I | R | $\underline{L}$ | I | $\underline{R}$ | $\underline{L}$ | I | $\underline{R}$ | $\underline{L}$ | I | R | 101 |
| 1600-1700 | 0 | 1918 | 15 | 56 | 1 | 47 | 11 | 1663 | 0 | 0 | 0 | 0 | 3711 |
| 1615-1715 | 0 | 1999 | 14 | 68 | 1 | 50 | 10 | 1662 | 0 | 0 | 0 | 0 | 3804 |
| 1630-1730 | 0 | 2082 | 10 | 62 | 1 | 42 | 12 | 1624 | 0 | 0 | 0 | 0 | 3833 |
| 1645-1745 | 0 | 2156 | 13 | 74 | 1 | 46 | 10 | 1586 | 0 | 0 | 0 | 0 | 3886 |
| 1700-1800 | 0 | 2100 | 11 | 67 | 0 | 39 | 12 | 1561 | 0 | 0 | 0 | 0 | 3790 |
| 1715-1815 | 0 | 1927 | 20 | 44 | 0 | 25 | 12 | 1497 | 0 | 0 | 0 | 0 | 3525 |
| 1730-1830 | 0 | 1696 | 24 | 44 | 0 | 23 | 12 | 1388 | 0 | 0 | 0 | 0 | 3187 |


| PEAK HOUR | 0 | 2156 | 13 | 74 | 1 | 46 | 10 | 1586 | 0 | 0 | 0 | 0 | 3886 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

## R.O.A.R DATA

Reliable, Original \& Authentic Results $\quad$ Springhill Rd
Ph.88196847, Fax 88196849, Mob.0418-239019
Client
Job No/Nam
Day/Date

Traffix
: 2654 Wollongong Area Traffic Surveys
: Thursday 9th April 2009

| Peds | NORTH | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Springhill Rd | Bluescope Steel | Springhill Rd | Colourbond |  |
| Time Per | UNCLASSIFIED | UNCLASSIFIED | UNCLASSIFIED | UNCLASSIFIED |  |
| 1600-1615 |  |  |  |  | 0 |
| 1615-1630 |  |  |  |  | 0 |
| 1630-1645 |  | NOT | REQUIRED |  | 0 |
| 1645-1700 |  |  |  |  | 0 |
| 1700-1715 |  |  |  |  | 0 |
| 1715-1730 |  |  |  |  | 0 |
| 1730-1745 |  |  |  |  | 0 |
| 1745-1800 |  |  |  |  | 0 |
| 1800-1815 |  |  |  |  | 0 |
| 1815-1830 |  |  |  |  | 0 |
| Period End | 0 | 0 | 0 | 0 | 0 |



| Peds | NORTH | WEST | SOUTH | EAST | TOT |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Springhill Rd | Bluescope Steel | Springhill Rd | Colourbond |  |
| Peak Per | UNCLASSIFIED | UNCLASSIFIED | UNCLASSIFIED | UNCLASSIFIED |  |
| 1600-1700 | 0 | 0 | 0 | 0 | 0 |
| 1615-1715 | 0 | 0 | 0 | 0 | 0 |
| 1630-1730 | 0 | 0 | 0 | 0 | 0 |
| 1645-1745 | 0 | 0 | 0 | 0 | 0 |
| 1700-1800 | 0 | 0 | 0 | 0 | 0 |
| 1715-1815 | 0 | 0 | 0 | 0 | 0 |
| 1730-1830 | 0 | 0 | 0 | 0 | 0 |

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019 Day/Date : Thursday 9th April 2009


Springhill Rd


Wilton Rd \& Douglas Park Dr
TC - 20

To
Tim Lewis
at Traffix
your results for

## Wollongong Area Traffic Counts

supplied by
R.O.A.R. DATA Pty. Ltd.
www.roardata.com.au
R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849
Mobile. 0418239019

Client
Job No/Nam
Day/Date

Traffix
2654 Wollongong Area Traffic Surveys
Thursday 9th April 2009

| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Wilton Rd | Douglas ParI | Wilton Rd | TOT |
| $0700-0715$ |  |  |  | $\mathbf{0}$ |
| $0715-0730$ |  |  |  | $\mathbf{0}$ |
| $0730-0745$ |  | NOT |  | $\mathbf{0}$ |
| $0745-0800$ |  | REQUIRED |  | $\mathbf{0}$ |
| $0800-0815$ |  |  |  | $\mathbf{0}$ |
| $0815-0830$ |  |  |  | $\mathbf{0}$ |
| $0830-0845$ |  |  |  | $\mathbf{0}$ |
| $0845-0900$ |  |  |  | $\mathbf{0}$ |
| $0900-0915$ |  |  |  | $\mathbf{0}$ |
| $0915-0930$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Wilton $\boldsymbol{R d}$ | Douglas Parl | Wilton $\boldsymbol{R} \boldsymbol{l}$ | TOT |
| $0700-0800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0715-0815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0730-0830$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0745-0845$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0800-0900$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0815-0915$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0830-0930$ | 0 | 0 | 0 | $\mathbf{0}$ |


| PEAK HR | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas Parl |  | Wilton Rd |  |  |
| Time Per | I | $\underline{\text { L }}$ | $\underline{R}$ | $\underline{\text { L }}$ | $\underline{R}$ | I | TOT |
| 0700-0715 | 0 | 1 | 1 | 0 | 0 | 0 | 2 |
| 0715-0730 | 0 | 1 | 1 | 0 | 0 | 0 | 2 |
| 0730-0745 | 1 | 1 | 1 | 1 | 0 | 0 | 4 |
| 0745-0800 | 1 | 1 | 2 | 0 | 0 | 0 | 4 |
| 0800-0815 | 0 | 2 | 1 | 0 | 0 | 0 | 3 |
| 0815-0830 | 0 | 1 | 3 | 0 | 0 | 0 | 4 |
| 0830-0845 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0845-0900 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 0900-0915 | 0 |  | 0 | 0 | 0 | 1 | 1 |
| 0915-0930 | 0 | 1 | 0 | 0 | 0 | 0 | 1 |
| Per End | 2 | 8 | 9 | 1 | 0 | 1 | 21 |


| Combined | WEST |  | NORTH |  | $\begin{gathered} \hline \text { EAST } \\ \hline \text { Wilton Rd } \end{gathered}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas Parl |  |  |  |  |
| Time Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{\underline{L}}$ | R | I | TOT |
| 0700-0715 | 6 | 10 | 15 | 2 | 3 | 15 | 51 |
| 0715-0730 | 19 | 15 | 11 | 4 | 2 | 11 | 62 |
| 0730-0745 | 15 | 13 | 21 | 4 | 6 | 13 | 72 |
| 0745-0800 | 17 | 16 | 38 | 11 | 6 | 11 | 99 |
| 0800-0815 | 17 | 16 | 25 | 7 | 4 | 12 | 81 |
| 0815-0830 | 14 | 16 | 19 | 4 | 0 | 6 | 59 |
| 0830-0845 | 12 | 10 | 11 | 6 | 2 | 15 | 56 |
| 0845-0900 | 8 | 12 | 14 | 4 | 5 | 24 | 67 |
| 0900-0915 | 13 | 6 | 20 | 7 | 5 | 13 | 64 |
| 0915-0930 | 9 | 6 | 9 | 3 | 2 | 12 | 41 |
| Per End | 130 | 120 | 183 | 52 | 35 | 132 | 652 |


| Lights | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas Parl |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 0700-0800 | 55 | 50 | 80 | 20 | 17 | 50 | 272 |
| 0715-0815 | 66 | 55 | 90 | 25 | 18 | 47 | 301 |
| 0730-0830 | 61 | 56 | 96 | 25 | 16 | 42 | 296 |
| 0745-0845 | 59 | 54 | 87 | 28 | 12 | 44 | 284 |
| 0800-0900 | 51 | 51 | 65 | 21 | 11 | 57 | 256 |
| 0815-0915 | 47 | 43 | 61 | 21 | 12 | 57 | 241 |
| 0830-0930 | 42 | 33 | 54 | 20 | 14 | 63 | 226 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas Parl |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | R | I | TOT |
| 0700-0800 | 2 | 4 | 5 | 1 | 0 | 0 | 12 |
| 0715-0815 | 2 | 5 | 5 | 1 | 0 | 0 | 13 |
| 0730-0830 | 2 | 5 | 7 | 1 | 0 | 0 | 15 |
| 0745-0845 | 1 | 4 | 6 | 0 | 0 | 0 | 11 |
| 0800-0900 | 0 | 3 | 4 | 0 | 0 | 0 | 7 |
| 0815-0915 | 0 | 1 | 3 | 0 | 0 | 1 | 5 |
| 0830-0930 | 0 | 1 | 0 | 0 | 0 | 1 | 2 |


| Combined | WEST |  | NORTH |  | $\begin{gathered} \hline \text { EAST } \\ \hline \text { Wilton Rd } \end{gathered}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas Parl |  |  |  |  |
| Peak Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{\text { R }}$ | I | TOT |
| 0700-0800 | 57 | 54 | 85 | 21 | 17 | 50 | 284 |
| 0715-0815 | 68 | 60 | 95 | 26 | 18 | 47 | 314 |
| 0730-0830 | 63 | 61 | 103 | 26 | 16 | 42 | 311 |
| 0745-0845 | 60 | 58 | 93 | 28 | 12 | 44 | 295 |
| 0800-0900 | 51 | 54 | 69 | 21 | 11 | 57 | 263 |
| 0815-0915 | 47 | 44 | 64 | 21 | 12 | 58 | 246 |
| 0830-0930 | 42 | 34 | 54 | 20 | 14 | 64 | 228 |



## Douglas Parl Dr




| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Wilton Rd | Douglas | Wilton Rd | TOT |
| $1600-1615$ |  |  |  | $\mathbf{0}$ |
| $1615-1630$ |  |  |  | $\mathbf{0}$ |
| $1630-1645$ |  | NOT |  | $\mathbf{0}$ |
| $1645-1700$ |  | REQUIRED |  | $\mathbf{0}$ |
| $1700-1715$ |  |  |  | $\mathbf{0}$ |
| $1715-1730$ |  |  |  | $\mathbf{0}$ |
| $1730-1745$ |  |  |  | $\mathbf{0}$ |
| $1745-1800$ |  |  |  | $\mathbf{0}$ |
| $1800-1815$ |  |  |  | $\mathbf{0}$ |
| $1815-1830$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Wilton Rd | Douglas | Wilton Rd | TOT |
| $1600-1700$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1615-1715$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1630-1730$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1645-1745$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1700-1800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1715-1815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1730-1830$ | 0 | 0 | 0 | $\mathbf{0}$ |


| Lights | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas |  | Wilton Rd |  |  |
| Time Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{\text { L }}$ | $\underline{R}$ | I | TOT |
| 1600-1615 | 21 | 18 | 31 | 6 | 4 | 28 | 108 |
| 1615-1630 | 16 | 19 | 18 | 4 | 5 | 22 | 84 |
| 1630-1645 | 22 | 15 | 19 | 3 | 8 | 24 | 91 |
| 1645-1700 | 16 | 15 | 14 | 5 | 2 | 30 | 82 |
| 1700-1715 | 20 | 24 | 26 | 6 | 10 | 21 | 107 |
| 1715-1730 | 13 | 18 | 21 | 4 | 1 | 18 | 75 |
| 1730-1745 | 16 | 19 | 26 | 2 | 9 | 31 | 103 |
| 1745-1800 | 17 | 17 | 18 | 3 | 4 | 18 | 77 |
| 1800-1815 | 14 | 15 | 12 | 3 | 5 | 28 | 77 |
| 1815-1830 | 12 | 13 | 14 | 2 | 3 | 20 | 64 |
| Per End | 167 | 173 | 199 | 38 | 51 | 240 | 868 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas |  | Wilton Rd |  |  |
| Time Per | $\underline{\mathbf{I}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{}}$ | TOT |
| $1600-1615$ | 0 | 1 | 0 | 0 | 0 | 1 | $\mathbf{2}$ |
| $1615-1630$ | 1 | 0 | 1 | 0 | 0 | 1 | $\mathbf{3}$ |
| $1630-1645$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |
| $1645-1700$ | 0 | 2 | 0 | 1 | 0 | 0 | $\mathbf{3}$ |
| $1700-1715$ | 0 | 1 | 2 | 0 | 0 | 0 | $\mathbf{3}$ |
| $1715-1730$ | 0 | 1 | 2 | 0 | 0 | 0 | $\mathbf{3}$ |
| $1730-1745$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |
| $1745-1800$ | 0 | 0 | 0 | 0 | 0 | 1 | $\mathbf{1}$ |
| $1800-1815$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |
| $1815-1830$ | 1 | 0 | 0 | 0 | 0 | 0 | $\mathbf{1}$ |
| Per End | $\mathbf{2}$ | $\mathbf{5}$ | $\mathbf{5}$ | $\mathbf{1}$ | $\mathbf{0}$ | $\mathbf{3}$ | $\mathbf{1 6}$ |


| Combined | WEST |  | $\begin{aligned} & \hline \text { NORTH } \\ & \hline \text { Douglas } \end{aligned}$ |  | $\begin{gathered} \hline \text { EAST } \\ \hline \text { Wilton Rd } \end{gathered}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | R | I | TOT |
| 1600-1615 | 21 | 19 | 31 | 6 | 4 | 29 | 110 |
| 1615-1630 | 17 | 19 | 19 | 4 | 5 | 23 | 87 |
| 1630-1645 | 22 | 15 | 19 | 3 | 8 | 24 | 91 |
| 1645-1700 | 16 | 17 | 14 | 6 | 2 | 30 | 85 |
| 1700-1715 | 20 | 25 | 28 | 6 | 10 | 21 | 110 |
| 1715-1730 | 13 | 19 | 23 | 4 | 1 | 18 | 78 |
| 1730-1745 | 16 | 19 | 26 | 2 | 9 | 31 | 103 |
| 1745-1800 | 17 | 17 | 18 | 3 | 4 | 19 | 78 |
| 1800-1815 | 14 | 15 | 12 | 3 | 5 | 28 | 77 |
| 1815-1830 | 13 | 13 | 14 | 2 | 3 | 20 | 65 |
| Per End | 169 | 178 | 204 | 39 | 51 | 243 | 884 |


| Lights | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{\square}$ | R | $\underline{\underline{L}}$ | $\underline{R}$ | I | TOT |
| 1600-1700 | 75 | 67 | 82 | 18 | 19 | 104 | 365 |
| 1615-1715 | 74 | 73 | 77 | 18 | 25 | 97 | 364 |
| 1630-1730 | 71 | 72 | 80 | 18 | 21 | 93 | 355 |
| 1645-1745 | 65 | 76 | 87 | 17 | 22 | 100 | 367 |
| 1700-1800 | 66 | 78 | 91 | 15 | 24 | 88 | 362 |
| 1715-1815 | 60 | 69 | 77 | 12 | 19 | 95 | 332 |
| 1730-1830 | 59 | 64 | 70 | 10 | 21 | 97 | 321 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 1600-1700 | 1 | 3 | 1 | 1 | 0 | 2 | 8 |
| 1615-1715 | 1 | 3 | 3 | 1 | 0 | 1 | 9 |
| 1630-1730 | 0 | 4 | 4 | 1 | 0 | 0 | 9 |
| 1645-1745 | 0 | 4 | 4 | 1 | 0 | 0 | 9 |
| 1700-1800 | 0 | 2 | 4 | 0 | 0 | 1 | 7 |
| 1715-1815 | 0 | 1 | 2 | 0 | 0 | 1 | 4 |
| 1730-1830 | 1 | 0 | 0 | 0 | 0 | 1 | 2 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Douglas |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT |
| 1600-1700 | 76 | 70 | 83 | 19 | 19 | 106 | 373 |
| 1615-1715 | 75 | 76 | 80 | 19 | 25 | 98 | 373 |
| 1630-1730 | 71 | 76 | 84 | 19 | 21 | 93 | 364 |
| 1645-1745 | 65 | 80 | 91 | 18 | 22 | 100 | 376 |
| 1700-1800 | 66 | 80 | 95 | 15 | 24 | 89 | 369 |
| 1715-1815 | 60 | 70 | 79 | 12 | 19 | 96 | 336 |
| 1730-1830 | 60 | 64 | 70 | 10 | 21 | 98 | 323 |

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Ph.88196847, Fax 88196849, Mob.0418-239019

Job No/Name


Douglas Park Dr



## R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019


Douglas Park Dr


| $R R$22 18 <br> PM AM <br>  100 |
| :--- |



Wilton Rd
$T C-20$
Combined figures only
PM PEAK HOUR
1645-1745

## To



Appin Rd \& Wilton Rd AM

Tim Lewis
at Traffix your results for

Wollongong Area Traffic Counts
supplied by
R.O.A.R. DATA Pty. Ltd.
www.roardata.com.au

## R.O.A.R. DATA

Reliable, Original \& Authentic Results Ph.88196847, Fax 88196849. Mobile.0418239019

| Client | : Traffix |
| :---: | :--- |
| Job No/Name | $:$ 2654 Wollongong Area Traffic Surveys |
| Day/Date | $:$ Thursday 9th April 2009 |
|  |  |
|  | TC-21 |


|  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| PEDS | NORTH | EAST | SOUTH |  |
| $0700-0715$ |  |  |  | 0 |
| $0715-0730$ |  | NOT |  | 0 |
| $0730-0745$ |  | REQUIRED |  | 0 |
| $0745-0800$ |  |  |  | $\mathbf{0}$ |
| $0800-0815$ |  |  |  | $\mathbf{0}$ |
| $0815-0830$ |  |  |  | $\mathbf{0}$ |
| $0830-0845$ |  |  |  | $\mathbf{0}$ |
| $0845-0900$ |  |  |  | $\mathbf{0}$ |
| $0900-0915$ |  |  |  | $\mathbf{0}$ |
| $0915-0930$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


| PEDS | NORTH | EAST | SOUTH |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Appin Rd | Appin Rd | Wilton Rd | TOT |
| 0700-0800 | 0 | 0 | 0 | 0 |
| 0715-0815 | 0 | 0 | 0 | 0 |
| 0730-0830 | 0 | 0 | 0 | 0 |
| 0745-0845 | 0 | 0 | 0 | 0 |
| 0800-0900 | 0 | 0 | 0 | 0 |
| 0815-0915 | 0 | 0 | 0 | 0 |
| 0830-0930 | 0 | 0 | 0 | 0 |


| PEAKHR | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- |


| Lights | NORTH |  | EAST |  | SOUTH |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  | Wilton Rd |  |  |
| Time Per | I | $\underline{L}$ | R | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 0700-0715 | 8 | 56 | 69 | 15 | 5 | 13 | 166 |
| 0715-0730 | 17 | 76 | 105 | 11 | 5 | 18 | 232 |
| 0730-0745 | 9 | 96 | 159 | 10 | 11 | 13 | 298 |
| 0745-0800 | 8 | 60 | 131 | 13 | 12 | 15 | 239 |
| 0800-0815 | 7 | 70 | 139 | 10 | 12 | 20 | 258 |
| 0815-0830 | 8 | 52 | 114 | 7 | 11 | 20 | 212 |
| 0830-0845 | 12 | 47 | 77 | 10 | 5 | 18 | 169 |
| 0845-0900 | 18 | 51 | 58 | 14 | 4 | 21 | 166 |
| 0900-0915 | 20 | 48 | 51 | 11 | 8 | 33 | 171 |
| 0915-0930 | 27 | 50 | 50 | 5 | 6 | 14 | 152 |
| Per End | 134 | 606 | 953 | 106 | 79 | 185 | 2063 |


| Heavies | NORTH |  | EAST |  | SOUTH |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  | Wilton Rd |  |  |
| Time Per | I | $\underline{L}$ | R | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 0700-0715 | 1 | 2 | 1 | 0 | 0 | 0 | 4 |
| 0715-0730 | 4 | 2 | 1 | 0 | 0 | 1 | 8 |
| 0730-0745 | 2 | 2 | 4 | 0 | 1 | 0 | 9 |
| 0745-0800 | 1 | 1 | 2 | 0 | 1 | 0 | 5 |
| 0800-0815 | 0 | 4 | 3 | 1 | 0 | 1 | 9 |
| 0815-0830 | 0 | 8 | 4 | 1 | 0 | 1 | 14 |
| 0830-0845 | 0 | 0 | 2 | 0 | 0 | 3 | 5 |
| 0845-0900 | 0 | 5 | 2 | 0 | 0 | 1 | 8 |
| 0900-0915 | 1 | 1 | 4 | 0 | 1 | 2 | 9 |
| 0915-0930 | 1 | 3 | 2 | 0 | 1 | 0 | 7 |
| Per End | 10 | 28 | 25 | 2 | 4 | 9 | 78 |


| Combined | NORTH |  | EAST |  | $\begin{gathered} \hline \text { SOUTH } \\ \hline \text { Wilton Rd } \end{gathered}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  |  |  |  |
| Time Per | I | $\underline{\square}$ | R | $\underline{L}$ | R | I | TOT |
| 0700-0715 | 9 | 58 | 70 | 15 | 5 | 13 | 170 |
| 0715-0730 | 21 | 78 | 106 | 11 | 5 | 19 | 240 |
| 0730-0745 | 11 | 98 | 163 | 10 | 12 | 13 | 307 |
| 0745-0800 | 9 | 61 | 133 | 13 | 13 | 15 | 244 |
| 0800-0815 | 7 | 74 | 142 | 11 | 12 | 21 | 267 |
| 0815-0830 | 8 | 60 | 118 | 8 | 11 | 21 | 226 |
| 0830-0845 | 12 | 47 | 79 | 10 | 5 | 21 | 174 |
| 0845-0900 | 18 | 56 | 60 | 14 | 4 | 22 | 174 |
| 0900-0915 | 21 | 49 | 55 | 11 | 9 | 35 | 180 |
| 0915-0930 | 28 | 53 | 52 | 5 | 7 | 14 | 159 |
| Per End | 144 | 634 | 978 | 108 | 83 | 194 | 2141 |


| Lights | NORTH |  | EAST |  | SOUTH |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT |
| 0700-0800 | 42 | 288 | 464 | 49 | 33 | 59 | 935 |
| 0715-0815 | 41 | 302 | 534 | 44 | 40 | 66 | 1027 |
| 0730-0830 | 32 | 278 | 543 | 40 | 46 | 68 | 1007 |
| 0745-0845 | 35 | 229 | 461 | 40 | 40 | 73 | 878 |
| 0800-0900 | 45 | 220 | 388 | 41 | 32 | 79 | 805 |
| 0815-0915 | 58 | 198 | 300 | 42 | 28 | 92 | 718 |
| 0830-0930 | 77 | 196 | 236 | 40 | 23 | 86 | 658 |


| Heavies | NORTH |  | EAST |  | SOUTH |  | TOT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I |  |
| 0700-0800 | 8 | 7 | 8 | 0 | 2 | 1 | 26 |
| 0715-0815 | 7 | 9 | 10 | 1 | 2 | 2 | 31 |
| 0730-0830 | 3 | 15 | 13 | 2 | 2 | 2 | 37 |
| 0745-0845 | 1 | 13 | 11 | 2 | 1 | 5 | 33 |
| 0800-0900 | 0 | 17 | 11 | 2 | 0 | 6 | 36 |
| 0815-0915 | 1 | 14 | 12 | 1 | 1 | 7 | 36 |
| 0830-0930 | 2 | 9 | 10 | 0 | 2 | 6 | 29 |


| Combined | NORTH |  | EAST |  | SOUTH |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT |
| 0700-0800 | 50 | 295 | 472 | 49 | 35 | 60 | 961 |
| 0715-0815 | 48 | 311 | 544 | 45 | 42 | 68 | 1058 |
| 0730-0830 | 35 | 293 | 556 | 42 | 48 | 70 | 1044 |
| 0745-0845 | 36 | 242 | 472 | 42 | 41 | 78 | 911 |
| 0800-0900 | 45 | 237 | 399 | 43 | 32 | 85 | 841 |
| 0815-0915 | 59 | 212 | 312 | 43 | 29 | 99 | 754 |
| 0830-0930 | 79 | 205 | 246 | 40 | 25 | 92 | 687 |

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AM PEAK
0715-0815

TOTAL VOLUMES FOR COUNT

N $\frac{d x}{40}$


## R.O.A.R. DATA

Reliable, Original \& Authentic Results Ph.88196847, Fax 88196849, Mob.0418-239019

Client : Traffix
Job No/Name : 2654 Wollongong Area Traffic Surveys
Day/Date

## Intersection Details

Obtained via satellite
May be incorrect


Wilton Rd

## To



Appin Rd \& Wilton Rd PM

Tim Lewis
at Traffix your results for

Wollongong Area Traffic Counts
supplied by
R.O.A.R. DATA Pty. Ltd.
www.roardata.com.au

## R.O.A.R. DATA

Reliable, Original \& Authentic Results Ph.88196847, Fax 88196849. Mobile.0418239019

| Client | : Traffix |
| :---: | :--- |
| Job No/Name | $: 265$ Wollongong Area Traffic Surveys |
| Day/Date | $:$ Thursday 9th April 2009 |
|  |  |
|  | TC-21 |


| PEDS | NORTH | EAST | SOUTH |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Appin Rd | Appin Rd | Wilton Rd | TOT |
| $1600-1615$ |  |  |  | $\mathbf{0}$ |
| $1615-1630$ |  | NOT |  | 0 |
| $1630-1645$ |  | REQUIRED |  | $\mathbf{0}$ |
| $1645-1700$ |  |  |  | $\mathbf{0}$ |
| $1700-1715$ |  |  |  | $\mathbf{0}$ |
| $1715-1730$ |  |  |  | $\mathbf{0}$ |
| $1730-1745$ |  |  |  | $\mathbf{0}$ |
| $1745-1800$ |  |  |  | $\mathbf{0}$ |
| $1800-1815$ |  |  |  | $\mathbf{0}$ |
| $1815-1830$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


| PEDS | NORTH | EAST | SOUTH |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Appin Rd | Appin Rd | Wilton Rd | TOT |
| $1600-1700$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1615-1715$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1630-1730$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1645-1745$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1700-1800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1715-1815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1730-1830$ | 0 | 0 | 0 | $\mathbf{0}$ |


| PEAKHR | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- |


| Lights | NORTH |  | EAST |  | SOUTH |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  | Wilton Rd |  |  |
| Time Per | I | $\underline{\text { L }}$ | $\underline{\text { R }}$ | $\underline{L}$ | $\underline{\text { R }}$ | I | TOT |
| 1600-1615 | 23 | 131 | 84 | 13 | 15 | 23 | 289 |
| 1615-1630 | 18 | 104 | 74 | 15 | 16 | 32 | 259 |
| 1630-1645 | 24 | 110 | 84 | 16 | 11 | 23 | 268 |
| 1645-1700 | 18 | 122 | 90 | 23 | 14 | 11 | 278 |
| 1700-1715 | 18 | 112 | 62 | 20 | 20 | 14 | 246 |
| 1715-1730 | 26 | 110 | 67 | 24 | 7 | 14 | 248 |
| 1730-1745 | 21 | 123 | 72 | 19 | 17 | 12 | 264 |
| 1745-1800 | 23 | 111 | 76 | 11 | 6 | 18 | 245 |
| 1800-1815 | 23 | 95 | 60 | 20 | 7 | 13 | 218 |
| 1815-1830 | 24 | 118 | 44 | 18 | 8 | 14 | 226 |
| Per End | 218 | 1136 | 713 | 179 | 121 | 174 | 2541 |


| Heavies | NORTH |  | EAST |  | SOUTH |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  | Wilton Rd |  |  |
| Time Per | I | $\underline{\text { L }}$ | $\underline{\text { R }}$ | $\underline{L}$ | R | I | TOT |
| 1600-1615 | 1 | 2 | 4 | 2 | 0 | 0 | 9 |
| 1615-1630 | 0 | 0 | 1 | 0 | 1 | 0 | 2 |
| 1630-1645 | 0 | 1 | 2 | 0 | 0 | 1 | 4 |
| 1645-1700 | 0 | 1 | 0 | 0 | 0 | 0 | 1 |
| 1700-1715 | 0 | 0 | 2 | 0 | 0 | 1 | 3 |
| 1715-1730 | 0 | 1 | 1 | 0 | 0 | 0 | 2 |
| 1730-1745 | 2 | 0 | 0 | 0 | 0 | 0 | 2 |
| 1745-1800 | 0 | 3 | 1 | 0 | 0 | 0 | 4 |
| 1800-1815 | 1 | 0 | 3 | 0 | 1 | 0 | 5 |
| 1815-1830 | 0 | 0 | 2 | 0 | 0 | 0 | 2 |
| Per End | 4 | 8 | 16 | 2 | 2 | 2 | 34 |


| Combined | NORTH |  | EAST |  | SOUTH |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  | Wilton Rd |  |  |
| Time Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | R | I | TOT |
| 1600-1615 | 24 | 133 | 88 | 15 | 15 | 23 | 298 |
| 1615-1630 | 18 | 104 | 75 | 15 | 17 | 32 | 261 |
| 1630-1645 | 24 | 111 | 86 | 16 | 11 | 24 | 272 |
| 1645-1700 | 18 | 123 | 90 | 23 | 14 | 11 | 279 |
| 1700-1715 | 18 | 112 | 64 | 20 | 20 | 15 | 249 |
| 1715-1730 | 26 | 111 | 68 | 24 | 7 | 14 | 250 |
| 1730-1745 | 23 | 123 | 72 | 19 | 17 | 12 | 266 |
| 1745-1800 | 23 | 114 | 77 | 11 | 6 | 18 | 249 |
| 1800-1815 | 24 | 95 | 63 | 20 | 8 | 13 | 223 |
| 1815-1830 | 24 | 118 | 46 | 18 | 8 | 14 | 228 |
| Per End | 222 | 1144 | 729 | 181 | 123 | 176 | 2575 |



| Combined | NORTH |  | EAST |  | SOUTH |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Appin Rd |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{L}$ | R | $\underline{\text { L }}$ | $\underline{\text { R }}$ | I | TOT |
| 1600-1700 | 84 | 471 | 339 | 69 | 57 | 90 | 1110 |
| 1615-1715 | 78 | 450 | 315 | 74 | 62 | 82 | 1061 |
| 1630-1730 | 86 | 457 | 308 | 83 | 52 | 64 | 1050 |
| 1645-1745 | 85 | 469 | 294 | 86 | 58 | 52 | 1044 |
| 1700-1800 | 90 | 460 | 281 | 74 | 50 | 59 | 1014 |
| 1715-1815 | 96 | 443 | 280 | 74 | 38 | 57 | 988 |
| 1730-1830 | 94 | 450 | 258 | 68 | 39 | 57 | 966 |

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| PM PEAK |
| :---: |
| $1600-1700$ |

N $\frac{d x}{4}$

## Appin Rd



Wilton Rd

Appin Rd


Wilton Rd
R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019
orsection Details
Obtained via satellite
May be incorrect
PM PEAK HOUR
1600-1700


Wilton Rd

To


Appin Rd \& Princess Hwy Off Ramp

R.O.A.R. DATA Pty. Ltd.

www.roardata.com.au

R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849
Mobile. 0418239019

Client
Job No/Name Day/Date

Traffix
2654 Wollongong Area Traffic Surveys
Thursday 9th April 2009

TC-22

| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Peak Per | Appin Rd | Princess Hwy | Appin $\boldsymbol{R} \boldsymbol{d}$ |
| TOT |  |  |  |  |
| $0700-0800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0715-0815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0730-0830$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0745-0845$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0800-0900$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0815-0915$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0830-0930$ | 0 | 0 | 0 | $\mathbf{0}$ |


| PEAK HR | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Princess Hwy |  | Appin Rd |  |  |
| Time Per | I | $\underline{L}$ | R | L | R | I | TOT |
| 0700-0715 | 10 | 0 | 0 | 0 | 0 | 4 | 14 |
| 0715-0730 | 5 | 1 | 1 | 0 | 0 | 10 | 17 |
| 0730-0745 | 8 | 0 | 4 | 0 | 0 | 10 | 22 |
| 0745-0800 | 12 | 0 | 0 | 0 | 0 | 10 | 22 |
| 0800-0815 | 9 | 1 | 0 | 0 | 0 | 9 | 19 |
| 0815-0830 | 5 | 0 | 0 | 0 | 0 | 10 | 15 |
| 0830-0845 | 9 | 1 | 1 | 0 | 0 | 6 | 17 |
| 0845-0900 | 11 | 0 | 2 | 0 | 0 | 9 | 22 |
| 0900-0915 | 13 | 0 | 2 | 0 | 0 | 2 | 17 |
| 0915-0930 | 16 | 0 | 0 | 0 | 0 | 6 | 22 |
| Per End | 98 | 3 | 10 | 0 | 0 | 76 | 187 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Princess Hwy |  | Appin Rd |  |  |
| Time Per | I | $\underline{\text { L }}$ | $\underline{R}$ | $\underline{L}$ | R | I | TOT |
| 0700-0715 | 58 | 0 | 7 | 0 | 0 | 97 | 162 |
| 0715-0730 | 64 | 2 | 15 | 0 | 0 | 133 | 214 |
| 0730-0745 | 76 | 1 | 13 | 0 | 0 | 164 | 254 |
| 0745-0800 | 105 | 2 | 12 | 0 | 0 | 157 | 276 |
| 0800-0815 | 72 | 2 | 8 | 0 | 0 | 98 | 180 |
| 0815-0830 | 79 | 3 | 14 | 0 | 0 | 88 | 184 |
| 0830-0845 | 72 | 3 | 9 | 0 | 0 | 68 | 152 |
| 0845-0900 | 63 | 0 | 14 | 0 | 0 | 58 | 135 |
| 0900-0915 | 59 | 1 | 14 | 0 | 0 | 50 | 124 |
| 0915-0930 | 55 | 1 | 9 | 0 | 0 | 60 | 125 |
| Per End | 703 | 15 | 115 | 0 | 0 | 973 | 1806 |


| Lights |  |  | Princ | Hwy | $\begin{gathered} \hline \text { EAST } \\ \hline \text { Appin Rd } \end{gathered}$ |  |  | Heavies | WEST |  | $\frac{\text { NORTH }}{\text { Princess Hwy }}$ |  | $\begin{gathered} \hline \text { EAST } \\ \hline \text { Appin Rd } \end{gathered}$ |  | Combined |  | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I |  | Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT | Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT |
| 0700-0800 | 268 | 4 | 42 | 0 | 0 | 517 | 831 | 0700-0800 | 35 | 1 | 5 | 0 | 0 | 34 | 75 | 0700-0800 | 303 | 5 | 47 | 0 | 0 | 551 | 906 |
| 0715-0815 | 283 | 5 | 43 | 0 | 0 | 513 | 844 | 0715-0815 | 34 | 2 | 5 | 0 | 0 | 39 | 80 | 0715-0815 | 317 | 7 | 48 | 0 | 0 | 552 | 924 |
| 0730-0830 | 298 | 7 | 43 | 0 | 0 | 468 | 816 | 0730-0830 | 34 | 1 | 4 | 0 | 0 | 39 | 78 | 0730-0830 | 332 | 8 | 47 | 0 | 0 | 507 | 894 |
| 0745-0845 | 293 | 8 | 42 | 0 | 0 | 376 | 719 | 0745-0845 | 35 | 2 | 1 | 0 | 0 | 35 | 73 | 0745-0845 | 328 | 10 | 43 | 0 | 0 | 411 | 792 |
| 0800-0900 | 252 | 6 | 42 | 0 | 0 | 278 | 578 | 0800-0900 | 34 | 2 | 3 | 0 | 0 | 34 | 73 | 0800-0900 | 286 | 8 | 45 | 0 | 0 | 312 | 651 |
| 0815-0915 | 235 | 6 | 46 | 0 | 0 | 237 | 524 | 0815-0915 | 38 | 1 | 5 | 0 | 0 | 27 | 71 | 0815-0915 | 273 | 7 | 51 | 0 | 0 | 264 | 595 |
| 0830-0930 | 200 | 4 | 41 | 0 | 0 | 213 | 458 | 0830-0930 | 49 | 1 | 5 | 0 | 0 | 23 | 78 | 0830-0930 | 249 | 5 | 46 | 0 | 0 | 236 | 536 |


| Lights |  |  | Princ | Hwy | $\begin{gathered} \hline \text { EAST } \\ \hline \text { Appin Rd } \end{gathered}$ |  |  | Heavies | WEST |  | $\frac{\text { NORTH }}{\text { Princess Hwy }}$ |  | $\begin{gathered} \hline \text { EAST } \\ \hline \text { Appin Rd } \end{gathered}$ |  | Combined |  | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I |  | Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT | Peak Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT |
| 0700-0800 | 268 | 4 | 42 | 0 | 0 | 517 | 831 | 0700-0800 | 35 | 1 | 5 | 0 | 0 | 34 | 75 | 0700-0800 | 303 | 5 | 47 | 0 | 0 | 551 | 906 |
| 0715-0815 | 283 | 5 | 43 | 0 | 0 | 513 | 844 | 0715-0815 | 34 | 2 | 5 | 0 | 0 | 39 | 80 | 0715-0815 | 317 | 7 | 48 | 0 | 0 | 552 | 924 |
| 0730-0830 | 298 | 7 | 43 | 0 | 0 | 468 | 816 | 0730-0830 | 34 | 1 | 4 | 0 | 0 | 39 | 78 | 0730-0830 | 332 | 8 | 47 | 0 | 0 | 507 | 894 |
| 0745-0845 | 293 | 8 | 42 | 0 | 0 | 376 | 719 | 0745-0845 | 35 | 2 | 1 | 0 | 0 | 35 | 73 | 0745-0845 | 328 | 10 | 43 | 0 | 0 | 411 | 792 |
| 0800-0900 | 252 | 6 | 42 | 0 | 0 | 278 | 578 | 0800-0900 | 34 | 2 | 3 | 0 | 0 | 34 | 73 | 0800-0900 | 286 | 8 | 45 | 0 | 0 | 312 | 651 |
| 0815-0915 | 235 | 6 | 46 | 0 | 0 | 237 | 524 | 0815-0915 | 38 | 1 | 5 | 0 | 0 | 27 | 71 | 0815-0915 | 273 | 7 | 51 | 0 | 0 | 264 | 595 |
| 0830-0930 | 200 | 4 | 41 | 0 | 0 | 213 | 458 | 0830-0930 | 49 | 1 | 5 | 0 | 0 | 23 | 78 | 0830-0930 | 249 | 5 | 46 | 0 | 0 | 236 | 536 |



## R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019

## Client

Job No/Name
Day/Date
: 2654 Wollongong Area Traffic Surveys


TOTAL VOLUMES FOR COUNT PERIOD



| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Appin Rd | Princess | Appin Rd | TOT |
| $1600-1615$ |  |  |  | $\mathbf{0}$ |
| $1615-1630$ |  |  |  | $\mathbf{0}$ |
| $1630-1645$ |  | NOT |  | $\mathbf{0}$ |
| $1645-1700$ |  | REQUIRED |  | $\mathbf{0}$ |
| $1700-1715$ |  |  |  | $\mathbf{0}$ |
| $1715-1730$ |  |  |  | $\mathbf{0}$ |
| $1730-1745$ |  |  |  | $\mathbf{0}$ |
| $1745-1800$ |  |  |  | $\mathbf{0}$ |
| $1800-1815$ |  |  |  | $\mathbf{0}$ |
| $1815-1830$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Appin Rd | Princess | Appin Rd | TOT |
| $1600-1700$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1615-1715$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1630-1730$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1645-1745$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1700-1800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1715-1815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1730-1830$ | 0 | 0 | 0 | $\mathbf{0}$ |


| Lights | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Princess |  | Appin Rd |  |  |
| Time Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT |
| 1600-1615 | 153 | 2 | 14 | 0 | 0 | 64 | 233 |
| 1615-1630 | 120 | 1 | 17 | 0 | 0 | 65 | 203 |
| 1630-1645 | 115 | 0 | 24 | 0 | 0 | 63 | 202 |
| 1645-1700 | 138 | 0 | 19 | 0 | 0 | 59 | 216 |
| 1700-1715 | 116 | 1 | 27 | 0 | 0 | 54 | 198 |
| 1715-1730 | 106 | 0 | 24 | 0 | 0 | 53 | 183 |
| 1730-1745 | 110 | 0 | 19 | 0 | 0 | 64 | 193 |
| 1745-1800 | 112 | 1 | 24 | 1 | 0 | 43 | 181 |
| 1800-1815 | 87 | 1 | 18 | 0 | 0 | 45 | 151 |
| 1815-1830 | 71 | 0 | 17 | 1 | 0 | 47 | 136 |
| Per End | 1128 | 6 | 203 | 2 | 0 | 557 | 1896 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Princess |  | Appin Rd |  |  |
| Time Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT |
| 1600-1615 | 8 | 0 | 0 | 0 | 0 | 3 | 11 |
| 1615-1630 | 5 | 1 | 0 | 0 | 0 | 3 | 9 |
| 1630-1645 | 4 | 0 | 0 | 0 | 0 | 5 | 9 |
| 1645-1700 | 5 | 0 | 0 | 0 | 0 | 4 | 9 |
| 1700-1715 | 10 | 0 | 0 | 0 | 0 | 3 | 13 |
| 1715-1730 | 10 | 0 | 0 | 0 | 0 | 5 | 15 |
| 1730-1745 | 3 | 0 | 1 | 0 | 0 | 7 | 11 |
| 1745-1800 | 5 | 0 | 0 | 0 | 0 | 3 | 8 |
| 1800-1815 | 6 | 0 | 0 | 0 | 0 | 6 | 12 |
| 1815-1830 | 5 | 0 | 0 | 0 | 0 | 5 | 10 |
| Per End | 61 | 1 | 1 | 0 | 0 | 44 | 107 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Princess |  | Appin Rd |  |  |
| Time Per | I | $\underline{L}$ | R | $\underline{L}$ | R | I | TOT |
| 1600-1615 | 161 | 2 | 14 | 0 | 0 | 67 | 244 |
| 1615-1630 | 125 | 2 | 17 | 0 | 0 | 68 | 212 |
| 1630-1645 | 119 | 0 | 24 | 0 | 0 | 68 | 211 |
| 1645-1700 | 143 | 0 | 19 | 0 | 0 | 63 | 225 |
| 1700-1715 | 126 | 1 | 27 | 0 | 0 | 57 | 211 |
| 1715-1730 | 116 | 0 | 24 | 0 | 0 | 58 | 198 |
| 1730-1745 | 113 | 0 | 20 | 0 | 0 | 71 | 204 |
| 1745-1800 | 117 | 1 | 24 | 1 | 0 | 46 | 189 |
| 1800-1815 | 93 | 1 | 18 | 0 | 0 | 51 | 163 |
| 1815-1830 | 76 | 0 | 17 | 1 | 0 | 52 | 146 |
| Per End | 1189 | 7 | 204 | 2 | 0 | 601 | 2003 |


| Lights | $\frac{\text { WEST }}{\text { Appin Rd }}$ |  | NORTH |  | $\frac{\text { EAST }}{\text { Appin Rd }}$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Peak Per | I | L | R | $\underline{L}$ | R | I | TOT |
| 1600-1700 | 526 | 3 | 74 | 0 | 0 | 251 | 854 |
| 1615-1715 | 489 | 2 | 87 | 0 | 0 | 241 | 819 |
| 1630-1730 | 475 | 1 | 94 | 0 | 0 | 229 | 799 |
| 1645-1745 | 470 | 1 | 89 | 0 | 0 | 230 | 790 |
| 1700-1800 | 444 | 2 | 94 | 1 | 0 | 214 | 755 |
| 1715-1815 | 415 | 2 | 85 | 1 | 0 | 205 | 708 |
| 1730-1830 | 380 | 2 | 78 | 2 | 0 | 199 | 661 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Princess |  | Appin Rd |  |  |
| Peak Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{\text { R }}$ | I | TOT |
| 1600-1700 | 22 | 1 | 0 | 0 | 0 | 15 | 38 |
| 1615-1715 | 24 | 1 | 0 | 0 | 0 | 15 | 40 |
| 1630-1730 | 29 | 0 | 0 | 0 | 0 | 17 | 46 |
| 1645-1745 | 28 | 0 | 1 | 0 | 0 | 19 | 48 |
| 1700-1800 | 28 | 0 | 1 | 0 | 0 | 18 | 47 |
| 1715-1815 | 24 | 0 | 1 | 0 | 0 | 21 | 46 |
| 1730-1830 | 19 | 0 | 1 | 0 | 0 | 21 | 41 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Appin Rd |  | Princess |  | Appin Rd |  |  |
| Peak Per | I | $\underline{L}$ | $\underline{\text { R }}$ | $\underline{L}$ | $\underline{\mathrm{R}}$ | I | TOT |
| 1600-1700 | 548 | 4 | 74 | 0 | 0 | 266 | 892 |
| 1615-1715 | 513 | 3 | 87 | 0 | 0 | 256 | 859 |
| 1630-1730 | 504 | 1 | 94 | 0 | 0 | 246 | 845 |
| 1645-1745 | 498 | 1 | 90 | 0 | 0 | 249 | 838 |
| 1700-1800 | 472 | 2 | 95 | 1 | 0 | 232 | 802 |
| 1715-1815 | 439 | 2 | 86 | 1 | 0 | 226 | 754 |
| 1730-1830 | 399 | 2 | 79 | 2 | 0 | 220 | 702 |

R.O.A.R. DATA

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## Princess Hwy Off Ramp



## R.O.A.R. DATA

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Ph.88196847, Fax 88196849, Mob.0418-239019


Wilton Rd \& Macarthur Rd
TC-23

To
Tim Lewis at Traffix
your results for

## Wollongong Area Traffic Counts

supplied by
R.O.A.R. DATA Pty. Ltd.
www.roardata.com.au
R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849.
Mobile.0418239019

Client Job No/Name Day/Date

Traffix
2654 Wollongong Area Traffic Surveys : Thursday 9th April 2009

TC-23

| PEDS | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| time Per | Wilton Rd | Macarthur Rd | Wilton Rd | TOT |
| $0700-0715$ |  |  |  | 0 |
| $0715-0730$ |  |  |  | 0 |
| $0730-0745$ |  | NOT |  | 0 |
| $0745-0800$ |  | REQUIRED |  | 0 |
| $0800-0815$ |  |  |  | $\mathbf{0}$ |
| $0815-0830$ |  |  |  | $\mathbf{0}$ |
| $0830-0845$ |  |  |  | $\mathbf{0}$ |
| $0845-0900$ |  |  |  | $\mathbf{0}$ |
| $0900-0915$ |  |  |  | $\mathbf{0}$ |
| $0915-0930$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


| PEDS | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Wilton Rd | Macarthur Rd | Wilton Rd | TOT |
| $0700-0800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0715-0815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0730-0830$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0745-0845$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0800-0900$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0815-0915$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $0830-0930$ | 0 | 0 | 0 | $\mathbf{0}$ |


| PEAK HR | 0 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur Rd |  | Wilton Rd |  |  |
| Time Per | I | R | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | I | TOT |
| 0700-0715 | 9 | 1 | 0 | 6 | 7 | 18 | 41 |
| 0715-0730 | 23 | 0 | 0 | 7 | 8 | 16 | 54 |
| 0730-0745 | 21 | 0 | 1 | 4 | 10 | 22 | 58 |
| 0745-0800 | 17 | 3 | 1 | 15 | 30 | 19 | 85 |
| 0800-0815 | 23 | 0 | 1 | 8 | 11 | 23 | 66 |
| 0815-0830 | 25 | 3 | 1 | 4 | 8 | 11 | 52 |
| 0830-0845 | 14 | 2 | 2 | 8 | 12 | 16 | 54 |
| 0845-0900 | 16 | 0 | 1 | 4 | 10 | 25 | 56 |
| 0900-0915 | 16 | 2 | 3 | 2 | 12 | 18 | 53 |
| 0915-0930 | 13 | 0 | 0 | 1 | 6 | 15 | 35 |
| Per End | 177 | 11 | 10 | 59 | 114 | 183 | 554 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur Rd |  | Wilton Rd |  |  |
| Time Per | $\underline{\mathbf{T}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{T}}$ | TOT |
| $0700-0715$ | 0 | 0 | 0 | 1 | 0 | 1 | $\mathbf{2}$ |
| $0715-0730$ | 1 | 0 | 0 | 1 | 0 | 1 | $\mathbf{3}$ |
| $0730-0745$ | 1 | 1 | 1 | 1 | 0 | 1 | $\mathbf{5}$ |
| $0745-0800$ | 1 | 0 | 0 | 1 | 1 | 1 | $\mathbf{4}$ |
| $0800-0815$ | 0 | 0 | 0 | 2 | 0 | 0 | $\mathbf{2}$ |
| $0815-0830$ | 1 | 0 | 0 | 0 | 3 | 1 | $\mathbf{5}$ |
| $0830-0845$ | 0 | 2 | 0 | 0 | 0 | 0 | $\mathbf{2}$ |
| $0845-0900$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |
| $0900-0915$ | 0 | 0 | 0 | 0 | 0 | 1 | $\mathbf{1}$ |
| $0915-0930$ | $\mathbf{1}$ | 0 | 2 | 0 | 0 | 0 | $\mathbf{3}$ |
| Per End | $\mathbf{5}$ | $\mathbf{3}$ | $\mathbf{3}$ | $\mathbf{6}$ | $\mathbf{4}$ | $\mathbf{6}$ | $\mathbf{2 7}$ |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur Rd |  | Wilton Rd |  |  |
| Time Per | $\underline{\mathbf{T}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{I}}$ | TOT |
| $0700-0715$ | 9 | 1 | 0 | 7 | 7 | 19 | $\mathbf{4 3}$ |
| $0715-0730$ | 24 | 0 | 0 | 8 | 8 | 17 | $\mathbf{5 7}$ |
| $0730-0745$ | 22 | 1 | 2 | 5 | 10 | 23 | $\mathbf{6 3}$ |
| $0745-0800$ | 18 | 3 | 1 | 16 | 31 | 20 | $\mathbf{8 9}$ |
| $0800-0815$ | 23 | 0 | 1 | 10 | 11 | 23 | $\mathbf{6 8}$ |
| $0815-0830$ | 26 | 3 | 1 | 4 | 11 | 12 | $\mathbf{5 7}$ |
| $0830-0845$ | 14 | 4 | 2 | 8 | 12 | 16 | $\mathbf{5 6}$ |
| $0845-0900$ | 16 | 0 | 1 | 4 | 10 | 25 | $\mathbf{5 6}$ |
| $0900-0915$ | 16 | 2 | 3 | 2 | 12 | 19 | $\mathbf{5 4}$ |
| $0915-0930$ | 14 | 0 | 2 | 1 | 6 | 15 | $\mathbf{3 8}$ |
| Per End | $\mathbf{1 8 2}$ | $\mathbf{1 4}$ | $\mathbf{1 3}$ | $\mathbf{6 5}$ | $\mathbf{1 1 8}$ | $\mathbf{1 8 9}$ | $\mathbf{5 8 1}$ |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur Rd |  | Wilton Rd |  |  |
| Peak Per | $\underline{\mathbf{T}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{T}}$ | TOT |
| $0700-0800$ | 70 | 4 | 2 | 32 | 55 | 75 | $\mathbf{2 3 8}$ |
| $\mathbf{0 7 1 5 - 0 8 1 5}$ | 84 | 3 | 3 | 34 | 59 | 80 | $\mathbf{2 6 3}$ |
| $0730-0830$ | 86 | 6 | 4 | 31 | 59 | 75 | $\mathbf{2 6 1}$ |
| $0745-0845$ | 79 | 8 | 5 | 35 | 61 | 69 | $\mathbf{2 5 7}$ |
| $0800-0900$ | 78 | 5 | 5 | 24 | 41 | 75 | $\mathbf{2 2 8}$ |
| $0815-0915$ | 71 | 7 | 7 | 18 | 42 | 70 | $\mathbf{2 1 5}$ |
| $0830-0930$ | 59 | 4 | 6 | 15 | 40 | 74 | $\mathbf{1 9 8}$ |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur Rd |  | Wilton Rd |  |  |
| Peak Per | T | R | $\underline{L}$ | R | $\underline{L}$ | I | TOT |
| 0700-0800 | 3 | 1 | 1 | 4 | 1 | 4 | 14 |
| 0715-0815 | 3 | 1 | 1 | 5 | 1 | 3 | 14 |
| 0730-0830 | 3 | 1 | 1 | 4 | 4 | 3 | 16 |
| 0745-0845 | 2 | 2 | 0 | 3 | 4 | 2 | 13 |
| 0800-0900 | 1 | 2 | 0 | 2 | 3 | 1 | 9 |
| 0815-0915 | 1 | 2 | 0 | 0 | 3 | 2 | 8 |
| 0830-0930 | 1 | 2 | 2 | 0 | 0 | 1 | 6 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur Rd |  | Wilton Rd |  |  |
| Peak Per | I | R | $\underline{L}$ | R | $\underline{L}$ | I | TOT |
| 0700-0800 | 73 | 5 | 3 | 36 | 56 | 79 | 252 |
| 0715-0815 | 87 | 4 | 4 | 39 | 60 | 83 | 277 |
| 0730-0830 | 89 | 7 | 5 | 35 | 63 | 78 | 277 |
| 0745-0845 | 81 | 10 | 5 | 38 | 65 | 71 | 270 |
| 0800-0900 | 79 | 7 | 5 | 26 | 44 | 76 | 237 |
| 0815-0915 | 72 | 9 | 7 | 18 | 45 | 72 | 223 |
| 0830-0930 | 60 | 6 | 8 | 15 | 40 | 75 | 204 |



Macarthur Rd

| R.O.A.R. DATA <br> Reliable, Original \& Authentic Results | PEDS | WEST | SOUTH | EAST |  | PEDS | WEST | SOUTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Time Per | Wilton Rd | Macarthur | Wilton Rd | 101 | Peak Per | Wilton Rd | Macarthur | Wilton Rd | 101 |
| - Mobile. 0418239019 | 1600-1615 |  |  |  | 0 | 0700-0800 | 0 | 0 | 0 | 0 |
|  | 1615-1630 |  |  |  | 0 | 0715-0815 | 0 | 0 | 0 | 0 |
| Client : Traffix | 1630-1645 |  | NOT |  | 0 | 0730-0830 | 0 | 0 | 0 | 0 |
| Job No/Name : 2654 Wollongong Area Traffic Surveys | 1645-1700 |  | REQUIRED |  | 0 | 0745-0845 | 0 | 0 | 0 | 0 |
| Day/Date : Thursday 9th April 2009 | 1700-1715 |  |  |  | 0 | 0800-0900 | 0 | 0 | 0 | 0 |
|  | 1715-1730 |  |  |  | 0 | 0815-0915 | 0 | 0 | 0 | 0 |
|  | 1730-1745 |  |  |  | 0 | 0830-0930 | 0 | 0 | 0 | 0 |
| TC-23 | 1745-1800 |  |  |  | 0 |  |  |  |  |  |
|  | 1800-1815 |  |  |  | 0 |  |  |  |  |  |
|  | 1815-1830 |  |  |  | 0 |  |  |  |  |  |
|  | Per End | 0 | 0 | 0 | 0 | PEAK HR | 0 | 0 | 0 | 0 |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur |  | Wilton Rd |  |  |
| Time Per | I | R | $\underline{L}$ | R | $\underline{L}$ | I | TOT |
| 1600-1615 | 31 | 0 | 1 | 11 | 20 | 40 | 103 |
| 1615-1630 | 25 | 0 | 2 | 7 | 7 | 33 | 74 |
| 1630-1645 | 27 | 4 | 1 | 10 | 17 | 27 | 86 |
| 1645-1700 | 23 | 1 | 2 | 9 | 7 | 36 | 78 |
| 1700-1715 | 28 | 1 | 1 | 15 | 7 | 37 | 89 |
| 1715-1730 | 29 | 0 | 0 | 6 | 9 | 31 | 75 |
| 1730-1745 | 22 | 0 | 0 | 10 | 15 | 43 | 90 |
| 1745-1800 | 24 | 1 | 2 | 9 | 6 | 30 | 72 |
| 1800-1815 | 21 | 4 | 1 | 8 | 9 | 33 | 76 |
| 1815-1830 | 20 | 1 | 1 | 5 | 8 | 28 | 63 |
| Per End | 250 | 12 | 11 | 90 | 105 | 338 | 806 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur |  | Wilton Rd |  |  |
| Time Per | $\underline{T}$ | $\underline{R}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{T}}$ | TOT |
| $1600-1615$ | 1 | 0 | 0 | 0 | 0 | 1 | $\mathbf{2}$ |
| $1615-1630$ | 1 | 0 | 0 | 0 | 1 | 1 | $\mathbf{3}$ |
| $1630-1645$ | 0 | 1 | 0 | 0 | 0 | 0 | $\mathbf{1}$ |
| $1645-1700$ | 3 | 1 | 0 | 0 | 0 | 0 | $\mathbf{4}$ |
| $1700-1715$ | 0 | 0 | 0 | 0 | 0 | 2 | $\mathbf{2}$ |
| $1715-1730$ | 1 | 0 | 0 | 0 | 1 | 1 | $\mathbf{3}$ |
| $1730-1745$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |
| $1745-1800$ | 1 | 0 | 0 | 0 | 0 | 0 | $\mathbf{1}$ |
| $1800-1815$ | 0 | 0 | 0 | 0 | 0 | 0 | $\mathbf{0}$ |
| $1815-1830$ | 1 | 0 | 1 | 1 | 0 | 0 | $\mathbf{3}$ |
| Per End | $\mathbf{8}$ | $\mathbf{2}$ | $\mathbf{1}$ | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{5}$ | $\mathbf{1 9}$ |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur |  | Wilton Rd |  |  |
| Time Per | $\underline{\mathbf{I}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{R}}$ | $\underline{\mathbf{L}}$ | $\underline{\mathbf{I}}$ | TOT |
| $1600-1615$ | 32 | 0 | 1 | 11 | 20 | 41 | $\mathbf{1 0 5}$ |
| $1615-1630$ | 26 | 0 | 2 | 7 | 8 | 34 | $\mathbf{7 7}$ |
| $1630-1645$ | 27 | 5 | 1 | 10 | 17 | 27 | $\mathbf{8 7}$ |
| $1645-1700$ | 26 | 2 | 2 | 9 | 7 | 36 | $\mathbf{8 2}$ |
| $1700-1715$ | 28 | 1 | 1 | 15 | 7 | 39 | $\mathbf{9 1}$ |
| $1715-1730$ | 30 | 0 | 0 | 6 | 10 | 32 | $\mathbf{7 8}$ |
| $1730-1745$ | 22 | 0 | 0 | 10 | 15 | 43 | $\mathbf{9 0}$ |
| $1745-1800$ | 25 | 1 | 2 | 9 | 6 | 30 | $\mathbf{7 3}$ |
| $1800-1815$ | 21 | 4 | 1 | 8 | 9 | 33 | $\mathbf{7 6}$ |
| $1815-1830$ | 21 | 1 | 2 | 6 | 8 | 28 | $\mathbf{6 6}$ |
| Per End | $\mathbf{2 5 8}$ | $\mathbf{1 4}$ | $\mathbf{1 2}$ | $\mathbf{9 1}$ | $\mathbf{1 0 7}$ | $\mathbf{3 4 3}$ | $\mathbf{8 2 5}$ |


| Lights | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur |  | Wilton Rd |  |  |
| Peak Per | I | $\underline{R}$ | $\underline{L}$ | R | $\underline{L}$ | $\underline{\text { I }}$ | TOT |
| 1600-1700 | 106 | 5 | 6 | 37 | 51 | 136 | 341 |
| 1615-1715 | 103 | 6 | 6 | 41 | 38 | 133 | 327 |
| 1630-1730 | 107 | 6 | 4 | 40 | 40 | 131 | 328 |
| 1645-1745 | 102 | 2 | 3 | 40 | 38 | 147 | 332 |
| 1700-1800 | 103 | 2 | 3 | 40 | 37 | 141 | 326 |
| 1715-1815 | 96 | 5 | 3 | 33 | 39 | 137 | 313 |
| 1730-1830 | 87 | 6 | 4 | 32 | 38 | 134 | 301 |


| Heavies | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur |  | Wilton Rd |  |  |
| Peak Per | I | R | $\underline{L}$ | R | $\underline{L}$ | I | TOT |
| 1600-1700 | 5 | 2 | 0 | 0 | 1 | 2 | 10 |
| 1615-1715 | 4 | 2 | 0 | 0 | 1 | 3 | 10 |
| 1630-1730 | 4 | 2 | 0 | 0 | 1 | 3 | 10 |
| 1645-1745 | 4 | 1 | 0 | 0 | 1 | 3 | 9 |
| 1700-1800 | 2 | 0 | 0 | 0 | 1 | 3 | 6 |
| 1715-1815 | 2 | 0 | 0 | 0 | 1 | 1 | 4 |
| 1730-1830 | 2 | 0 | 1 | 1 | 0 | 0 | 4 |


| Combined | WEST |  | SOUTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Wilton Rd |  | Macarthur |  | Wilton Rd |  |  |
| Peak Per | I | R | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | I | TOT |
| 1600-1700 | 111 | 7 | 6 | 37 | 52 | 138 | 351 |
| 1615-1715 | 107 | 8 | 6 | 41 | 39 | 136 | 337 |
| 1630-1730 | 111 | 8 | 4 | 40 | 41 | 134 | 338 |
| 1645-1745 | 106 | 3 | 3 | 40 | 39 | 150 | 341 |
| 1700-1800 | 105 | 2 | 3 | 40 | 38 | 144 | 332 |
| 1715-1815 | 98 | 5 | 3 | 33 | 40 | 138 | 317 |
| 1730-1830 | 89 | 6 | 5 | 33 | 38 | 134 | 305 |

## R.O.A.R. DATA



TOTAL VOLUMES FOR COUNT PERIOD


## Macarthur Rd

## R.O.A.R. DATA

Reliable, Original \& Authentic Results
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## Intersection Details

satellite via satellite
May be incorrect


Wilton Rd


Macarthur Rd


Picton Rd \& Almond St

To
Tim Lewis
at Traffix
your results for

## Wollongong Area Traffic Counts

supplied by
R.O.A.R. DATA Pty. Ltd.
www.roardata.com.au
R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849
Mobile. 0418239019

Client
Job No/Name : 2654 Wollongong Area Traffic Surveys
Day/Date
Traffix

Thursday 9th April 2009

TC-24

| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Picton Rd | Almond St | Picton Rd | TOT |
| 0700-0715 |  |  |  | 0 |
| 0715-0730 |  |  |  | 0 |
| 0730-0745 |  | NOT |  | 0 |
| 0745-0800 |  | REQUIRED |  | 0 |
| 0800-0815 |  |  |  | 0 |
| 0815-0830 |  |  |  | 0 |
| 0830-0845 |  |  |  | 0 |
| 0845-0900 |  |  |  | 0 |
| 0900-0915 |  |  |  | 0 |
| 0915-0930 |  |  |  | 0 |
| Per End | 0 | 0 | 0 | 0 |


| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Picton Rd | Almond St | Picton Rd | TOT |
| 0700-0800 | 0 | 0 | 0 | 0 |
| 0715-0815 | 0 | 0 | 0 | 0 |
| 0730-0830 | 0 | 0 | 0 | 0 |
| 0745-0845 | 0 | 0 | 0 | 0 |
| 0800-0900 | 0 | 0 | 0 | 0 |
| 0815-0915 | 0 | 0 | 0 | 0 |
| 0830-0930 | 0 | 0 | 0 | 0 |


| PEAK HR | 0 | 0 | 0 | 0 |
| :--- | :--- | :--- | :--- | :--- |


| Lights | WEST |  | NORTH |  | EAST |  | Heavies |  | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picton Rd |  | Almond St |  | Picton Rd |  |  |  | Picton Rd |  | Almond St |  | Picton Rd |  |  |
| Time Per | I | $\underline{\underline{L}}$ | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | I | TOT | Time Per | I | $\underline{L}$ | $\underline{\text { R }}$ | $\underline{L}$ | $\underline{\text { R }}$ | I | TOT |
| 0700-0715 | 100 | 13 | 28 | 5 | 1 | 86 | 233 | 0700-0715 | 24 | 1 | 2 | 0 | 0 | 17 | 44 |
| 0715-0730 | 118 | 23 | 18 | 2 | 1 | 95 | 257 | 0715-0730 | 23 | 2 | 0 | 0 | 0 | 10 | 35 |
| 0730-0745 | 140 | 19 | 24 | 3 | 1 | 111 | 298 | 0730-0745 | 22 | 1 | 1 | 0 | 0 | 14 | 38 |
| 0745-0800 | 137 | 10 | 31 | 6 | 1 | 121 | 306 | 0745-0800 | 20 | 3 | 2 | 0 | 0 | 13 | 38 |
| 0800-0815 | 131 | 28 | 30 | 2 | 5 | 127 | 323 | 0800-0815 | 10 | 1 | 1 | 0 | 3 | 28 | 43 |
| 0815-0830 | 131 | 15 | 24 | 4 | 1 | 89 | 264 | 0815-0830 | 22 | 2 | 0 | 0 | 1 | 15 | 40 |
| 0830-0845 | 119 | 12 | 28 | 4 | 2 | 103 | 268 | 0830-0845 | 24 | 1 | 3 | 0 | 0 | 11 | 39 |
| 0845-0900 | 97 | 9 | 22 | 1 | 1 | 83 | 213 | 0845-0900 | 18 | 0 | 0 | 0 | 0 | 27 | 45 |
| 0900-0915 | 90 | 15 | 29 | 0 | 1 | 76 | 211 | 0900-0915 | 17 | 0 | 1 | 0 | 0 | 27 | 45 |
| 0915-0930 | 100 | 23 | 25 | 0 | 0 | 71 | 219 | 0915-0930 | 19 | 3 | 1 | 0 | 0 | 16 | 39 |
| Per End | 1163 | 167 | 259 | 27 | 14 | 962 | 2592 | Per End | 199 | 14 | 11 | 0 | 4 | 178 | 406 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picton Rd |  | Almond St |  | Picton Rd |  |  |
| Time Per | I | $\underline{\underline{L}}$ | $\underline{\mathrm{R}}$ | $\underline{L}$ | $\underline{\text { R }}$ | $\underline{\text { I }}$ | TOT |
| 0700-0715 | 124 | 14 | 30 | 5 | 1 | 103 | 277 |
| 0715-0730 | 141 | 25 | 18 | 2 | 1 | 105 | 292 |
| 0730-0745 | 162 | 20 | 25 | 3 | 1 | 125 | 336 |
| 0745-0800 | 157 | 13 | 33 | 6 | 1 | 134 | 344 |
| 0800-0815 | 141 | 29 | 31 | 2 | 8 | 155 | 366 |
| 0815-0830 | 153 | 17 | 24 | 4 | 2 | 104 | 304 |
| 0830-0845 | 143 | 13 | 31 | 4 | 2 | 114 | 307 |
| 0845-0900 | 115 | 9 | 22 | 1 | 1 | 110 | 258 |
| 0900-0915 | 107 | 15 | 30 | 0 | 1 | 103 | 256 |
| 0915-0930 | 119 | 26 | 26 | 0 | 0 | 87 | 258 |
| Per End | 1362 | 181 | 270 | 27 | 18 | 1140 | 2998 |


| Lights | WEST |  | NORTH |  | $\begin{gathered} \hline \text { EAST } \\ \hline \text { Picton Rd } \end{gathered}$ |  | Heavies |  | WEST |  | NORTH |  | EAST |  | Combined |  | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picton Rd |  |  |  | Picton Rd | Almond St |  | Picton Rd |  | Picton Rd |  | Almond St |  | Picton Rd |  |  |
| Peak Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ |  |  | R | I | TOT | Peak Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ |  |  | $\underline{\text { R }}$ | I | TOT | Peak Per | I | $\underline{\underline{L}}$ | $\underline{R}$ | $\underline{\text { L }}$ | R | I |  |
| 0700-0800 | 495 | 65 | 101 | 16 | 4 | 413 |  |  | 1094 | 0700-0800 | 89 | 7 | 5 | 0 | 0 | 54 | 155 | 0700-0800 | 584 | 72 | 106 | 16 | 4 | 467 | 1249 |
| 0715-0815 | 526 | 80 | 103 | 13 | 8 | 454 | 1184 | 0715-0815 | 75 | 7 | 4 | 0 | 3 | 65 | 154 | 0715-0815 | 601 | 87 | 107 | 13 | 11 | 519 | 1338 |
| 0730-0830 | 539 | 72 | 109 | 15 | 8 | 448 | 1191 | 0730-0830 | 74 | 7 | 4 | 0 | 4 | 70 | 159 | 0730-0830 | 613 | 79 | 113 | 15 | 12 | 518 | 1350 |
| 0745-0845 | 518 | 65 | 113 | 16 | 9 | 440 | 1161 | 0745-0845 | 76 | 7 | 6 | 0 | 4 | 67 | 160 | 0745-0845 | 594 | 72 | 119 | 16 | 13 | 507 | 1321 |
| 0800-0900 | 478 | 64 | 104 | 11 | 9 | 402 | 1068 | 0800-0900 | 74 | 4 | 4 | 0 | 4 | 81 | 167 | 0800-0900 | 552 | 68 | 108 | 11 | 13 | 483 | 1235 |
| 0815-0915 | 437 | 51 | 103 | 9 | 5 | 351 | 956 | 0815-0915 | 81 | 3 | 4 | 0 | 1 | 80 | 169 | 0815-0915 | 518 | 54 | 107 | 9 | 6 | 431 | 1125 |
| 0830-0930 | 406 | 59 | 104 | 5 | 4 | 333 | 911 | 0830-0930 | 78 | 4 | 5 | 0 | 0 | 81 | 168 | 0830-0930 | 484 | 63 | 109 | 5 | 4 | 414 | 1079 |

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Almond St



| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Time Per | Picton Rd | Almond St | Picton Rd | TOT |
| $1600-1615$ |  |  |  | $\mathbf{0}$ |
| $1615-1630$ |  |  |  | $\mathbf{0}$ |
| $1630-1645$ |  | NOT |  | $\mathbf{0}$ |
| $1645-1700$ |  | REQUIRED |  | $\mathbf{0}$ |
| $1700-1715$ |  |  |  | $\mathbf{0}$ |
| $1715-1730$ |  |  |  | $\mathbf{0}$ |
| $1730-1745$ |  |  |  | $\mathbf{0}$ |
| $1745-1800$ |  |  |  | $\mathbf{0}$ |
| $1800-1815$ |  |  |  | $\mathbf{0}$ |
| $1815-1830$ |  |  |  | $\mathbf{0}$ |
| Per End | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{0}$ |


| PEDS | WEST | NORTH | EAST |  |
| :---: | :---: | :---: | :---: | :---: |
| Peak Per | Picton Rd | Almond $\boldsymbol{S} \boldsymbol{t}$ | Picton $\boldsymbol{R} \boldsymbol{d}$ | TOT |
| $1600-1700$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1615-1715$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1630-1730$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1645-1745$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1700-1800$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1715-1815$ | 0 | 0 | 0 | $\mathbf{0}$ |
| $1730-1830$ | 0 | 0 | 0 | $\mathbf{0}$ |


| Lights | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picton Rd |  | Almond St |  | Picton Rd |  |  |
| Time Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 1600-1615 | 171 | 38 | 37 | 5 | 3 | 150 | 404 |
| 1615-1630 | 164 | 32 | 35 | 5 | 4 | 135 | 375 |
| 1630-1645 | 183 | 37 | 28 | 6 | 2 | 151 | 407 |
| 1645-1700 | 164 | 37 | 33 | 4 | 1 | 144 | 383 |
| 1700-1715 | 160 | 35 | 29 | 1 | 1 | 141 | 367 |
| 1715-1730 | 164 | 34 | 35 | 2 | 6 | 111 | 352 |
| 1730-1745 | 148 | 25 | 34 | 4 | 2 | 125 | 338 |
| 1745-1800 | 164 | 35 | 35 | 2 | 9 | 136 | 381 |
| 1800-1815 | 140 | 22 | 33 | 4 | 2 | 114 | 315 |
| 1815-1830 | 159 | 29 | 31 | 2 | 2 | 83 | 306 |
| Per End | 1617 | 324 | 330 | 35 | 32 | 1290 | 3628 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picton Rd |  | Almond St |  | Picton Rd |  |  |
| Time Per | I | $\underline{L}$ | R | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 1600-1615 | 7 | 1 | 3 | 0 | 0 | 9 | 20 |
| 1615-1630 | 5 | 0 | 2 | 0 | 0 | 15 | 22 |
| 1630-1645 | 4 | 3 | 0 | 0 | 0 | 10 | 17 |
| 1645-1700 | 3 | 2 | 0 | 0 | 0 | 13 | 18 |
| 1700-1715 | 7 | 2 | 2 | 0 | 1 | 3 | 15 |
| 1715-1730 | 10 | 1 | 1 | 0 | 0 | 7 | 19 |
| 1730-1745 | 4 | 0 | 0 | 0 | 0 | 8 | 12 |
| 1745-1800 | 5 | 1 | 0 | 0 | 0 | 9 | 15 |
| 1800-1815 | 5 | 1 | 0 | 0 | 0 | 5 | 11 |
| 1815-1830 | 8 | 2 | 0 | 0 | 0 | 4 | 14 |
| Per End | 58 | 13 | 8 | 0 | 1 | 83 | 163 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picton Rd |  | Almond St |  | Picton Rd |  |  |
| Time Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 1600-1615 | 178 | 39 | 40 | 5 | 3 | 159 | 424 |
| 1615-1630 | 169 | 32 | 37 | 5 | 4 | 150 | 397 |
| 1630-1645 | 187 | 40 | 28 | 6 | 2 | 161 | 424 |
| 1645-1700 | 167 | 39 | 33 | 4 | 1 | 157 | 401 |
| 1700-1715 | 167 | 37 | 31 | 1 | 2 | 144 | 382 |
| 1715-1730 | 174 | 35 | 36 | 2 | 6 | 118 | 371 |
| 1730-1745 | 152 | 25 | 34 | 4 | 2 | 133 | 350 |
| 1745-1800 | 169 | 36 | 35 | 2 | 9 | 145 | 396 |
| 1800-1815 | 145 | 23 | 33 | 4 | 2 | 119 | 326 |
| 1815-1830 | 167 | 31 | 31 | 2 | 2 | 87 | 320 |
| Per End | 1675 | 337 | 338 | 35 | 33 | 1373 | 3791 |


| Lights | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picton Rd |  | Almond St |  | Picton Rd |  |  |
| Peak Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{\text { R }}$ | I | TOT |
| 1600-1700 | 682 | 144 | 133 | 20 | 10 | 580 | 1569 |
| 1615-1715 | 671 | 141 | 125 | 16 | 8 | 571 | 1532 |
| 1630-1730 | 671 | 143 | 125 | 13 | 10 | 547 | 1509 |
| 1645-1745 | 636 | 131 | 131 | 11 | 10 | 521 | 1440 |
| 1700-1800 | 636 | 129 | 133 | 9 | 18 | 513 | 1438 |
| 1715-1815 | 616 | 116 | 137 | 12 | 19 | 486 | 1386 |
| 1730-1830 | 611 | 111 | 133 | 12 | 15 | 458 | 1340 |


| Heavies | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picton Rd |  | Almond St |  | Picton Rd |  |  |
| Peak Per | $\underline{I}$ | $\underline{L}$ | R | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 1600-1700 | 19 | 6 | 5 | 0 | 0 | 47 | 77 |
| 1615-1715 | 19 | 7 | 4 | 0 | 1 | 41 | 72 |
| 1630-1730 | 24 | 8 | 3 | 0 | 1 | 33 | 69 |
| 1645-1745 | 24 | 5 | 3 | 0 | 1 | 31 | 64 |
| 1700-1800 | 26 | 4 | 3 | 0 | 1 | 27 | 61 |
| 1715-1815 | 24 | 3 | 1 | 0 | 0 | 29 | 57 |
| 1730-1830 | 22 | 4 | 0 | 0 | 0 | 26 | 52 |


| Combined | WEST |  | NORTH |  | EAST |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picton Rd |  | Almond St |  | Picton Rd |  |  |
| Peak Per | I | $\underline{L}$ | $\underline{R}$ | $\underline{L}$ | $\underline{R}$ | I | TOT |
| 1600-1700 | 701 | 150 | 138 | 20 | 10 | 627 | 1646 |
| 1615-1715 | 690 | 148 | 129 | 16 | 9 | 612 | 1604 |
| 1630-1730 | 695 | 151 | 128 | 13 | 11 | 580 | 1578 |
| 1645-1745 | 660 | 136 | 134 | 11 | 11 | 552 | 1504 |
| 1700-1800 | 662 | 133 | 136 | 9 | 19 | 540 | 1499 |
| 1715-1815 | 640 | 119 | 138 | 12 | 19 | 515 | 1443 |
| 1730-1830 | 633 | 115 | 133 | 12 | 15 | 484 | 1392 |

## R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019


## R.O.A.R. DATA

Reliable, Original \& Authentic Results
Ph.88196847, Fax 88196849, Mob.0418-239019

Intersection Details
Obtained via satellite


Picton Rd

Location：
TCS：
TCS：
Date：
Conditions：
Surveyor：
Surveyor：
Thursday 9th April 2009

Time Period：AM（7．00－9．00am）

| Time | Movement |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 |  |  | 2 |  |  | 3 |  |  | 4 |  |  | 5 |  |  | 6 |  |  | 7 |  |  | 8 |  |  | 9 |  |  | 10 |  |  | 11 |  |  | 12 |  |  |
|  | $\begin{aligned} & \text { 喜 } \\ & \hline \end{aligned}$ | $\bar{\circ}$ |  | 菏 | 厄̄ |  | $\begin{aligned} & \text { 宮 } \\ & \hline \end{aligned}$ | 厄َ0 |  | $\begin{aligned} & \text { 宮 } \\ & \hline \end{aligned}$ | 厄్రై |  |  | 厄ु |  | 菬 | ©َ |  | 言 | 응 |  | 菬 | 응 |  | $\begin{aligned} & \text { 呂 } \\ & \hline \end{aligned}$ | $\overline{\text { oj }}$ |  | $\begin{aligned} & \text { 苗 } \\ & \hline \end{aligned}$ | 厄̈ |  | 德 | 흥 |  | $\begin{aligned} & \text { 菏 } \\ & \hline \end{aligned}$ | 厄ס |  |
| 7．00－7．15 | 13 | 4 | 4 | 332 | 2 | 27 | 28 | 0 | 5 | 15 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7．15－7．30 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 10 | 0 | 0 | 219 | 1 | 21 | 5 | 6 | 2 | 5 | 4 | 3 | 0 | 0 | 1 | 1 | 5 | 0 |
| 7．30－7．45 | 5 | 1 | 0 | 452 | 0 | 29 | 8 | 0 | 3 | 4 | 0 | 0 | 0 | 0 | 1 | 3 | 0 | 2 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7．45－8．00 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 11 | 0 | 0 | 311 | 2 | 25 | 8 | 3 | 0 | 1 | 3 | 2 | 1 | 0 | 1 | 1 | 1 | 0 |
| 8．00－8．15 | 6 | 4 | 3 | 585 | 0 | 18 | 5 | 0 | 1 | 2 | 0 | 0 | 1 | 0 | 0 | 2 | 0 | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8．15－8．30 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 5 | 0 | 1 | 297 | 4 | 15 | 14 | 8 | 2 | 5 | 9 | 0 | 1 | 0 | 1 | 1 | 4 | 3 |
| 8．30－8．45 | 3 | 7 | 3 | 684 | 4 | 20 | 10 | 0 | 1 | 4 | 0 | 2 | 0 | 0 | 1 | 0 | 0 | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 8．45－9．00 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | 7 | 0 | 1 | 246 | 9 | 22 | 3 | 4 | 0 | 2 | 7 | 2 | 1 | 0 | 0 | 4 | 1 | 2 |
| Equiv Hourly | 27 | 16 | 10 | 2053 | 6 | 94 | 51 | 0 | 10 | 25 | 0 | 2 | 1 | 0 | 2 | 7 | 0 | 2 | 33 | 0 | 2 | 1073 | 16 | 83 | 30 | 21 | 4 | 13 | 23 | 7 | 3 | 0 | 3 | 7 | 11 | 5 |
| Combined Heavy | 27 | 26 |  | 2053 | 100 |  | 51 | 10 |  | 25 | 2 |  | 1 | 2 |  | 7 | 2 |  | 33 | 2 |  | 1073 | 99 |  | 30 | 25 |  | 13 | 30 |  | 3 | 3 |  | 7 | 16 |  |


10$\uparrow$
11
12
Recycling Road
9
87

\}
6

4
$1 \prod_{2} 3$

Springhill Road

SIDRA outputs

## attachment d1

existing intersection performance

## SIDRA -- <br> INTERSECTION

## Movement Summary

## Appin Rd - George St (Appin East Colliery Exit)

## Existing - AM

Signalised - Fixed time

```
Cycle Time = 50 seconds
```

Vehicle Movements

| Mov ID Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| ET T | 613 | 5.5 | 0.557 | 18.3 | LOS B | 68 | 0.82 | 0.76 | 46.6 |
| Approach | 613 | 5.5 | 0.557 | 18.3 | Los B | 68 | 0.82 | 0.76 | 46.6 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 362 | 3.9 | 0.264 | 12.5 | LOS A | 32 | 0.75 | 0.61 | 43.4 |
| Approach | 362 | 3.9 | 0.264 | 12.5 | Los A | 32 | 0.75 | 0.61 | 43.4 |
| George St (south) |  |  |  |  |  |  |  |  |  |
| SR R | 48 | 69.4 | 0.099 | 18.8 | LOS B | 13 | 0.65 | 0.73 | 34.1 |
| Approach | 49 | 69.4 | 0.099 | 18.8 | LOS B | 13 | 0.65 | 0.73 | 34.1 |
| All Vehicles | 1024 | 8.0 | 0.557 | 16.3 | LOS B | 68 | 0.79 | 0.71 | 44.4 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: EX - AM
T:\2007\07267\Survey Results \& Modelling\SIDRA Files\Appin Rd \& George St (Appin East Colliery-Exit).aap Processed Apr 21, 2009 09:42:25AM

A1835, Traffix, Small Office
Produced by SIDRA Intersection 3.2.2.1563
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www.sidrasolutions.com

## SIDRA -- <br> INTERSECTION

## Movement Summary

## Appin Rd - George St (Appin East Colliery Exit)

## Existing - PM

Signalised - Fixed time

Cycle Time $=\mathbf{4 0}$ seconds
Vehicle Movements

| Mov ID Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow/ } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| ET T | 406 | 2.2 | 0.651 | 21.6 | LOS B | 47 | 0.95 | 0.81 | 43.3 |
| Approach | 406 | 2.2 | 0.651 | 21.6 | LOS B | 47 | 0.95 | 0.81 | 43.3 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 531 | 0.9 | 0.684 | 18.1 | LOS B | 49 | 0.98 | 0.88 | 39.7 |
| Approach | 530 | 0.9 | 0.684 | 18.1 | LOS B | 49 | 0.98 | 0.88 | 39.7 |
| George St (south) |  |  |  |  |  |  |  |  |  |
| SR R | 27 | 60.7 | 0.043 | 13.7 | LOS A | 5 | 0.53 | 0.69 | 36.5 |
| Approach | 28 | 60.7 | 0.043 | 13.7 | LOS A | 5 | 0.53 | 0.69 | 36.5 |
| All Vehicles | 964 | 3.2 | 0.684 | 19.5 | Los B | 49 | 0.95 | 0.85 | 40.9 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


SIDRA SOLUTIONS

Site: EX - PM
$\mathrm{T}: \backslash 2007 \backslash 07267 \backslash$ Survey Results \& Modelling\SIDRA Files\Appin Rd \& George St (Appin East Colliery-Exit).aap Processed Apr 21, 2009 09:39:49AM

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Produced by SIDRA Intersection 3.2.2.1563
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www.sidrasolutions.com

## SIDRA -- <br> INTERSECTION

## Movement Summary

## Appin Rd / Princes Hwy Off-Ramp

Existing-AM

Two-way stop
Vehicle Movements

| Mov 1D | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 581 | 7.1 | 0.312 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 581 | 7.1 | 0.312 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| Merge Lane |  |  |  |  |  |  |  |  |  |  |
| Merge | T | 51 | 10.0 | 0.068 | 13.3 | LOS A | 2 | 0.50 | 0.78 | 56.4 |
| Approach |  | 50 | 10.0 | 0.068 | 13.3 | LOS A | 2 | 0.50 | 0.78 | 56.4 |
| Princes Hwy Off-Ramp (north) |  |  |  |  |  |  |  |  |  |  |
| NR | R | 51 | 10.0 | 0.104 | 18.5 | LOS B | 4 | 0.52 | 0.94 | 51.9 |
| Approach |  | 50 | 10.0 | 0.104 | 18.5 | LOS B | 4 | 0.52 | 0.94 | 51.9 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 7 | 28.6 | 0.017 | 12.6 | LOS A | 0 | 0.00 | 0.73 | 58.9 |
| WT | T | 334 | 10.8 | 0.171 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 341 | 11.1 | 0.171 | 0.3 | LOS A |  | 0.00 | 0.02 | 79.4 |
| All Vehicles |  | 1022 | 8.7 | 0.312 | 1.6 | Applicable | 4 | 0.05 | 0.09 | 76.3 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement

Site: EX - AM
T:\2007\07267\Survey Results \& Modelling\SIDRA Files\Appin Rd \& Princes Hwy Off-ramp.aap Processed Apr 20, 2009 08:33:16PM

A1835, Traffix, Small Office
Produced by SIDRA Intersection 3.2.2.1563
Copyright © 2000-2008 Akcelik and Associates Pty Ltd
www.sidrasolutions.com

## Movement Summary

## Appin Rd / Princes Hwy Off-Ramp

Existing - PM

Two-way stop

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | $95 \%$ <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 280 | 5.7 | 0.149 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 280 | 5.7 | 0.149 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| Merge Lane |  |  |  |  |  |  |  |  |  |  |
| Merge | T | 78 | 0.0 | 0.069 | 10.8 | LOS A | 2 | 0.32 | 0.66 | 58.0 |
| Approach |  | 78 | 0.0 | 0.069 | 10.8 | $\operatorname{LOSA}$ | 2 | 0.32 | 0.66 | 58.0 |
| Princes Hwy Off-Ramp (north) |  |  |  |  |  |  |  |  |  |  |
| NR | R | 78 | 0.0 | 0.225 | 22.3 | LOS B | 8 | 0.67 | 1.01 | 47.5 |
| Approach |  | 78 | 0.0 | 0.226 | 22.3 | Los B | 8 | 0.67 | 1.01 | 47.5 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 4 | 25.0 | 0.028 | 12.4 | LOS A | 0 | 0.00 | 0.73 | 58.9 |
| WT | T | 577 | 4.0 | 0.278 | 0.0 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 581 | 4.1 | 0.278 | 0.1 | $\operatorname{Los} A$ |  | 0.00 | 0.01 | 79.8 |
| All Vehicles |  | 1017 | 3.9 | 0.278 | 2.6 | Not <br> Applicable | 8 | 0.08 | 0.13 | 74.0 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement


SIDRA SOLUTIONS

Site: EX - PM
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## SIDRA

Movement Summary
Appin Rd - Sherrif Rd (Appin East Colliery Entry)
Existing - AM

Two-way stop
Vehicle Movements

| Mov ID Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 38 | 94.7 | 0.180 | 14.5 | LOS B | 0 | 0.00 | 0.72 | 57.6 |
| ET T | 613 | 5.5 | 0.180 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 651 | 10.8 | 0.180 | 0.8 | Los A |  | 0.00 | 0.04 | 78.6 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 459 | 17.6 | 0.131 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 459 | 17.6 | 0.131 | 0.0 | Los A |  | 0.00 | 0.00 | 80.0 |
| All Vehicles | 1110 | 13.6 | 0.180 | 0.5 | Applicable | 0 | 0.00 | 0.02 | 79.2 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow
$* x=1.00$ due to minimum capacity
Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: EX - AM
T: \2007\07267\Survey Results \& Modelling \SIDRA Files\Appin Rd \& Sherrif Rd (Appin East Colliery).aap Processed Apr 17, 2009 03:23:17PM

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## SIDRA INTERSECTION <br> Movement Summary <br> Appin Rd - Sherrif Rd (Appin East Colliery Entry)

Existing - PM

Two-way stop
Vehicle Movements

| Mov ID Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 9 | 100.0 | 0.110 | 14.8 | LOS B | 0 | 0.00 | 0.72 | 52.4 |
| ET T | 406 | 2.2 | 0.110 | 0.0 | Los A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 415 | 4.3 | 0.110 | 0.3 | Los A |  | 0.00 | 0.02 | 79.2 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 585 | 6.7 | 0.157 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 585 | 6.7 | 0.156 | 0.0 | Los A |  | 0.00 | 0.00 | 80.0 |
| Sherrif Rd (south) |  |  |  |  |  |  |  |  |  |
| 32 R | 1 | 0.0 | 0.001 | 8.3 | LOS A | 0 | 0.00 | 0.68 | 48.8 |
| Approach | 1 | 0.0 | 0.001 | 8.3 | LOS A |  | 0.00 | 0.68 | 48.8 |
| All Vehicles | 1001 | 5.7 | 0.157 | 0.1 | Applicable | 0 | 0.00 | 0.01 | 79.6 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


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## SIDRA -- <br> INTERSECTION

## Movement Summary

## Appin Rd - West Cliff Colliery Access

## Existing - AM

Two-way stop

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 627 | 1.3 | 0.324 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 100.0 |
| ER | R | 45 | 67.4 | 0.101 | 21.6 | LOS B | 6 | 0.56 | 0.83 | 54.7 |
| Approach |  | 673 | 5.8 | 0.324 | 1.5 | LOS A | 6 | 0.04 | 0.06 | 95.8 |
| Median Storage |  |  |  |  |  |  |  |  |  |  |
| Store | T | 38 | 86.8 | 0.202 | 22.0 | LOS B | 11 | 0.80 | 0.90 | 27.0 |
| Approach |  | 38 | 86.8 | 0.202 | 22.0 | LOS B | 11 | 0.80 | 0.90 | 27.0 |
| West Cliff Access (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 51 | 66.7 | 0.338 | 30.7 | LOS C | 22 | 0.67 | 1.07 | 42.6 |
| NR | R | 38 | 86.8 | 0.339 | 32.3 | LOS C | 22 | 0.67 | 1.06 | 35.7 |
| Approach |  | 89 | 75.3 | 0.338 | 31.4 | Los C | 22 | 0.67 | 1.07 | 40.1 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 58 | 15.8 | 0.034 | 13.5 | LOS A | 0 | 0.00 | 0.76 | 63.3 |
| WT | T | 359 | 3.1 | 0.094 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 100.0 |
| Approach |  | 416 | 4.8 | 0.094 | 1.8 | $\operatorname{LOS~A}$ |  | 0.00 | 0.10 | 94.2 |
| All Vehicles |  | 1216 | 13.1 | 0.339 | 4.4 | Not Applicable | 22 | 0.10 | 0.17 | 85.0 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## $\rho$

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## SIDRA - - <br> INTERSECTION

## Movement Summary

## Appin Rd - West Cliff Colliery Access

## Existing - PM

Two-way stop

## Vehicle Movements



Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

[^6]
## SIDRA

INTERSECTION

## Movement Summary

## Appin Rd / Wilton Rd

## Existing - AM

Two-way stop

## Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | $\begin{aligned} & \text { Deg of } \\ & \text { Satn } \\ & (v / c) \end{aligned}$ | Aver Delay (sec) | Level of Service | $95 \%$ <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (south) |  |  |  |  |  |  |  |  |  |
| ST T | 72 | 2.8 | 0.198 | 18.3 | LOS B | 7 | 0.70 | 1.00 | 18.4 |
| SR R | 44 | 4.5 | 0.146 | 19.9 | LOS B | 5 | 0.70 | 1.00 | 34.6 |
| Approach | 115 | 3.5 | 0.198 | 18.9 | LOS B | 7 | 0.70 | 1.00 | 27.8 |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 47 | 2.1 | 0.338 | 6.5 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.61 | 43.3 |
| ER R | 573 | 1.9 | 0.338 | 6.6 | LOS A | 0 | 0.00 | 0.63 | 43.2 |
| Approach | 620 | 1.9 | 0.338 | 6.6 | LOS A |  | 0.00 | 0.63 | 43.2 |
| Appin Rd (north) |  |  |  |  |  |  |  |  |  |
| NL L | 327 | 2.8 | 0.295 | 5.9 | LOS A | 12 | 0.16 | 0.51 | 35.2 |
| NT T | 51 | 14.0 | 0.179 | 21.9 | $\operatorname{LOS~B}$ | 7 | 0.74 | 1.00 | 33.9 |
| Approach | 377 | 4.2 | 0.295 | 8.0 | LOS A | 12 | 0.24 | 0.57 | 34.8 |
| All Vehicles | 11.12 | 2.9 | 0.338 | 8.4 | Not <br> Applicable | 12 | 0.15 | 0.65 | 39.7 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement

## J

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## SIDRA --- <br> INTERSECTION

## Movement Summary

## Appin Rd / Wilton Rd

Existing - PM

Two-way stop

## Vehicle Movements

| Mov ID | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (south) |  |  |  |  |  |  |  |  |  |  |
| ST | T | 95 | 1.1 | 0.172 | 13.6 | LOS A | 6 | 0.56 | 0.96 | 22.0 |
| SR | R | 60 | 1.7 | 0.144 | 15.8 | Los B | 5 | 0.59 | 1.00 | 36.9 |
| Approach |  | 155 | 1.3 | 0.172 | 14.4 | Los A | 6 | 0.57 | 0.97 | 31.2 |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 73 | 2.7 | 0.235 | 6.5 | LOS A | 0 | 0.00 | 0.61 | 43.3 |
| ER | R | 357 | 2.0 | 0.235 | 6.6 | LOS A | 0 | 0.00 | 0.63 | 43.2 |
| Approach |  | 429 | 2.1 | 0.234 | 6.6 | Los A |  | 0.00 | 0.63 | 43.2 |
| Appin Rd (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 496 | 0.8 | 0.446 | 6.1 | LOS A | 22 | 0.23 | 0.51 | 34.6 |
| NT | T | 88 | 1.1 | 0.171 | 14.2 | LOS A | 6 | 0.58 | 0.98 | 38.0 |
| Approach |  | 584 | 0.9 | 0.446 | 7.3 | LOS A | 22 | 0.28 | 0.58 | 35.6 |
| All Vehicles |  | 1168 | 1.4 | 0.446 | 8.0 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 22 | 0.22 | 0.65 | 38.8 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## J <br> SIDRA SOLUTIONS

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## SIDRA <br> INTERSECTION

## Movement Summary

## Masters Rd / Springhill Rd

Existing (2008) - AM
Signalised - Fixed time Cycle Time $=\mathbf{1 0 0}$ seconds
Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow/ } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 495 | 0.0 | 0.163 | 13.0 | LOS A | 25 | 0.22 | 0.73 | 56.2 |
| 2 | T | 2386 | 0.0 | 0.832 | 27.1 | LOS B | 248 | 0.93 | 0.89 | 42.8 |
| Approach |  | 2881 | 0.0 | 0.832 | 24.7 | LOS B | 248 | 0.81 | 0.86 | 44.6 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 644 | 0.0 | 0.337 | 16.6 | LOS B | 78 | 0.65 | 0.56 | 52.2 |
| 9 | R | 282 | 0.0 | 0.844 | 70.0 | LOS E | 47 | 1.00 | 0.91 | 24.4 |
| Approach |  | 926 | 0.0 | 0.844 | 32.8 | LOS C | 78 | 0.76 | 0.67 | 38.9 |
| Masters Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 517 | 0.0 | 0.470 | 34.7 | LOS C | 97 | 0.77 | 0.81 | 37.6 |
| 12 | R | 730 | 0.0 | 0.485 | 44.4 | LOS D | 82 | 0.89 | 0.82 | 32.8 |
| Approach |  | 1247 | 0.0 | 0.485 | 40.4 | LOS C | 97 | 0.84 | 0.82 | 34.7 |
| All Vehicles |  | 5054 | 0.0 | 0.844 | 30.1 | Los C | 248 | 0.81 | 0.82 | 40.6 |

Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathrm{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P5 | 50 | 44.2 | LOS E | 0 | 0.94 | 0.94 |
| All Peds | 50 | 44.2 | LOS D | 0 | 0.94 | 0.94 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue

## SIDRA -- <br> INTERSECTION <br> Movement Summary <br> Masters Rd / Springhill Rd <br> Existing (2008) - PM

Signalised - Fixed time Cycle Time $=\mathbf{1 0 0}$ seconds
Vehicle Movements

| Mov ID | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 817 | 0.0 | 0.328 | 18.4 | LOS B | 70 | 0.45 | 0.78 | 50.0 |
| 2 | T | 1256 | 0.0 | 0.631 | 30.0 | Los C | 127 | 0.90 | 0.78 | 40.8 |
| Approach |  | 2073 | 0.0 | 0.631 | 25.4 | Los B | 127 | 0.72 | 0.78 | 44.0 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 1164 | 0.0 | 0.878 | 43.6 | LOS D | 219 | 1.00 | 1.01 | 33.3 |
| 9 | R | 1029 | 0.0 | 0.880 | 63.7 | LOS E | 140 | 1.00 | 1.02 | 26.0 |
| Approach |  | 2193 | 0.0 | 0.879 | 53.0 | LOS D | 219 | 1.00 | 1.01 | 29.5 |
| Masters Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 188 | 0.0 | 0.127 | 22.9 | LOS B | 30 | 0.51 | 0.75 | 45.9 |
| 12 | R | 680 | 0.0 | 0.452 | 44.0 | LOS D | 76 | 0.88 | 0.82 | 33.0 |
| Approach |  | 868 | 0.0 | 0.452 | 39.4 | LOS C | 76 | 0.80 | 0.80 | 35.1 |
| All Vehicles |  | 5134 | 0.0 | 0.880 | 39.6 | Los C | 219 | 0.85 | 0.88 | 35.1 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathrm{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P5 | 50 | 44.2 | LOS E | 0 | 0.94 | 0.94 |
| All Peds | 50 | 44.2 | LOS D | 0 | 0.94 | 0.94 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue

## SIDRA <br> INTERSECTION

Movement Summary
Mt Ousley Rd / Southern Fwy
Existing - AM

Two-way stop
Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | $95 \%$ <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Southern Fwy (south) |  |  |  |  |  |  |  |  |  |
| ST $T$ | 1453 | 12.3 | 0.402 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 1453 | 12.3 | 0.402 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| Mt Ousley Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 135 | 0.7 | 0.073 | 7.6 | LOS A\# | 2\# | 0.00 | 0.60 | 49.8 |
| ER $\quad$ R | 206 | 1.9 | 1.776 | 768.1 | LOS F | 453 | 1.00 | 3.88 | 3.0 |
| Approach | 341 | 1.5 | 1.776 | 467.0 | LOS F | 453 | 0.60 | 2.58 | 4.7 |
| Mt Ousley Rd (north) |  |  |  |  |  |  |  |  |  |
| NL L | 389 | 2.8 | 0.214 | 11.2 | LOS A\# | 5\# | 0.00 | 0.69 | 58.8 |
| NT T | 1876 | 7.9 | 0.506 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 2265 | 7.1 | 0.506 | 1.9 | LOS A |  | 0.00 | 0.12 | 75.4 |
| All Vehicles | 4059 | 8.5 | 1.776 | 40.3 | Not <br> Applicable | 453 | 0.05 | 0.28 | 34.4 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA INTERSECTION

## Movement Summary

Mt Ousley Rd / Southern Fwy
Existing - PM

Two-way stop
Vehicle Movements


Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## 

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## SIDRA <br> INTERSECTION

## Movement Summary

## Piston Rd / Almond St (Wilton Rd)

## Existing -AM

Two-way stop

## Vehicle Movements



Symbols which may appear in this table:
Following Degree of Saturation
\# $\mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## J

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## SIDRA -- <br> INTERSECTION

## Movement Summary

## Picton Rd / Almond St (Wilton Rd)

## Existing - PM

Two-way stop

## Vehicle Movements



Symbols which may appear in this table:
Following Degree of Saturation
\# $\mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## J

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## SIDRA -- <br> INTERSECTION

## Movement Summary

Springhill Rd / Port Kembla Rd
Existing (2008) - AM
Signalised - Fixed time $\quad$ Cycle Time $=50$ seconds
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Port Kembla Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 31 | 0.0 | 0.046 | 19.0 | LOS B | 5 | 0.68 | 0.71 | 37.9 |
| 3 | R | 8 | 0.0 | 0.036 | 29.2 | LOS C | 2 | 0.91 | 0.66 | 30.4 |
| Approach |  | 39 | 0.0 | 0.046 | 21.1 | LOS B | 5 | 0.72 | 0.70 | 36.3 |
| Springhill Rd (east) |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 12 | 0.0 | 0.204 | 18.2 | LOS B | 26 | 0.69 | 0.76 | 38.8 |
| 5 | T | 465 | 0.0 | 0.204 | 10.7 | LOS A | 26 | 0.69 | 0.56 | 50.3 |
| Approach |  | 477 | 0.0 | 0.204 | 10.9 | LOS A | 26 | 0.69 | 0.56 | 50.0 |
| Sprinhill Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 11 | T | 1040 | 0.0 | 0.267 | 4.1 | LOS A\# | 7\# | 0.00 | 0.32 | 70.8 |
| 12 | R | 38 | 0.0 | 0.171 | 32.9 | LOS C | 10 | 0.93 | 0.73 | 35.1 |
| Approach |  | 1078 | 0.0 | 0.267 | 5.1 | LOS A | 10 | 0.03 | 0.33 | 68.8 |
| All Vehicles |  | 1594 | 0.0 | 0.267 | 7.2 | LOS A | 26 | 0.25 | 0.41 | 60.7 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 50 | 13.0 | LOS B | 0 | 0.72 | 0.72 |
| All Peds | 50 | $\mathbf{1 3 . 0}$ | LOS A | $\mathbf{0}$ | $\mathbf{0 . 7 2}$ | $\mathbf{0 . 7 2}$ |

Symbols which may appear in this table:
Following Degree of Saturation
$\# \mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue

## SIDRA -- <br> INTERSECTION

## Movement Summary

## Springhill Rd / Port Kembla Rd

Existing (2008) - PM

Signalised - Fixed time $\quad$ Cycle Time $=50$ seconds

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Port Kembla Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 67 | 0.0 | 0.100 | 19.3 | LOS B | 12 | 0.69 | 0.74 | 37.8 |
| 3 | R | 28 | 0.0 | 0.126 | 29.7 | LOS C | 7 | 0.92 | 0.71 | 30.1 |
| Approach |  | 95 | 0.0 | 0.126 | 22.4 | LOS B | 12 | 0.76 | 0.73 | 35.3 |
| Springhill Rd (east) |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 5 | 0.0 | 0.446 | 19.5 | LOS B | 56 | 0.78 | 0.81 | 37.9 |
| 5 | T | 1041 | 0.0 | 0.447 | 12.1 | LOS A | 56 | 0.78 | 0.66 | 49.0 |
| Approach |  | 1046 | 0.0 | 0.447 | 12.1 | LOS A | 56 | 0.78 | 0.66 | 48.9 |
| Sprinhill Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 11 | T | 569 | 0.0 | 0.146 | 4.1 | LOS A\# | 4\# | 0.00 | 0.32 | 70.8 |
| 12 | R | 36 | 0.0 | 0.162 | 32.8 | LOS C | 9 | 0.93 | 0.72 | 35.2 |
| Approach |  | 605 | 0.0 | 0.162 | 5.8 | $\operatorname{LOS~A}$ | 9 | 0.06 | 0.34 | 67.4 |
| All Vehicles |  | 1746 | 0.0 | 0.447 | 10.5 | $\operatorname{LOS} A$ | 56 | 0.53 | 0.56 | 52.9 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathrm{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 50 | 13.0 | LOS B | 0 | 0.72 | 0.72 |
| All Peds | $\mathbf{5 0}$ | $\mathbf{1 3 . 0}$ | LOS A | $\mathbf{0}$ | $\mathbf{0 . 7 2}$ | $\mathbf{0 . 7 2}$ |

## Symbols which may appear in this table:

Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue

## SIDRA -- <br> INTERSECTION

## Movement Summary

## Springhill Rd / Recycling Rd

Existing - AM

Signalised - Actuated isolated $\quad$ Cycle Time $=\mathbf{9 0}$ seconds

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 56 | 49.1 | 0.908 | 43.5 | LOS D | 267 | 0.98 | 0.96 | 30.5 |
| ST | T | 2266 | 4.6 | 0.905 | 30.6 | LOS C | 266 | 0.98 | 0.95 | 40.4 |
| SR | R | 64 | 16.9 | 0.392 | 55.8 | LOS D | 28 | 0.95 | 0.76 | 25.3 |
| Approach |  | 2386 | 6.0 | 0.905 | 31.6 | Los C | 267 | 0.98 | 0.95 | 39.6 |
| Bluescope Steel (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 28 | 7.1 | 0.158 | 49.1 | LOS D | 11 | 0.92 | 0.71 | 23.5 |
| ET | T | 3 | 66.7 | 0.022 | 41.7 | LOS C | 2 | 0.91 | 0.58 | 26.1 |
| ER | R | 9 | 22.2 | 0.072 | 51.0 | LOS D | 4 | 0.93 | 0.67 | 23.2 |
| Approach |  | 40 | 15.0 | 0.158 | 48.9 | LOS D | 11 | 0.92 | 0.69 | 23.6 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 37 | 5.4 | 0.027 | 13.9 | LOS A | 2 | 0.17 | 0.96 | 53.8 |
| NT | T | 1234 | 8.4 | 0.488 | 19.2 | LOS B | 110 | 0.73 | 0.64 | 49.5 |
| NR | R | 58 | 44.8 | 0.412 | 57.4 | LOS E | 31 | 0.95 | 0.76 | 25.1 |
| Approach |  | 1328 | 9.9 | 0.488 | 20.7 | LOS B | 110 | 0.73 | 0.65 | 47.9 |
| Recycling Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 45 | 69.6 | 0.411 | 53.4 | LOS D | 32 | 0.95 | 0.75 | 22.9 |
| WT | T | 6 | 50.0 | 0.411 | 45.4 | LOS D | 32 | 0.95 | 0.73 | 25.1 |
| WR | R | 24 | 70.8 | 0.250 | 54.2 | LOS D | 15 | 0.95 | 0.71 | 22.7 |
| Approach |  | 76 | 68.4 | 0.412 | 53.0 | LOS D | 32 | 0.95 | 0.74 | 23.0 |
| All Vehicles |  | 3830 | 8.7 | 0.908 | 28.4 | Los B | 267 | 0.89 | 0.84 | 41.2 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SP | 11 | 46.7 | LOS E | 0 | 0.94 | 0.94 |
| EP | 21 | 39.2 | LOS D | 0 | 0.93 | 0.93 |
| WP | 21 | 39.2 | LOS D | 0 | 0.93 | 0.93 |

## SIDRA -- INTERSECTION

## Movement Summary

## Springhill Rd / Recycling Rd

## Existing - PM

Signalised - Actuated isolated
Cycle Time $=\mathbf{8 3}$ seconds

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | $95 \%$ <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 11 | 20.0 | 0.642 | 31.0 | LOS C | 138 | 0.80 | 0.86 | 36.9 |
| ST | T | 1669 | 1.5 | 0.634 | 19.3 | LOS B | 138 | 0.80 | 0.71 | 49.4 |
| SR | R | 28 | 7.1 | 0.219 | 53.7 | LOS D | 11 | 0.95 | 0.72 | 25.9 |
| Approach |  | 1707 | 1.7 | 0.634 | 19.9 | LOS B | 138 | 0.81 | 0.71 | 48.7 |
| Bluescope Steel (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 64 | 16.9 | 0.362 | 46.9 | LOS D | 26 | 0.94 | 0.75 | 24.0 |
| ET | T | 6 | 50.0 | 0.038 | 38.0 | LOS C | 3 | 0.90 | 0.60 | 27.1 |
| ER | R | 37 | 5.4 | 0.286 | 49.2 | LOS D | 14 | 0.96 | 0.72 | 23.5 |
| Approach |  | 108 | 14.8 | 0.362 | 47.2 | LOS D | 26 | 0.95 | 0.73 | 24.0 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 9 | 22.2 | 0.007 | 14.8 | LOS B | 0 | 0.16 | 0.95 | 54.0 |
| NT | T | 2269 | 1.7 | 0.857 | 23.9 | LOS B | 214 | 0.94 | 0.87 | 45.3 |
| NR | R | 14 | 46.2 | 0.129 | 55.1 | LOS D | 7 | 0.94 | 0.69 | 25.8 |
| Approach |  | 2292 | 2.0 | 0.857 | 24.0 | LOS B | 214 | 0.94 | 0.87 | 45.2 |
| Recycling Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 78 | 9.0 | 0.417 | 46.9 | LOS D | 30 | 0.95 | 0.76 | 24.0 |
| WT | T | 1 | 0.0 | 0.419 | 40.6 | LOS C | 30 | 0.95 | 0.73 | 26.4 |
| WR | R | 48 | 6.2 | 0.373 | 49.8 | LOS D | 19 | 0.97 | 0.73 | 23.3 |
| Approach |  | 127 | 7.9 | 0.417 | 47.9 | LOS D | 30 | 0.96 | 0.75 | 23.7 |
| All Vehicles |  | 4234 | 2.4 | 0.857 | 23.7 | LOS B | 214 | 0.89 | 0.80 | 44.3 |

## Pedestrian Movements

| Mov ID | Dem FIow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SP | 11 | 43.2 | LOS E | 0 | 0.94 | 0.94 |
| EP | 21 | 35.7 | LOS D | 0 | 0.93 | 0.93 |
| WP | 21 | 35.7 | LOS D | 0 | 0.93 | 0.93 |

## SIDRA -- <br> INTERSECTION

## Movement Summary

## Wilton Rd / Douglas Park Dr

Existing - AM

Two-way stop
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 49 | 0.0 | 0.039 | 0.3 | LOS A | 2 | 0.17 | 0.00 | 75.8 |
| ER | R | 19 | 0.0 | 0.039 | 11.0 | LOS A | 2 | 0.17 | 0.68 | 58.2 |
| Approach |  | 68 | 0.0 | 0.039 | 3.3 | Los A | 2 | 0.17 | 0.19 | 70.0 |
| Douglas Park Dr (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 27 | 3.7 | 0.168 | 14.7 | LOS B | 7 | 0.33 | 0.84 | 55.2 |
| NR | R | 100 | 5.0 | 0.168 | 14.6 | LOS B | 7 | 0.33 | 0.90 | 55.4 |
| Approach |  | 127 | 4.7 | 0.168 | 14.6 | LOS B | 7 | 0.33 | 0.88 | 55.4 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 63 | 7.9 | 0.051 | 11.5 | LOS A | 2 | 0.08 | 0.65 | 58.3 |
| WT | T | 72 | 2.8 | 0.037 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 134 | 5.2 | 0.051 | 5.4 | LOS A | 2 | 0.04 | 0.31 | 68.2 |
| All Vehicles |  | 329 | 4.0 | 0.168 | 8.5 | Applicable | 7 | 0.18 | 0.51 | 63.0 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: EX-AM
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## SIDRA -- <br> INTERSECTION

## Movement Summary

Wilton Rd / Douglas Park Dr
Existing - PM

Two-way stop
Vehicle Movements

| Mov ID | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 105 | 0.0 | 0.071 | 0.3 | LOS A | 3 | 0.18 | 0.00 | 75.8 |
| ER | R | 23 | 0.0 | 0.071 | 11.0 | LOS A | 3 | 0.18 | 0.68 | 58.2 |
| Approach |  | 128 | 0.0 | 0.071 | 2.2 | Los A | 3 | 0.18 | 0.12 | 71.9 |
| Douglas Park Dr (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 19 | 5.3 | 0.170 | 15.6 | LOS B | 6 | 0.39 | 0.82 | 54.4 |
| NR | R | 96 | 4.2 | 0.169 | 15.3 | LOS B | 6 | 0.39 | 0.91 | 54.6 |
| Approach |  | 115 | 4.3 | 0.169 | 15.4 | LOS B | 6 | 0.39 | 0.90 | 54.6 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 84 | 4.8 | 0.066 | 11.4 | LOS A | 2 | 0.09 | 0.65 | 58.3 |
| WT | T | 68 | 0.0 | 0.035 | 0.0 | Los A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 152 | 2.6 | 0.066 | 6.3 | LOS A | 2 | 0.05 | 0.36 | 66.4 |
| All Vehicles |  | 395 | 2.3 | 0.170 | 7.6 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 6 | 0.19 | 0.44 | 64.0 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


SIDRA SOLUTIONS

Site: EX-PM
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## SIDRA -- <br> INTERSECTION

## Movement Summary

## Wilton Rd / Macarthur Rd

Existing - AM

Two-way stop
Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow/ } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn ( $\mathrm{v} / \mathrm{c}$ ) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Macarthur Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 4 | 25.0 | 0.006 | 15.5 | LOS B | 0 | 0.22 | 0.86 | 42.7 |
| SR | R | 41 | 12.2 | 0.068 | 16.3 | LOS B | 3 | 0.40 | 0.88 | 54.3 |
| Approach |  | 45 | 13.3 | 0.068 | 16.3 | LOS B | 3 | 0.38 | 0.88 | 53.6 |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 63 | 1.6 | 0.034 | 11.0 | LOS A | 0 | 0.00 | 0.73 | 58.9 |
| ET | T | 87 | 3.4 | 0.046 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 150 | 2.7 | 0.046 | 4.6 | LOS A |  | 0.00 | 0.31 | 69.7 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WT | T | 92 | 3.3 | 0.035 | 0.5 | LOS A | 2 | 0.21 | 0.00 | 74.9 |
| WR | R | 4 | 25.0 | 0.035 | 13.2 | LOS A | 2 | 0.33 | 0.66 | 57.1 |
| Approach |  | 95 | 4.2 | 0.035 | 1.1 | LOS A | 2 | 0.22 | 0.03 | 74.0 |
| All Vehicles |  | 290 | 4.8 | 0.068 | 5.3 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 3 | 0.13 | 0.30 | 68.0 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: EX - AM
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## SIDRA -- <br> INTERSECTION

## Movement Summary

## Wilton Rd / Macarthur Rd

Existing - PM

Two-way stop

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Macarthur Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 6 | 0.0 | 0.007 | 13.5 | LOS A | 0 | 0.26 | 0.86 | 42.8 |
| SR | R | 39 | 0.0 | 0.067 | 15.9 | LOS B | 2 | 0.45 | 0.90 | 53.6 |
| Approach |  | 45 | 0.0 | 0.067 | 15.6 | LOS B | 2 | 0.43 | 0.89 | 52.7 |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 55 | 1.8 | 0.030 | 11.0 | LOS A | 0 | 0.00 | 0.73 | 58.9 |
| ET | T | 145 | 1.4 | 0.075 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 200 | 1.5 | 0.075 | 3.0 | $\operatorname{LOS} A$ |  | 0.00 | 0.20 | 72.9 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WT | $T$ | 117 | 4.3 | 0.047 | 0.8 | LOS A | 3 | 0.25 | 0.00 | 74.1 |
| WR | R | 7 | 28.6 | 0.048 | 13.9 | LOS A | 3 | 0.39 | 0.68 | 56.7 |
| Approach |  | 124 | 5.6 | 0.047 | 1.5 | $\operatorname{LOS} A$ | 3 | 0.26 | 0.04 | 72.9 |
| All Vehicles |  | 369 | 2.7 | 0.075 | 4.1 | Not <br> Applicable | 3 | 0.14 | 0.23 | 69.9 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: EX - PM
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## attachment d2

future (2019) intersection performance

## SIDRA <br> INTERSECTION

## Movement Summary

Appin Rd - George St (Appin East Colliery Exit)
Future (2019) - AM (Without Project)
Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{5 0}$ seconds

## Vehicle Movements

| Mov ID Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| ET T | 674 | 5.5 | 0.613 | 18.6 | LOS B | 75 | 0.84 | 0.77 | 46.3 |
| Approach | 674 | 5.5 | 0.613 | 18.6 | LOS B | 75 | 0.84 | 0.77 | 46.3 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 398 | 3.8 | 0.290 | 12.6 | LOS A | 35 | 0.75 | 0.62 | 43.3 |
| Approach | 398 | 3.8 | 0.290 | 12.6 | LOS A | 35 | 0.75 | 0.62 | 43.3 |
| George St (south) |  |  |  |  |  |  |  |  |  |
| SR R | 48 | 69.4 | 0.099 | 18.8 | LOS B | 13 | 0.65 | 0.73 | 34.1 |
| Approach | 49 | 69.4 | 0.099 | 18.8 | LOS B | 13 | 0.65 | 0.73 | 34.1 |
| All Vehicles | 1121 | 7.7 | 0.613 | 16.5 | LOS B | 75 | 0.80 | 0.72 | 44.3 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


Site: FU (2019) - AM (Without Development)
T:\2007\07267\Survey Results \& Modelling\SIDRA Files\Appin Rd \& George St (Appin East Colliery-Exit).aap Processed Apr 21, 2009 09:39:00AM

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## SIDRA <br> Movement Summary

Appin Rd - George St (Appin East Colliery Exit)
Future (2019) - PM (Without Project)

Signalised - Fixed time Cycle Time $=\mathbf{4 0}$ seconds

## Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| ET T | 446 | 2.5 | 0.717 | 22.3 | LOS B | 52 | 0.96 | 0.84 | 42.7 |
| Approach | 447 | 2.5 | 0.717 | 22.3 | LOS B | 52 | 0.96 | 0.84 | 42.7 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 582 | 0.9 | 0.750 | 19.4 | LOS B | 55 | 0.99 | 0.95 | 38.9 |
| Approach | 582 | 0.9 | 0.750 | 19.4 | Los B | 55 | 0.99 | 0.95 | 38.9 |
| George St (south) |  |  |  |  |  |  |  |  |  |
| SR R | 27 | 60.7 | 0.043 | 13.7 | LOS A | 5 | 0.53 | 0.69 | 36.5 |
| Approach | 28 | 60.7 | 0.043 | 13.7 | Los A | 5 | 0.53 | 0.69 | 36.5 |
| All Vehicles | 1057 | 3.1 | 0.750 | 20.5 | Los B | 55 | 0.97 | 0.90 | 40.2 |

Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


SIDRA SOLUTIONS

Site: FU (2019) - PM (Without Development)
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## SIDRA <br> INTERSECTION

## Movement Summary

## Appin Rd / Princes Hwy Off-Ramp

FU (2019) - AM (no development)

## Two-way stop

## Vehicle Movements

| Mov 1D | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | $95 \%$ <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 639 | 7.0 | 0.343 | 0.0 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 639 | 7.0 | 0.343 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| Merge Lane |  |  |  |  |  |  |  |  |  |  |
| Merge | T | 55 | 9.3 | 0.079 | 13.7 | LOS A | 2 | 0.52 | 0.81 | 55.8 |
| Approach |  | 54 | 9.3 | 0.079 | 13.7 | LOS A | 2 | 0.52 | 0.81 | 55.8 |
| Princes Hwy Off-Ramp (north) |  |  |  |  |  |  |  |  |  |  |
| NR | R | 55 | 9.3 | 0.118 | 19.0 | LOS B | 4 | 0.54 | 0.96 | 51.3 |
| Approach |  | 54 | 9.3 | 0.118 | 19.0 | LOS B | 4 | 0.54 | 0.96 | 51.3 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 7 | 28.6 | 0.019 | 12.6 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.73 | 58.9 |
| WT | T | 366 | 10.7 | 0.187 | 0.0 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 373 | 11.0 | 0.187 | 0.2 | Los A |  | 0.00 | 0.01 | 79.5 |
| All Vehicles |  | 1120 | 8.6 | 0.343 | 1.7 | Not Applicable | 4 | 0.05 | 0.09 | 76.3 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


SIDRA SOLUTIONS

Site: FU (2019) - AM (no development)
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## SIDRA <br> INTERSECTION

## Movement Summary

## Appin Rd / Princes Hwy Off-Ramp

FU (2019) - PM (without project)

Two-way stop

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | $95 \%$ <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver <br> Speed <br> (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 307 | 5.5 | 0.164 | 0.0 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 308 | 5.5 | 0.164 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| Merge Lane |  |  |  |  |  |  |  |  |  |  |
| Merge | T | 85 | 0.0 | 0.077 | 10.9 | $\operatorname{LOS~A}$ | 2 | 0.34 | 0.67 | 57.9 |
| Approach |  | 85 | 0.0 | 0.077 | 10.9 | $\operatorname{LoS} A$ | 2 | 0.34 | 0.67 | 57.9 |
| Princes Hwy Off-Ramp (north) |  |  |  |  |  |  |  |  |  |  |
| NR | R | 85 | 0.0 | 0.274 | 24.7 | LOS B | 10 | 0.72 | 1.02 | 45.5 |
| Approach |  | 85 | 0.0 | 0.275 | 24.7 | LOS B | 10 | 0.72 | 1.02 | 45.5 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 4 | 25.0 | 0.031 | 12.4 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.73 | 58.9 |
| WT | T | 634 | 3.9 | 0.305 | 0.0 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 637 | 4.1 | 0.305 | 0.1 | LOS A |  | 0.00 | 0.00 | 79.8 |
| All Vehicles |  | 1115 | 3.9 | 0.305 | 2.8 | Not Applicable | 10 | 0.08 | 0.13 | 73.6 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


SIDRA SOLUTIONS

Site: FU (2019) - PM (no development)
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## Movement Summary

## Appin Rd - Sherrif Rd (Appin East Colliery Entry)

Future (2019) - AM

Two-way stop

## Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver <br> Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop, Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 38 | 94.7 | 0.196 | 14.5 | LOS B | 0 | 0.00 | 0.72 | 57.6 |
| ET T | 674 | 5.5 | 0.196 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 712 | 10.3 | 0.196 | 0.8 | LOS A |  | 0.00 | 0.04 | 78.7 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 505 | 17.6 | 0.144 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 505 | 17.6 | 0.144 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| All Vehicles | 1217 | 13.3 | 0.196 | 0.5 | Applicable | 0 | 0.00 | 0.02 | 79.2 |

Symbols which may appear in this table:
Following Degree of Saturation
\# x $=1.00$ for Short Lane with resulting Excess Flow
$* x=1.00$ due to minimum capacity
Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - AM
T: \2007\07267\Survey Results \& Modelling $\backslash$ SIDRA Files $\backslash$ Appin Rd \& Sherrif Rd (Appin East Colliery).aap Processed Apr 21, 2009 03:18:10PM

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## SIDRA <br> INTERSECTION

## Movement Summary

Appin Rd - Sherrif Rd (Appin East Colliery Entry)
Future (2019) - PM
Two-way stop
Vehicle Movements

| Mov ID Turn | $\begin{gathered} \text { Dem } \\ \text { Flow/ } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 9 | 100.0 | 0.120 | 14.8 | LOS B | 0 | 0.00 | 0.72 | 52.4 |
| ET T | 446 | 2.5 | 0.121 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 456 | 4.4 | 0.121 | 0.3 | $\operatorname{LOS} A$ |  | 0.00 | 0.01 | 79.3 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 643 | 6.7 | 0.172 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 643 | 6.7 | 0.172 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| Sherrif Rd (south) |  |  |  |  |  |  |  |  |  |
| 32 R | 1 | 0.0 | 0.001 | 8.3 | LOS A | 0 | 0.00 | 0.68 | 48.8 |
| Approach | 1 | 0.0 | 0.001 | 8.3 | $\operatorname{Los} A$ |  | 0.00 | 0.68 | 48.8 |
| All Vehicles | 1100 | 5.7 | 0.172 | 0.1 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 0 | 0.00 | 0.01 | 79.7 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


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Site: FU (2019) - PM
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## SIDRA <br> INTERSECTION

## Movement Summary

## Appin Rd - West Cliff Colliery Access

## Future (2019) - AM

## Two-way stop

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 689 | 1.3 | 0.356 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 100.0 |
| ER | R | 45 | 67.4 | 0.108 | 22.3 | $\operatorname{LOS} B$ | 6 | 0.57 | 0.85 | 53.7 |
| Approach |  | 735 | 5.4 | 0.356 | 1.4 | $\operatorname{LOS~A}$ | 6 | 0.04 | 0.05 | 96.0 |
| Median Storage |  |  |  |  |  |  |  |  |  |  |
| Store | T | 38 | 86.8 | 0.247 | 28.3 | LOS B | 14 | 0.85 | 0.97 | 23.4 |
| Approach |  | 38 | 86.8 | 0.247 | 28.3 | LOS B | 14 | 0.85 | 0.97 | 23.4 |
| West Cliff Access (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 51 | 66.7 | 0.367 | 33.1 | LOS C | 24 | 0.71 | 1.09 | 41.2 |
| NR | R | 38 | 86.8 | 0.369 | 34.7 | LOS C | 24 | 0.71 | 1.08 | 34.2 |
| Approach |  | 89 | 75.3 | 0.368 | 33.8 | Los C | 24 | 0.71 | 1.08 | 38.6 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 58 | 15.8 | 0.034 | 13.5 | LOS A | 0 | 0.00 | 0.76 | 63.3 |
| WT | T | 395 | 3.0 | 0.103 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 100.0 |
| Approach |  | 452 | 4.6 | 0.103 | 1.7 | LOS A |  | 0.00 | 0.10 | 94.6 |
| All Vehicles |  | 1314 | 12.3 | 0.369 | 4.5 | Not Applicable | 24 | 0.09 | 0.16 | 85.0 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $\mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA ${ }^{-\quad-}$ INTERSECTION

## Movement Summary

## Appin Rd - West Cliff Colliery Access

Future (2019) - PM

## Two-way stop

## Vehicle Movements



Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - PM
$\mathrm{T}: \backslash 2007 \backslash 07267 \backslash$ Survey Results \& Modelling ${ }^{\text {S }}$ SIDRA Files $\backslash$ Appin Rd \& West Cliff Access.aap
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## SIDRA INTERSECTION

## Movement Summary

## Appin Rd / Wilton Rd

Future (2019) - AM

Two-way stop

## Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (south) |  |  |  |  |  |  |  |  |  |  |
| ST | T | 75 | 2.7 | 0.242 | 21.0 | LOS B | 9 | 0.75 | 1.02 | 16.8 |
| SR | R | 48 | 4.2 | 0.180 | 21.9 | LOS B | 6 | 0.74 | 1.00 | 33.5 |
| Approach |  | 123 | 3.3 | 0.242 | 21.3 | Los B | 9 | 0.75 | 1.01 | 26.4 |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 52 | 1.9 | 0.371 | 6.5 | LOS A | 0 | 0.00 | 0.61 | 43.3 |
| ER | R | 629 | 1.9 | 0.372 | 6.6 | LOS A | 0 | 0.00 | 0.63 | 43.2 |
| Approach |  | 682 | 1.9 | 0.372 | 6.6 | LOS A |  | 0.00 | 0.63 | 43.2 |
| Appin Rd (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 360 | 3.1 | 0.325 | 6.0 | LOS A | 14 | 0.17 | 0.51 | 35.1 |
| NT | T | 53 | 13.5 | 0.218 | 25.3 | LOS B | 8 | 0.79 | 1.02 | 32.1 |
| Approach |  | 412 | 4.4 | 0.325 | 8.4 | LOS A | 14 | 0.25 | 0.57 | 34.2 |
| All Vehicles |  | 1217 | 2.9 | 0.372 | 8.7 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 14 | 0.16 | 0.65 | 39.4 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## J

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Site: FU (2019) - AM
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SIDRA
INTERSECTION
Movement Summary

## Appin Rd / Wilton Rd

Future (2019) - PM

Two-way stop

## Vehicle Movements

| Mov ID Turn | Dem Fiow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (south) |  |  |  |  |  |  |  |  |  |
| ST T | 99 | 1.0 | 0.194 | 14.4 | LOS A | 7 | 0.59 | 0.99 | 21.3 |
| SR R | 66 | 1.5 | 0.173 | 16.9 | LOS B | 6 | 0.62 | 1.00 | 36.2 |
| Approach | 165 | 1.2 | 0.194 | 15.4 | LOS B | 7 | 0.60 | 0.99 | 30.6 |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 80 | 2.5 | 0.259 | 6.5 | LOS A | 0 | 0.00 | 0.61 | 43.3 |
| ER R | 395 | 2.0 | 0.259 | 6.6 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.63 | 43.2 |
| Approach | 474 | 2.1 | 0.259 | 6.6 | LOS A |  | 0.00 | 0.63 | 43.2 |
| Appin Rd (north) |  |  |  |  |  |  |  |  |  |
| NL L | 544 | 0.7 | 0.489 | 6.2 | LOS A | 26 | 0.26 | 0.52 | 34.4 |
| NT T | 93 | 1.1 | 0.198 | 15.2 | LOS B | 7 | 0.61 | 1.00 | 37.4 |
| Approach | 637 | 0.8 | 0.489 | 7.5 | Los A | 26 | 0.31 | 0.59 | 35.3 |
| All Vehicles | 1276 | 1.3 | 0.489 | 8.2 | Not <br> Applicable | 26 | 0.23 | 0.65 | 38.6 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - PM
$\mathrm{T}: \backslash 2007 \backslash 07267 \backslash$ Survey Results \& Modelling $\backslash$ SIDRA Files $\backslash A p p i n$ Rd \& Wilton Rd.aap
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## SIDRA <br> INTERSECTION

## Movement Summary

Masters Rd / Springhill Rd
Future (2019) - AM

Signalised - Fixed time Cycle Time $=\mathbf{1 1 0}$ seconds

## Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn ( $\mathrm{v} / \mathrm{c}$ ) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 542 | 0.0 | 0.176 | 13.1 | LOS A | 28 | 0.22 | 0.73 | 56.2 |
| 2 | T | 2613 | 0.0 | 0.847 | 27.4 | LOS B | 292 | 0.93 | 0.89 | 42.6 |
| Approach |  | 3155 | 0.0 | 0.847 | 24.9 | LOS B | 292 | 0.80 | 0.86 | 44.4 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 677 | 0.0 | 0.329 | 15.8 | LOS B | 83 | 0.61 | 0.53 | 53.1 |
| 9 | R | 296 | 0.0 | 0.835 | 74.4 | LOS F | 53 | 1.00 | 0.90 | 23.4 |
| Approach |  | 973 | 0.0 | 0.835 | 33.6 | LOS C | 83 | 0.73 | 0.64 | 38.4 |
| Masters Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 517 | 0.0 | 0.511 | 39.4 | LOS C | 110 | 0.80 | 0.82 | 35.1 |
| 12 | R | 730 | 0.0 | 0.534 | 50.2 | LOS D | 91 | 0.92 | 0.83 | 30.5 |
| Approach |  | 1247 | 0.0 | 0.534 | 45.7 | LOS D | 110 | 0.87 | 0.82 | 32.2 |
| All Vehicles |  | 5375 | 0.0 | 0.847 | 31.3 | Los C | 292 | 0.81 | 0.81 | 39.8 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathrm{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P5 | 50 | 49.2 | LOS E | 0 | 0.95 | 0.95 |
| All Peds | 50 | 49.2 | LOS D | 0 | 0.95 | $\mathbf{0 . 9 5}$ |

Symbols which may appear in this table:
Following Degree of Saturation
\# $\mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue

## SIDRA -- <br> INTERSECTION

## Movement Summary

## Masters Rd / Springhill Rd

Future (2019) - PM
Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{1 1 0}$ seconds

## Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 894 | 0.0 | 0.363 | 19.7 | LOS B | 84 | 0.48 | 0.78 | 48.8 |
| 2 | T | 1375 | 0.0 | 0.646 | 31.3 | Los C | 147 | 0.89 | 0.78 | 39.9 |
| Approach |  | 2269 | 0.0 | 0.646 | 26.7 | LOS B | 147 | 0.73 | 0.78 | 43.0 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 1224 | 0.0 | 0.863 | 42.6 | LOS D | 238 | 1.00 | 0.98 | 33.8 |
| 9 | R | 1082 | 0.0 | 0.855 | 63.4 | LOS E | 151 | 1.00 | 0.98 | 26.1 |
| Approach |  | 2306 | 0.0 | 0.863 | 52.4 | LOS D | 238 | 1.00 | 0.98 | 29.7 |
| Masters Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 188 | 0.0 | 0.133 | 24.8 | LOS B | 34 | 0.53 | 0.75 | 44.3 |
| 12 | R | 680 | 0.0 | 0.497 | 49.8 | LOS D | 85 | 0.91 | 0.82 | 30.6 |
| Approach |  | 868 | 0.0 | 0.497 | 44.4 | LOS D | 85 | 0.83 | 0.81 | 32.8 |
| All Vehicles |  | 5443 | 0.0 | 0.863 | 40.4 | Los C | 238 | 0.86 | 0.87 | 34.7 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathrm{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P5 | 50 | 49.2 | LOS E | 0 | 0.95 | 0.95 |
| All Peds | 50 | 49.2 | LOS D | 0 | 0.95 | $\mathbf{0 . 9 5}$ |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue

## Movement Summary

Mt Ousley Rd / Southern Fwy
Future (2019) - AM

Two-way stop

## Vehicle Movements



Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


SIDRA SOLUTIONS

Site: Future (2019) - AM
T:\2007\07267\Survey Results \& Modelling\SIDRA Files\Mt Ousley Rd \& Southern Fwy.aap Processed Apr 21, 2009 05:35:52PM

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## SIDRA -- <br> INTERSECTION

## Movement Summary

## Mt Ousley Rd / Southern Fwy

Future (2019) - PM

Two-way stop

## Vehicle Movements

| Mov ID Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Southern Fwy (south) |  |  |  |  |  |  |  |  |  |
| ST T | 2452 | 3.6 | 0.643 | 0.0 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 2451 | 3.6 | 0.643 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| Mt Ousley Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 224 | 0.0 | 0.121 | 7.6 | LOS A\# | 4\# | 0.00 | 0.60 | 49.8 |
| ER R | 147 | 0.7 | 2.450 | 46281.5 | LOS F | 962 | 1.00 | 9.75 | 0.1 |
| Approach | 371 | 0.3 | 2.450 | 18342.5 | LOS F | 962 | 0.40 | 4.23 | 0.1 |
| Mt Ousley Rd (north) |  |  |  |  |  |  |  |  |  |
| NL L | 472 | 0.4 | 0.254 | 11.1 | LOS A\# | 6\# | 0.00 | 0.69 | 58.8 |
| NT T | 3451 | 3.0 | 0.902 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 3921 | 2.7 | 0.902 | 1.3 | LOS A |  | 0.00 | 0.08 | 76.7 |
| All Vehicles | 6743 | 2.9 | 2.450 | 1010.0 | Not Applicable | 962 | 0.02 | 0.28 | 2.4 |

Symbols which may appear in this table:

Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - PM
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## SIDRA <br> INTERSECTION

## Movement Summary

## Picton Rd / Almond St (Wilton Rd)

Future (2019) - AM (Without Project)
Two-way stop

## Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Picton Rd (east) |  |  |  |  |  |  |  |  |  |
| ET T | 704 | 13.5 | 0.393 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 100.0 |
| ER R | 14 | 30.8 | 0.044 | 25.3 | LOS B | 2 | 0.75 | 0.94 | 47.9 |
| Approach | 717 | 13.8 | 0.393 | 0.5 | LOS A | 2 | 0.01 | 0.02 | 98.5 |
| Median Storage |  |  |  |  |  |  |  |  |  |
| $5 \quad T$ | 131 | 3.1 | 0.354 | 11.5 | LOS A | 1.2 | 0.74 | 0.96 | 37.0 |
| Approach | 130 | 3.1 | 0.355 | 11.5 | LOS A | 12 | 0.74 | 0.96 | 37.0 |
| Almond St (north) |  |  |  |  |  |  |  |  |  |
| NL L | 18 | 0.0 | 0.621 | 38.7 | LOS C | 30 | 0.90 | 1.19 | 35.7 |
| NR R | 131 | 3.1 | 0.628 | 40.7 | LOS C | 30 | 0.90 | 1.18 | 28.9 |
| Approach | 148 | 2.7 | 0.629 | 40.4 | Los C | 30 | 0.90 | 1.18 | 29.9 |
| Picton Rd (west) |  |  |  |  |  |  |  |  |  |
| WL L | 93 | 8.7 | 0.053 | 13.1 | LOS A | 0 | 0.00 | 0.76 | 63.3 |
| WT T | 835 | 12.1 | 0.462 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 100.0 |
| Approach | 927 | 11.8 | 0.462 | 1.3 | LOS A |  | 0.00 | 0.07 | 95.7 |
| All Vehicles | 1922 | 11.2 | 0.628 | 4.7 | Not Applicable | 30 | 0.12 | 0.20 | 82.7 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - AM (Without Development)
$\mathrm{T}: \backslash 2007 \backslash 07267 \backslash$ Survey Results \& Modelling $\backslash$ SIDRA Files $\backslash$ Picton Rd \& Almond St.aap Processed Apr 21, 2009 12:03:35PM

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## SIDRA -- <br> INTERSECTION

## Movement Summary

Picton Rd / Almond St (Wilton Rd)
Future (2019) - PM (Without Project)
Two-way stop
Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed ( $\mathrm{km} / \mathrm{h}$ ) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Picton Rd (east) |  |  |  |  |  |  |  |  |  |
| ET T | 789 | 5.7 | 0.420 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 100.0 |
| ER $\quad \mathrm{R}$ | 13 | 7.7 | 0.033 | 21.0 | LOS B | 1 | 0.73 | 0.91 | 51.3 |
| Approach | 802 | 5.7 | 0.420 | 0.3 | LOS A | 1 | 0.01 | 0.01 | 98.8 |
| Median Storage |  |  |  |  |  |  |  |  |  |
| 5 T | 148 | 2.0 | 0.430 | 13.1 | LOS A | 15 | 0.78 | 1.04 | 35.0 |
| Approach | 148 | 2.0 | 0.431 | 13.1 | $\operatorname{LOS} A$ | 15 | 0.78 | 1.04 | 35.0 |
| Almond St (north) |  |  |  |  |  |  |  |  |  |
| NL L | 15 | 0.0 | 0.833 | 61.3 | LOS E | 50 | 0.96 | 1.39 | 27.8 |
| NR R | 148 | 2.0 | 0.827 | 63.3 | LOS E | 50 | 0.96 | 1.38 | 21.4 |
| Approach | 163 | 1.8 | 0.827 | 63.1 | LOS E | 50 | 0.96 | 1.38 | 22.0 |
| Picton Rd (west) |  |  |  |  |  |  |  |  |  |
| WL L | 175 | 4.6 | 0.097 | 12.8 | LOS A | 0 | 0.00 | 0.75 | 63.3 |
| WT T | 945 | 3.5 | 0.496 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 100.0 |
| Approach | 1120 | 3.7 | 0.496 | 2.0 | $\operatorname{LOS~A}$ |  | 0.00 | 0.12 | 93.4 |
| All Vehicles | 2233 | 4.2 | 0.833 | 6.6 | Not <br> Applicable | 50 | 0.13 | 0.23 | 78.0 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $\mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - PM (Without Development)
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## SIDRA INTERSECTION <br> Movement Summary

Springhill Rd / Port Kembla Rd
Future (2019) - AM
Signalised - Fixed time Cycle Time $=\mathbf{5 0}$ seconds
Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Port Kembla Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 31 | 0.0 | 0.046 | 19.0 | LOS B | 5 | 0.68 | 0.71 | 37.9 |
| 3 | R | 8 | 0.0 | 0.036 | 29.2 | Los C | 2 | 0.91 | 0.66 | 30.4 |
| Approach |  | 39 | 0.0 | 0.046 | 21.1 | LOS B | 5 | 0.72 | 0.70 | 36.3 |
| Springhill Rd (east) |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 12 | 0.0 | 0.214 | 18.2 | LOS B | 27 | 0.69 | 0.76 | 38.8 |
| 5 | T | 489 | 0.0 | 0.214 | 10.8 | LOS A | 27 | 0.69 | 0.56 | 50.2 |
| Approach |  | 501 | 0.0 | 0.214 | 11.0 | Los A | 27 | 0.69 | 0.57 | 49.9 |
| Sprinhill Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 11 | T | 1093 | 0.0 | 0.280 | 4.1 | LOS A\# | 7\# | 0.00 | 0.32 | 70.8 |
| 12 | R | 38 | 0.0 | 0.171 | 32.9 | LOS C | 10 | 0.93 | 0.73 | 35.1 |
| Approach |  | 1131 | 0.0 | 0.280 | 5.0 | LOS A | 10 | 0.03 | 0.33 | 68.9 |
| All Vehicles |  | 1671 | 0.0 | 0.280 | 7.2 | Los A | 27 | 0.25 | 0.41 | 60.8 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> $(\mathbf{m})$ | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 50 | 13.0 | LOS B | 0 | 0.72 | 0.72 |
| All Peds | $\mathbf{5 0}$ | $\mathbf{1 3 . 0}$ | LOS A | $\mathbf{0}$ | $\mathbf{0 . 7 2}$ | $\mathbf{0 . 7 2}$ |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue

## SIDRA <br> Movement Summary

## Springhill Rd / Port Kembla Rd

Future (2019) - PM

Signalised - Fixed time $\quad$ Cycle Time $=\mathbf{5 0}$ seconds
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Port Kembla Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 67 | 0.0 | 0.100 | 19.3 | LOS B | 12 | 0.69 | 0.74 | 37.8 |
| 3 | R | 28 | 0.0 | 0.126 | 29.7 | Los C | 7 | 0.92 | 0.71 | 30.1 |
| Approach |  | 95 | 0.0 | 0.126 | 22.4 | LOS B | 12 | 0.76 | 0.73 | 35.3 |
| Springhill Rd (east) |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 5 | 0.0 | 0.463 | 19.7 | LOS B | 59 | 0.79 | 0.81 | 37.8 |
| 5 | T | 1094 | 0.0 | 0.470 | 12.2 | LOS A | 59 | 0.79 | 0.67 | 48.9 |
| Approach |  | 1099 | 0.0 | 0.470 | 12.3 | LOS A | 59 | 0.79 | 0.67 | 48.8 |
| Sprinhill Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 11 | T | 598 | 0.0 | 0.153 | 4.1 | LOS A\# | 4\# | 0.00 | 0.32 | 70.8 |
| 12 | R | 36 | 0.0 | 0.162 | 32.8 | LOS C | 9 | 0.93 | 0.72 | 35.2 |
| Approach |  | 634 | 0.0 | 0.162 | 5.7 | LOS A | 9 | 0.05 | 0.34 | 67.6 |
| All Vehicles |  | 1828 | 0.0 | 0.470 | 10.5 | LOS A | 59 | 0.53 | 0.56 | 52.9 |

Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 50 | 13.0 | LOS B | 0 | 0.72 | 0.72 |
| All Peds | 50 | $\mathbf{1 3 . 0}$ | LOS A | 0 | $\mathbf{0 . 7 2}$ | $\mathbf{0 . 7 2}$ |

Symbols which may appear in this table:
Following Degree of Saturation
\#x $=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue

## SIDRA -- <br> INTERSECTION

## Movement Summary

## Springhill Rd / Recycling Rd

## Future (2019) - AM (without project)

Signalised - Actuated isolated
Cycle Time $=94$ seconds

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 56 | 49.1 | 0.942 | 50.3 | LOS D | 330 | 1.00 | 1.03 | 27.6 |
| ST | T | 2481 | 4.6 | 0.940 | 37.5 | LOS C | 329 | 1.00 | 1.03 | 36.3 |
| SR | R | 64 | 16.9 | 0.369 | 56.8 | LOS E | 29 | 0.95 | 0.76 | 24.9 |
| Approach |  | 2601 | 5.9 | 0.940 | 38.2 | Los C | 330 | 1.00 | 1.02 | 35.8 |
| Bluescope Steel (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 28 | 7.1 | 0.149 | 50.1 | LOS D | 11 | 0.92 | 0.71 | 23.2 |
| ET | T | 3 | 66.7 | 0.021 | 42.7 | LOS D | 2 | 0.90 | 0.58 | 25.8 |
| ER | R | 9 | 22.2 | 0.106 | 56.3 | LOS D | 5 | 0.96 | 0.66 | 22.0 |
| Approach |  | 40 | 15.0 | 0.149 | 51.0 | LOS D | 11 | 0.93 | 0.69 | 23.1 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 37 | 5.4 | 0.027 | 13.9 | LOS A | 2 | 0.17 | 0.96 | 53.8 |
| NT | T | 1351 | 8.4 | 0.509 | 18.8 | LOS B | 122 | 0.72 | 0.64 | 49.9 |
| NR | R | 58 | 44.8 | 0.387 | 58.5 | LOS E | 32 | 0.95 | 0.76 | 24.7 |
| Approach |  | 1446 | 9.8 | 0.509 | 20.3 | LOS B | 122 | 0.72 | 0.65 | 48.3 |
| Recycling Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 45 | 69.6 | 0.387 | 54.5 | LOS D | 33 | 0.95 | 0.75 | 22.7 |
| WT | T | 6 | 50.0 | 0.386 | 46.5 | LOS D | 33 | 0.95 | 0.72 | 24.9 |
| WR | R | 24 | 70.8 | 0.366 | 60.0 | LOS E | 17 | 0.98 | 0.71 | 21.5 |
| Approach |  | 76 | 68.4 | 0.387 | 55.6 | LOS D | 33 | 0.96 | 0.74 | 22.4 |
| All Vehicles |  | 4163 | 8.5 | 0.942 | 32.4 | LOS C | 330 | 0.90 | 0.89 | 38.7 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |
| SP | 11 | 48.2 | LOS E | 0 | 0.94 | 0.94 |
| EP | 21 | 41.2 | LOS E | 0 | 0.94 | 0.94 |
| WP | 21 | 41.2 | LOS E | 0 | 0.94 | 0.94 |

## SIDRA ---

Movement Summary

## Springhill Rd / Recycling Rd

Future (2019) - PM (without project)
Signalised - Actuated isolated
Cycle Time $=\mathbf{8 7}$ seconds
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 11 | 20.0 | 0.663 | 31.6 | LOS C | 158 | 0.82 | 0.86 | 36.5 |
| ST | T | 1827 | 1.5 | 0.673 | 20.0 | LOS B | 158 | 0.82 | 0.73 | 48.8 |
| SR | R | 28 | 7.1 | 0.230 | 56.1 | LOS D | 11 | 0.96 | 0.72 | 25.1 |
| Approach |  | 1865 | 1.7 | 0.673 | 20.6 | LOS B | 158 | 0.82 | 0.73 | 48.1 |
| Bluescope Steel (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 64 | 16.9 | 0.341 | 48.0 | LOS D | 27 | 0.94 | 0.75 | 23.8 |
| ET | T | 6 | 50.0 | 0.035 | 39.0 | LOS C | 3 | 0.89 | 0.59 | 26.8 |
| ER | R | 37 | 5.4 | 0.300 | 51.6 | LOS D | 15 | 0.96 | 0.72 | 22.9 |
| Approach |  | 108 | 14.8 | 0.341 | 48.7 | LOS D | 27 | 0.94 | 0.73 | 23.6 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 9 | 22.2 | 0.007 | 14.7 | LOS B | 0 | 0.15 | 0.95 | 54.0 |
| NT | T | 2484 | 1.7 | 0.911 | 29.7 | LOS C | 269 | 0.98 | 0.96 | 41.0 |
| NR | R | 14 | 46.2 | 0.135 | 57.5 | LOS E | 7 | 0.95 | 0.69 | 25.1 |
| Approach |  | 2506 | 2.0 | 0.911 | 29.7 | LOS C | 269 | 0.98 | 0.96 | 40.9 |
| Recycling Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 78 | 9.0 | 0.393 | 48.0 | LOS D | 31 | 0.94 | 0.76 | 23.7 |
| WT | T | 1 | 0.0 | 0.395 | 41.7 | LOS C | 31 | 0.94 | 0.73 | 26.1 |
| WR | R | 48 | 6.2 | 0.391 | 52.2 | LOS D | 20 | 0.97 | 0.74 | 22.8 |
| Approach |  | 127 | 7.9 | 0.393 | 49.5 | LOS D | 31 | 0.95 | 0.75 | 23.4 |
| All Vehicles |  | 4606 | 2.3 | 0.911 | 27.0 | LOS B | 269 | 0.91 | 0.86 | 41.9 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SP | 11 | 44.7 | LOS E | 0 | 0.94 | 0.94 |
| EP | 21 | 37.7 | LOSD | 0 | 0.93 | 0.93 |
| WP | 21 | 37.7 | LOS D | 0 | 0.93 | 0.93 |

## SIDRA <br> INTERSECTION

## Movement Summary

Wilton Rd / Douglas Park Dr
Future (2019) - AM

Two-way stop
Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 49 | 0.0 | 0.039 | 0.3 | LOS A | 2 | 0.17 | 0.00 | 75.8 |
| ER | R | 19 | 0.0 | 0.039 | 11.0 | LOS A | 2 | 0.17 | 0.68 | 58.2 |
| Approach |  | 68 | 0.0 | 0.039 | 3.3 | $\operatorname{LOS} A$ | 2 | 0.17 | 0.19 | 70.0 |
| Douglas Park Dr (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 27 | 3.7 | 0.168 | 14.7 | LOS B | 7 | 0.33 | 0.84 | 55.2 |
| NR | R | 100 | 5.0 | 0.168 | 14.6 | LOS B | 7 | 0.33 | 0.90 | 55.4 |
| Approach |  | 127 | 4.7 | 0.168 | 14.6 | LOS B | 7 | 0.33 | 0.88 | 55.4 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 63 | 7.9 | 0.051 | 11.5 | LOS A | 2 | 0.08 | 0.65 | 58.3 |
| WT | T | 72 | 2.8 | 0.037 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 134 | 5.2 | 0.051 | 5.4 | Los A | 2 | 0.04 | 0.31 | 68.2 |
| All Vehicles |  | 329 | 4.0 | 0.168 | 8.5 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 7 | 0.18 | 0.51 | 63.0 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement


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Site: FU (2019) - AM
$\mathrm{T}: \backslash 2007 \backslash 07267 \backslash$ Survey Results \& Modelling\SIDRA Files $\backslash$ Wilton Rd \& Douglas Park Dr.aap
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## SIDRA

## Movement Summary

## Wilton Rd / Douglas Park Dr

Future (2019) - PM

Two-way stop
Vehicle Movements

| Mov ID | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 105 | 0.0 | 0.071 | 0.3 | LOS A | 3 | 0.18 | 0.00 | 75.8 |
| ER | R | 23 | 0.0 | 0.071 | 11.0 | LOS A | 3 | 0.18 | 0.68 | 58.2 |
| Approach |  | 128 | 0.0 | 0.071 | 2.2 | Los A | 3 | 0.18 | 0.12 | 71.9 |
| Douglas Park Dr (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 19 | 5.3 | 0.170 | 15.6 | LOS B | 6 | 0.39 | 0.82 | 54.4 |
| NR | R | 96 | 4.2 | 0.169 | 15.3 | LOS B | 6 | 0.39 | 0.91 | 54.6 |
| Approach |  | 115 | 4.3 | 0.169 | 15.4 | LOS B | 6 | 0.39 | 0.90 | 54.6 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 84 | 4.8 | 0.066 | 11.4 | LOS A | 2 | 0.09 | 0.65 | 58.3 |
| WT | T | 68 | 0.0 | 0.035 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 152 | 2.6 | 0.066 | 6.3 | Los A | 2 | 0.05 | 0.36 | 66.4 |
| All Vehicles |  | 395 | 2.3 | 0.170 | 7.6 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 6 | 0.19 | 0.44 | 64.0 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - PM
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## SIDRA --

Movement Summary
Wilton Rd / Macarthur Rd
Future (2019) - AM

Two-way stop

## Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Macarthur Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 4 | 25.0 | 0.006 | 15.5 | LOS B | 0 | 0.22 | 0.86 | 42.7 |
| SR | R | 41 | 12.2 | 0.068 | 16.3 | LOS B | 3 | 0.40 | 0.88 | 54.3 |
| Approach |  | 45 | 13.3 | 0.068 | 16.3 | Los B | 3 | 0.38 | 0.88 | 53.6 |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 63 | 1.6 | 0.034 | 11.0 | LOS A | 0 | 0.00 | 0.73 | 58.9 |
| ET | T | 87 | 3.4 | 0.046 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 150 | 2.7 | 0.046 | 4.6 | LOS A |  | 0.00 | 0.31 | 69.7 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WT | T | 92 | 3.3 | 0.035 | 0.5 | LOS A | 2 | 0.21 | 0.00 | 74.9 |
| WR | R | 4 | 25.0 | 0.035 | 13.2 | LOS A | 2 | 0.33 | 0.66 | 57.1 |
| Approach |  | 95 | 4.2 | 0.035 | 1.1 | Los A | 2 | 0.22 | 0.03 | 74.0 |
| All Vehicles |  | 290 | 4.8 | 0.068 | 5.3 | Applicable | 3 | 0.13 | 0.30 | 68.0 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - AM
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## SIDRA -- <br> INTERSECTION

## Movement Summary

## Wilton Rd / Macarthur Rd

Future (2019) - PM

Two-way stop

## Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Macarthur Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 6 | 0.0 | 0.007 | 13.5 | Los A | 0 | 0.26 | 0.86 | 42.8 |
| SR | R | 39 | 0.0 | 0.067 | 15.9 | Los B | 2 | 0.45 | 0.90 | 53.6 |
| Approach |  | 45 | 0.0 | 0.067 | 15.6 | Los B | 2 | 0.43 | 0.89 | 52.7 |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 55 | 1.8 | 0.030 | 11.0 | LOS A | 0 | 0.00 | 0.73 | 58.9 |
| ET | T | 145 | 1.4 | 0.075 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 200 | 1.5 | 0.075 | 3.0 | Los A |  | 0.00 | 0.20 | 72.9 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WT | T | 117 | 4.3 | 0.047 | 0.8 | LOS A | 3 | 0.25 | 0.00 | 74.1 |
| WR | R | 7 | 28.6 | 0.048 | 13.9 | LOS A | 3 | 0.39 | 0.68 | 56.7 |
| Approach |  | 124 | 5.6 | 0.047 | 1.5 | $\operatorname{Los} A$ | 3 | 0.26 | 0.04 | 72.9 |
| All Vehicles |  | 369 | 2.7 | 0.075 | 4.1 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 3 | 0.14 | 0.23 | 69.9 |

Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## attachment d3

future (2019) scenario plus additional project traffic

## SIDRA <br> INTERSECTION

## Movement Summary

Appin Rd/Wilton Rd
Future (2019) - AM (plus project)

## Two-way stop

## Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn ( $v / c$ ) | Aver <br> Delay <br> (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (south) |  |  |  |  |  |  |  |  |  |
| ST T | 86 | 2.3 | 0.292 | 22.5 | LOS B | 11 | 0.78 | 1.04 | 15.9 |
| SR $\quad R$ | 86 | 2.3 | 0.344 | 26.0 | LOS B | 13 | 0.80 | 1.06 | 31.4 |
| Approach | 172 | 2.3 | 0.345 | 24.3 | LOS B | 13 | 0.79 | 1.05 | 26.5 |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 89 | 1.1 | 0.394 | 6.4 | LOS A | 0 | 0.00 | 0.61 | 43.3 |
| ER R | 631 | 2.1 | 0.393 | 6.6 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.63 | 43.2 |
| Approach | 720 | 1.9 | 0.393 | 6.6 | Los A |  | 0.00 | 0.63 | 43.2 |
| Appin Rd (north) |  |  |  |  |  |  |  |  |  |
| NL L | 361 | 3.3 | 0.328 | 6.2 | LOS A | 14 | 0.25 | 0.52 | 34.5 |
| NT T | 64 | 10.9 | 0.283 | 27.5 | LOS B | 11 | 0.82 | 1.04 | 31.0 |
| Approach | 425 | 4.5 | 0.328 | 9.4 | $\operatorname{Los} A$ | 14 | 0.33 | 0.60 | 33.3 |
| All Vehicles | 1317 | 2.8 | 0.394 | 9.8 | Not Applicable | 14 | 0.21 | 0.67 | 38.5 |

Symbols which may appear in this table:

## Following Degree of Saturation

\# $\mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

## J

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## SIDRA <br> INTERSECTION

## Movement Summary

## Appin Rd / Wilton Rd

Future (2019) - PM (plus project)

## Two-way stop

## Vehicle Movements

| Mov ID | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow/ } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (south) |  |  |  |  |  |  |  |  |  |  |
| ST | T | 116 | 0.9 | 0.242 | 15.3 | LOS B | 9 | 0.62 | 1.01 | 20.5 |
| SR | R | 124 | 0.8 | 0.354 | 20.3 | LOS B | 15 | 0.71 | 1.07 | 34.2 |
| Approach |  | 240 | 0.8 | 0.355 | 17.9 | Los B | 15 | 0.67 | 1.04 | 30.5 |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 137 | 1.5 | 0.291 | 6.5 | LOS A | 0 | 0.00 | 0.61 | 43.3 |
| ER | R | 397 | 2.3 | 0.291 | 6.6 | LOS A | 0 | 0.00 | 0.63 | 43.2 |
| Approach |  | 533 | 2.1 | 0.291 | 6.6 | LOS A |  | 0.00 | 0.62 | 43.2 |
| Appin Rd (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 546 | 0.9 | 0.517 | 6.9 | LOS A | 30 | 0.38 | 0.57 | 33.5 |
| NT | T | 109 | 0.9 | 0.263 | 17.3 | LOS B | 10 | 0.67 | 1.03 | 36.1 |
| Approach |  | 655 | 0.9 | 0.517 | 8.6 | LOS A | 30 | 0.43 | 0.65 | 34.4 |
| All Vehicles |  | 1428 | 1.3 | 0.517 | 9.4 | Applicable | 30 | 0.31 | 0.71 | 37.8 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA INTERSECTION

## Movement Summary

## Appin Rd - Sherrif Rd (Appin East Colliery Entry)

Future (2019) - AM (plus project)

Two-way stop

## Vehicle Movements

| Mov ID Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn ( $\mathrm{v} / \mathrm{c}$ ) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| EL L | 81 | 62.2 | 0.219 | 13.3 | LOS A | 0 | 0.00 | 0.72 | 57.6 |
| ET T | 704 | 5.3 | 0.219 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach | 786 | 11.2 | 0.219 | 1.4 | LOS A |  | 0.00 | 0.08 | 77.4 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 579 | 18.1 | 0.186 | 9.4 | LOS A | 57 | 0.42 | 0.00 | 61.4 |
| WR R | 8 | 0.0 | 0.186 | 32.5 | Los C | 57 | 0.96 | 0.99 | 35.4 |
| Approach | 587 | 17.9 | 0.186 | 9.7 | LOS A | 57 | 0.43 | 0.01 | 60.9 |
| All Vehicles | 1373 | 14.1 | 0.219 | 5.0 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 57 | 0.18 | 0.05 | 69.3 |

Symbols which may appear in this table:

## Following Degree of Saturation

\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Foilowing Queue
\# - Density for continuous movement

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## SIDRA -- <br> INTERSECTION

Movement Summary
Appin Rd - Sherrif Rd (Appin East Colliery Entry)
Future (2019) - PM (plus project)

Two-way stop
Vehicle Movements

| Mov ID | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 66 | 36.4 | 0.151 | 12.3 | LOS A | 0 | 0.00 | 0.72 | 57.6 |
| ET | T | 493 | 2.2 | 0.151 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 559 | 6.3 | 0.151 | 1.5 | LOS A |  | 0.00 | 0.09 | 77.1 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WT | T | 746 | 7.9 | 0.218 | 4.9 | LOS A | 42 | 0.42 | 0.00 | 69.1 |
| WR | R | 12 | 0.0 | 0.218 | 21.7 | LOS B | 42 | 0.91 | 0.98 | 43.9 |
| Approach |  | 758 | 7.8 | 0.218 | 5.2 | LOS A | 42 | 0.42 | 0.02 | 68.6 |
| Sherrif Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 32 | R | 1 | 0.0 | 0.001 | 8.3 | LOS A | 0 | 0.00 | 0.68 | 48.8 |
| Approach |  | 1 | 0.0 | 0.001 | 8.3 | LOS A |  | 0.00 | 0.68 | 48.8 |
| All Vehicles |  | 1318 | 7.1 | 0.218 | 3.6 | Applicable | 42 | 0.24 | 0.05 | 71.9 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA -- <br> INTERSECTION <br> Movement Summary

## Appin Rd - George St (Appin East Colliery Exit)

Future (2019) - AM (Plus Project)

Signalised - Fixed time Cycle Time $=\mathbf{5 0}$ seconds
Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| ET T | 704 | 5.3 | 0.639 | 18.9 | LOS B | 79 | 0.85 | 0.78 | 46.0 |
| Approach | 704 | 5.3 | 0.639 | 18.9 | LOS B | 79 | 0.85 | 0.78 | 46.0 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 438 | 3.7 | 0.319 | 12.8 | LOS A | 39 | 0.76 | 0.63 | 43.2 |
| Approach | 438 | 3.7 | 0.319 | 12.8 | LOS A | 39 | 0.76 | 0.63 | 43.2 |
| George St (south) |  |  |  |  |  |  |  |  |  |
| 30 L | 8 | 0.0 | 0.179 | 16.1 | LOS B | 24 | 0.68 | 0.72 | 31.1 |
| SR R | 91 | 53.3 | 0.178 | 18.6 | LOS B | 24 | 0.68 | 0.75 | 33.9 |
| Approach | 98 | 49.0 | 0.178 | 18.4 | LOS B | 24 | 0.68 | 0.75 | 33.7 |
| All Vehicles | 1240 | 8.1 | 0.639 | 16.7 | Los B | 79 | 0.81 | 0.73 | 43.6 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA <br> Movement Summary

## Appin Rd - George St (Appin East Colliery Exit)

Future (2019) - PM (Plus Project)

Signalised - Fixed time
Cycle Time $=\mathbf{4 0}$ seconds
Vehicle Movements

| Mov ID Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |
| ET T | 493 | 2.2 | 0.790 | 23.5 | LOS B | 60 | 0.97 | 0.88 | 41.7 |
| Approach | 493 | 2.2 | 0.790 | 23.5 | Los B | 60 | 0.97 | 0.88 | 41.7 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |
| WT T | 641 | 0.9 | 0.827 | 21.9 | LOS B | 64 | 1.00 | 1.05 | 37.4 |
| Approach | 641 | 0.9 | 0.827 | 21.9 | LOS B | 64 | 1.00 | 1.05 | 37.4 |
| George St (south) |  |  |  |  |  |  |  |  |  |
| 30 L | 12 | 0.0 | 0.128 | 11.2 | LOS A | 14 | 0.56 | 0.69 | 33.5 |
| SR R | 83 | 38.1 | 0.128 | 13.2 | LOS A | 14 | 0.56 | 0.72 | 36.4 |
| Approach | 96 | 33.3 | 0.128 | 13.0 | LOS A | 14 | 0.56 | 0.72 | 36.0 |
| All Vehicles | 1230 | 4.0 | 0.827 | 21.9 | LOS B | 64 | 0.96 | 0.95 | 38.7 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $\mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA INTERSECTION

## Movement Summary

## Appin Rd / Princes Hwy Off-Ramp

## FU (2019) - AM (plus development)

Two-way stop

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | $95 \%$ <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 669 | 10.0 | 0.365 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 669 | 10.0 | 0.365 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| Merge Lane |  |  |  |  |  |  |  |  |  |  |
| Merge | T | 53 | 7.7 | 0.079 | 13.9 | LOS A | 2 | 0.55 | 0.84 | 55.4 |
| Approach |  | 52 | 7.7 | 0.079 | 13.9 | LOS A | 2 | 0.55 | 0.84 | 55.4 |
| Princes Hwy Off-Ramp (north) |  |  |  |  |  |  |  |  |  |  |
| NR | $R$ | 53 | 7.7 | 0.122 | 19.7 | LOS B | 4 | 0.57 | 0.98 | 50.5 |
| Approach |  | 52 | 7.7 | 0.122 | 19.7 | LOS B | 4 | 0.57 | 0.98 | 50.5 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 7 | 28.6 | 0.021 | 12.6 | LOS A | 0 | 0.00 | 0.73 | 58.9 |
| WT | T | 397 | 15.4 | 0.208 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 404 | 15.6 | 0.208 | 0.2 | Los A |  | 0.00 | 0.01 | 79.5 |
| All Vehicles |  | 1177 | 11.7 | 0.365 | 1.6 | Not <br> Applicable | 4 | 0.05 | 0.08 | 76.4 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - AM (plus development)
$\mathrm{T}: \backslash 2007 \backslash 07267 \backslash$ Survey Results \& Modelling\SIDRA_090521\Appin Rd - Princes Hwy (offramp).aap Processed May 21, 2009 11:06:24PM

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## SIDRA <br> INTERSECTION

## Movement Summary

## Appin Rd / Princes Hwy Off-Ramp

## FU (2019) - PM (plus project)

Two-way stop

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | $\begin{aligned} & \text { Deg of } \\ & \text { Satn } \\ & (v / c) \end{aligned}$ | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 342 | 11.4 | 0.188 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 342 | 11.4 | 0.188 | 0.0 | LOS A |  | 0.00 | 0.00 | 80.0 |
| Merge Lane |  |  |  |  |  |  |  |  |  |  |
| Merge | T | 84 | 1.2 | 0.081 | 11.2 | LOS A | 2 | 0.38 | 0.69 | 57.6 |
| Approach |  | 84 | 1.2 | 0.081 | 11.2 | Los A | 2 | 0.38 | 0.69 | 57.6 |
| Princes Hwy Off-Ramp (north) |  |  |  |  |  |  |  |  |  |  |
| NR | R | 84 | 1.2 | 0.300 | 26.8 | LOS B | 11 | 0.76 | 1.03 | 43.9 |
| Approach |  | 84 | 1.2 | 0.300 | 26.8 | LOS B | 11 | 0.76 | 1.03 | 43.9 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 4 | 25.0 | 0.033 | 12.4 | LOS A | 0 | 0.00 | 0.73 | 58.9 |
| WT | T | 668 | 7.0 | 0.328 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 672 | 7.1 | 0.328 | 0.1 | $\operatorname{LoS} A$ |  | 0.00 | 0.00 | 79.8 |
| All Vehicles |  | 1182 | 7.5 | 0.328 | 2.7 | Not <br> Applicable | 11 | 0.08 | 0.12 | 73.7 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - PM (plus development)
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## SIDRA INTERSECTION

Movement Summary
Masters Rd / Springhill Rd
Future (2019) - AM (plus project)

Signalised - Fixed time
Cycle Time $\mathbf{= 1 1 0} \mathbf{~ s e c o n d s}$
Vehicle Movements

| Mov ID | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 564 | 0.0 | 0.186 | 13.3 | LOS A | 31 | 0.23 | 0.73 | 55.9 |
| 2 | T | 2613 | 0.0 | 0.862 | 30.1 | LOS C | 306 | 0.94 | 0.92 | 40.7 |
| Approach |  | 3177 | 0.0 | 0.862 | 27.1 | LOS B | 306 | 0.82 | 0.89 | 42.7 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 677 | 0.0 | 0.335 | 16.4 | LOS B | 84 | 0.62 | 0.54 | 52.4 |
| 9 | R | 336 | 0.0 | 0.829 | 73.3 | LOS F | 58 | 1.00 | 0.91 | 23.6 |
| Approach |  | 1013 | 0.0 | 0.829 | 35.3 | Los C | 84 | 0.75 | 0.66 | 37.4 |
| Masters Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 557 | 0.0 | 0.539 | 39.0 | LOS C | 117 | 0.81 | 0.82 | 35.3 |
| 12 | R | 752 | 0.0 | 0.550 | 50.4 | LOS D | 93 | 0.93 | 0.83 | 30.4 |
| Approach |  | 1309 | 0.0 | 0.550 | 45.6 | LOS D | 117 | 0.87 | 0.83 | 32.3 |
| All Vehicles |  | 5499 | 0.0 | 0.862 | 33.0 | Los C | 306 | 0.82 | 0.83 | 38.8 |

Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Bueue of <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P5 | 50 | 49.2 | LOS E | 0 | 0.95 | 0.95 |
| All Peds | 50 | 49.2 | LOS D | 0 | 0.95 | 0.95 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
iviovement summary
\# - Density for continuous movement

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## SIDRA INTERSECTION

Movement Summary
Masters Rd / Springhill Rd
Future (2019) - PM (plus project)

```
Signalised - Fixed time Cycle Time = 110 seconds
```

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 916 | 0.0 | 0.377 | 20.3 | LOS B | 88 | 0.49 | 0.79 | 48.3 |
| 2 | T | 1375 | 0.0 | 0.663 | 32.2 | LOS C | 149 | 0.90 | 0.79 | 39.3 |
| Approach |  | 2291 | 0.0 | 0.663 | 27.4 | LOS B | 149 | 0.74 | 0.79 | 42.5 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| 8 | T | 1224 | 0.0 | 0.885 | 46.9 | LOS D | 250 | 1.00 | 1.02 | 31.9 |
| 9 | R | 1122 | 0.0 | 0.852 | 62.5 | LOS E | 155 | 1.00 | 0.97 | 26.4 |
| Approach |  | 2346 | 0.0 | 0.885 | 54.3 | LOS D | 250 | 1.00 | 1.00 | 29.0 |
| Masters Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 228 | 0.0 | 0.159 | 24.5 | LOS B | 40 | 0.53 | 0.76 | 44.6 |
| 12 | R | 702 | 0.0 | 0.513 | 50.0 | LOS D | 87 | 0.92 | 0.82 | 30.6 |
| Approach |  | 930 | 0.0 | 0.513 | 43.7 | LOS D | 87 | 0.82 | 0.81 | 33.1 |
| All Vehicles |  | 5567 | 0.0 | 0.885 | 41.5 | Los C | 250 | 0.86 | 0.88 | 34.2 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P5 | 50 | 49.2 | LOS E | 0 | 0.95 | 0.95 |
| All Peds | 50 | 49.2 | LOS D | 0 | 0.95 | $\mathbf{0 . 9 5}$ |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
iviovement summary
\# - Density for continuous movement

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## SIDRA -- <br> INTERSECTION

## Movement Summary

## Picton Rd / Almond St (Wilton Rd)

Future (2019) - AM (plus project)

Two-way stop

## Vehicle Movements

| Mov ID Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Picton Rd (east) |  |  |  |  |  |  |  |  |  |
| ET T | 704 | 13.5 | 0.393 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 100.0 |
| ER R | 14 | 30.8 | 0.045 | 25.6 | LOS B | 2 | 0.76 | 0.94 | 47.6 |
| Approach | 717 | 13.8 | 0.393 | 0.5 | LOS A | 2 | 0.01 | 0.02 | 98.5 |
| Median Storage |  |  |  |  |  |  |  |  |  |
| $5 \quad T$ | 142 | 3.5 | 0.388 | 11.8 | LOS A | 13 | 0.75 | 0.99 | 36.6 |
| Approach | 142 | 3.5 | 0.388 | 11.8 | LOS A | 13 | 0.75 | 0.99 | 36.6 |
| Almond St (north) |  |  |  |  |  |  |  |  |  |
| NL L | 18 | 0.0 | 0.692 | 42.3 | LOS C | 36 | 0.92 | 1.23 | 34.2 |
| NR $\quad$ R | 142 | 3.5 | 0.689 | 44.3 | LOS D | 36 | 0.92 | 1.23 | 27.3 |
| Approach | 160 | 3.1 | 0.691 | 44.0 | Los D | 36 | 0.92 | 1.23 | 28.3 |
| Picton Rd (west) |  |  |  |  |  |  |  |  |  |
| WL L | 104 | 8.7 | 0.059 | 13.1 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.76 | 63.3 |
| WT T | 835 | 12.1 | 0.462 | 0.0 | $\operatorname{LOS} A$ | 0 | 0.00 | 0.00 | 100.0 |
| Approach | 939 | 11.7 | 0.462 | 1.4 | Los A |  | 0.00 | 0.08 | 95.3 |
| All Vehicles | 1958 | 11.2 | 0.692 | 5.3 | Not Applicable | 36 | 0.13 | 0.22 | 80.9 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA ---

## Movement Summary

Piston Rd / Almond St (Wilton Rd)
Future (2019) - PM (plus project)

Two-way stop

## Vehicle Movements



Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA <br> Movement Summary

Springhill Rd / Port Kembla Rd
Future (2019) - AM (plus project)
Signalised - Fixed time Cycle Time $=\mathbf{5 0}$ seconds

Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow/ } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Port Kembla Rd (south) |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 71 | 0.0 | 0.091 | 17.2 | LOS B | 11 | 0.63 | 0.73 | 39.0 |
| 3 | R | 8 | 0.0 | 0.036 | 29.2 | LOS C | 2 | 0.91 | 0.66 | 30.4 |
| Approach |  | 79 | 0.0 | 0.091 | 18.4 | LOS B | 11 | 0.66 | 0.72 | 38.0 |
| Springhill Rd (east) |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 12 | 0.0 | 0.252 | 20.6 | LOS B | 30 | 0.76 | 0.77 | 37.2 |
| 5 | T | 489 | 0.0 | 0.252 | 13.1 | LOS A | 30 | 0.76 | 0.62 | 48.0 |
| Approach |  | 501 | 0.0 | 0.252 | 13.3 | LOS A | 30 | 0.76 | 0.62 | 47.7 |
| Sprinhill Rd (west) |  |  |  |  |  |  |  |  |  |  |
| 11 | T | 1093 | 0.0 | 0.280 | 4.1 | LOS A\# | 7\# | 0.00 | 0.32 | 70.8 |
| 12 | R | 78 | 0.0 | 0.233 | 30.0 | LOS C | 18 | 0.89 | 0.76 | 36.9 |
| Approach |  | 1171 | 0.0 | 0.280 | 5.8 | LOS A | 18 | 0.06 | 0.35 | 67.4 |
| All Vehicles |  | 1751 | 0.0 | 0.280 | 8.5 | Los A | 30 | 0.29 | 0.44 | 58.5 |

Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 50 | 15.2 | LOS B | 0 | 0.78 | 0.78 |
| All Peds | 50 | $\mathbf{1 5 . 2}$ | LOS B | 0 | 0.78 | 0.78 |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA <br> Movement Summary

Springhill Rd / Port Kembla Rd

## Future (2019) - PM (plus project)

Signalised - Fixed time Cycle Time $=\mathbf{5 0}$ seconds

Vehicle Movements

| Mov ID | Turn | Dem <br> Flow <br> (veh/h) | \% |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| P1 | 50 | 13.0 | LOS B | 0 | 0.72 | 0.72 |
| $\mathbf{1 3 . 0}$ | LOS A | 0 | 0.72 | 0.72 |  |  |

Symbols which may appear in this table:
Following Degree of Saturation
$\# x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA INTERSECTION <br> Movement Summary

Springhill Rd / Recycling Rd
Future (2019) - AM (plus project)

```
Signalised - Actuated isolated
Cycle Time \(=\mathbf{1 0 8}\) seconds
```

Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% <br> Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 56 | 49.1 | 0.974 | 67.0 | LOS E | 410 | 1.00 | 1.11 | 22.4 |
| ST | T | 2481 | 4.6 | 0.972 | 54.1 | LOS D | 409 | 1.00 | 1.11 | 29.2 |
| SR | R | 64 | 16.9 | 0.303 | 60.2 | LOS E | 32 | 0.92 | 0.76 | 24.0 |
| Approach |  | 2601 | 5.9 | 0.972 | 54.5 | Los D | 410 | 1.00 | 1.10 | 29.0 |
| Bluescope Steel (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 28 | 7.1 | 0.149 | 53.4 | LOS D | 13 | 0.89 | 0.71 | 22.5 |
| ET | T | 3 | 66.7 | 0.017 | 46.0 | LOS D | 2 | 0.88 | 0.56 | 25.0 |
| ER | R | 9 | 22.2 | 0.101 | 62.9 | LOS E | 5 | 0.96 | 0.67 | 20.8 |
| Approach |  | 40 | 15.0 | 0.149 | 55.0 | LOS D | 13 | 0.91 | 0.69 | 22.3 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 37 | 5.4 | 0.027 | 13.8 | LOS A | 2 | 0.14 | 0.97 | 53.9 |
| NT | T | 1351 | 8.4 | 0.526 | 22.9 | LOS B | 142 | 0.74 | 0.66 | 46.1 |
| NR | R | 69 | 54.3 | 0.404 | 63.0 | LOS E | 45 | 0.94 | 0.77 | 23.5 |
| Approach |  | 1458 | 10.6 | 0.526 | 24.6 | LOS B | 142 | 0.74 | 0.67 | 44.6 |
| Recycling Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 57 | 75.4 | 0.396 | 58.7 | LOS E | 46 | 0.93 | 0.77 | 21.8 |
| WT | T | 6 | 50.0 | 0.396 | 50.5 | LOS D | 46 | 0.93 | 0.73 | 23.9 |
| WR | R | 24 | 70.8 | 0.350 | 66.8 | LOS E | 19 | 0.97 | 0.72 | 20.3 |
| Approach |  | 87 | 72.4 | 0.396 | 60.4 | Los E | 46 | 0.95 | 0.75 | 21.5 |
| All Vehicles |  | 4186 | 9.0 | 0.974 | 44.2 | LOS D | 410 | 0.91 | 0.94 | 32.6 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SP | 11 | 53.2 | LOS E | 0 | 0.95 | 0.95 |
| EP | 21 | 48.2 | LOS E | 0 | 0.94 | 0.94 |
| WP | 21 | 48.2 | LOS E | 0 | 0.94 | 0.94 |


| All Peds | 53 | 49.2 | LOS D | 0 | 0.95 | 0.95 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

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## SIDRA $\overline{\text { INTERSECTION }}$

## Movement Summary

## Springhill Rd / Recycling Rd

Future (2019) - PM (plus project)

Signalised - Actuated isolated

## Vehicle Movements

| Mov ID | Turn | Dem <br> Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Springhill Rd (south) |  |  |  |  |  |  |  |  |  |  |
| SL | L | 11 | 20.0 | 0.669 | 33.1 | Los C | 167 | 0.82 | 0.86 | 35.6 |
| ST | T | 1827 | 1.5 | 0.679 | 21.4 | LOS B | 167 | 0.82 | 0.74 | 47.4 |
| SR | R | 28 | 7.1 | 0.243 | 59.1 | LOS E | 12 | 0.96 | 0.72 | 24.2 |
| Approach |  | 1865 | 1.7 | 0.679 | 22.0 | Los B | 167 | 0.82 | 0.74 | 46.8 |
| Bluescope Steel (east) |  |  |  |  |  |  |  |  |  |  |
| EL | L | 64 | 16.9 | 0.323 | 47.2 | LOS D | 27 | 0.91 | 0.75 | 23.9 |
| ET | T | 6 | 50.0 | 0.029 | 38.4 | LOS C | 3 | 0.87 | 0.58 | 27.0 |
| ER | R | 37 | 5.4 | 0.317 | 54.6 | LOS D | 16 | 0.97 | 0.72 | 22.3 |
| Approach |  | 108 | 14.8 | 0.323 | 49.3 | LOS D | 27 | 0.93 | 0.73 | 23.5 |
| Springhill Rd (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 9 | 22.2 | 0.007 | 14.7 | LOS B | 0 | 0.14 | 0.96 | 54.0 |
| NT | T | 2484 | 1.7 | 0.918 | 32.6 | Los C | 287 | 0.99 | 0.98 | 39.1 |
| NR | R | 25 | 72.0 | 0.313 | 62.9 | LOS E | 17 | 0.96 | 0.72 | 23.7 |
| Approach |  | 2518 | 2.4 | 0.918 | 32.8 | Los C | 287 | 0.99 | 0.98 | 38.9 |
| Recycling Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 89 | 21.1 | 0.398 | 48.4 | LOS D | 40 | 0.93 | 0.77 | 23.7 |
| WT | T | 1 | 0.0 | 0.393 | 41.8 | LOS C | 40 | 0.93 | 0.73 | 26.1 |
| WR | R | 48 | 6.2 | 0.414 | 55.2 | LOS D | 21 | 0.97 | 0.74 | 22.2 |
| Approach |  | 139 | 15.8 | 0.414 | 50.7 | LOS D | 40 | 0.94 | 0.76 | 23.2 |
| All Vehicles |  | 4630 | 2.8 | 0.918 | 29.4 | Los C | 287 | 0.92 | 0.87 | 40.3 |

## Pedestrian Movements

| Mov ID | Dem Flow <br> (ped/h) | Aver <br> Delay <br> (sec) | Level of <br> Service | 95\% <br> Back of <br> Queue <br> (m) | Prop. <br> Queued | Eff. Stop <br> Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SP | 11 | 45.7 | LOS E | 0 | 0.94 | 0.94 |
| EP | 21 | 40.2 | LOS E | 0 | 0.93 | 0.93 |
| WP | 21 | 40.2 | LOS E | 0 | 0.93 | 0.93 |


| All Peds | 53 | 41.3 | LOS C | 0 | 0.94 | 0.94 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Symbols which may appear in this table:
Following Degree of Saturation
\# $\mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

```
J
SIDRA SOLUTIONS
```

Site: FU (2019) - PM (plus project)
T:\2007\07267\Survey Results \& Modelling\SIDRA_090521\Springhill - Recycling.aap Processed May 21, 2009 11:45:43PM

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## SIDRA - - <br> INTERSECTION

## Movement Summary

Wilton Rd / Douglas Park Dr
Future (2019) - AM (plus project)

Two-way stop
Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow/ } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 45 | 0.0 | 0.079 | 0.3 | LOS A | 3 | 0.17 | 0.00 | 76.0 |
| ER | R | 76 | 0.0 | 0.079 | 11.0 | LOS A | 3 | 0.17 | 0.69 | 58.3 |
| Approach |  | 121 | 0.0 | 0.079 | 7.0 | Los A | 3 | 0.17 | 0.43 | 63.9 |
| Douglas Park Dr (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 84 | 1.2 | 0.387 | 16.2 | LOS B | 20 | 0.47 | 0.85 | 53.4 |
| NR | R | 187 | 4.8 | 0.388 | 16.4 | LOS B | 20 | 0.47 | 0.96 | 53.6 |
| Approach |  | 271 | 3.7 | 0.388 | 16.3 | LOS B | 20 | 0.47 | 0.93 | 53.5 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 151 | 6.0 | 0.119 | 11.7 | LOS A | 5 | 0.18 | 0.64 | 57.6 |
| WT | T | 67 | 3.0 | 0.035 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 217 | 5.1 | 0.119 | 8.1 | LOS A | 5 | 0.13 | 0.44 | 63.1 |
| All Vehicles |  | 609 | 3.4 | 0.388 | 11.5 | Applicable | 20 | 0.29 | 0.66 | 58.6 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - AM (plus project)
T:\2007\07267\Survey Results \& Modelling\SIDRA_090521\Wilton - Douglas Park.aap Processed May 21, 2009 11:49:48PM

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## SIDRA -- <br> INTERSECTION

## Movement Summary

Wilton Rd / Douglas Park Dr
Future (2019) - PM (plus project)

Two-way stop

## Vehicle Movements

| Mov ID | Turn | Dem Flow (veh/h) | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wilton Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 100 | 0.0 | 0.131 | 0.3 | LOS A | 6 | 0.17 | 0.00 | 75.9 |
| ER | R | 108 | 0.0 | 0.131 | 11.0 | Los A | 6 | 0.17 | 0.69 | 58.2 |
| Approach |  | 208 | 0.0 | 0.131 | 5.8 | LOS A | 6 | 0.17 | 0.36 | 65.7 |
| Douglas Park Dr (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 104 | 1.0 | 0.545 | 20.0 | LOS B | 38 | 0.59 | 0.96 | 49.6 |
| NR | R | 225 | 3.6 | 0.545 | 20.0 | Los B | 38 | 0.59 | 1.09 | 49.7 |
| Approach |  | 329 | 2.7 | 0.545 | 20.0 | Los B | 38 | 0.59 | 1.05 | 49.7 |
| Wilton Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 214 | 3.8 | 0.167 | 11.7 | LOS A | 7 | 0.23 | 0.64 | 57.3 |
| WT | T | 63 | 0.0 | 0.032 | 0.0 | LOS A | 0 | 0.00 | 0.00 | 80.0 |
| Approach |  | 276 | 2.9 | 0.167 | 9.0 | Los A | 7 | 0.18 | 0.50 | 61.3 |
| All Vehicles |  | 813 | 2.1 | 0.545 | 12.7 | $\begin{gathered} \text { Not } \\ \text { Applicable } \end{gathered}$ | 38 | 0.34 | 0.68 | 56.9 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $\mathrm{x}=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - PM (plus project)
T:\2007\07267\Survey Results \& Modelling\SIDRA_090521\Wilton - Douglas Park.aap
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## attachment d4

future performance of modified intersection arrangements

## SIDRA INTERSECTION

## Movement Summary

## Appin Rd - West Cliff Colliery Access

Future (2019) - AM (plus project) - Signals
Signalised $\boldsymbol{-}$ Fixed time $\quad$ Cycle Time $=\mathbf{4 0}$ seconds
Vehicle Movements

| Mov ID | Turn | $\begin{aligned} & \text { Dem } \\ & \text { Flow } \\ & \text { (veh/h) } \end{aligned}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 760 | 1.4 | 0.715 | 8.4 | LOS A | 95 | 0.80 | 0.74 | 76.6 |
| ER | R | 52 | 100.0 | 0.256 | 24.4 | LOS B | 13 | 0.60 | 0.77 | 48.9 |
| Approach |  | 812 | 7.8 | 0.715 | 9.4 | LOS A | 95 | 0.78 | 0.74 | 74.3 |
| West Cliff Access (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 55 | 100.0 | 0.189 | 16.5 | LOS B | 7 | 0.44 | 0.75 | 53.9 |
| NR | R | 46 | 100.0 | 0.260 | 32.8 | LOS C | 18 | 0.92 | 0.75 | 44.1 |
| Approach |  | 101 | 100.0 | 0.260 | 24.0 | LOS B | 18 | 0.66 | 0.75 | 48.9 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 61 | 37.7 | 0.076 | 19.3 | LOS B | 8 | 0.49 | 0.77 | 55.8 |
| WT | T | 465 | 2.8 | 0.221 | 5.0 | LOS A | 24 | 0.54 | 0.45 | 82.8 |
| Approach |  | 527 | 6.8 | 0.221 | 6.7 | LOS A | 24 | 0.54 | 0.49 | 79.4 |
| All Vehicles |  | 1440 | 13.9 | 0.715 | 9.4 | Los A | 95 | 0.68 | 0.65 | 72.5 |

Symbols which may appear in this table:
Following Degree of Saturation
\# x = 1.00 for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements
Following Queue
\# - Density for continuous movement

Site: FU (2019) - AM (plus project) - Signals_v2
T:\2007\07267\Survey Results \& Modelling\SIDRA_090521\West Cliff Access.aap
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## SIDRA -- <br> INTERSECTION

## Movement Summary

## Appin Rd - West Cliff Colliery Access

Future (2019) - PM (plus project) - Signals
Signalised - Fixed time Cycle Time $=\mathbf{3 0}$ seconds
Vehicle Movements

| Mov ID | Turn | $\begin{gathered} \text { Dem } \\ \text { Flow } \\ \text { (veh/h) } \end{gathered}$ | \%HV | Deg of Satn (v/c) | Aver Delay (sec) | Level of Service | 95\% Back of Queue (m) | Prop. Queued | Eff. Stop Rate | Aver Speed (km/h) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Appin Rd (east) |  |  |  |  |  |  |  |  |  |  |
| ET | T | 485 | 1.6 | 0.629 | 8.5 | LOS A | 56 | 0.85 | 0.75 | 75.4 |
| ER | R | 41 | 100.0 | 0.248 | 27.5 | LOS B | 11 | 0.79 | 0.76 | 46.1 |
| Approach |  | 526 | 9.3 | 0.628 | 10.0 | LOS A | 56 | 0.85 | 0.75 | 72.3 |
| West Cliff Access (north) |  |  |  |  |  |  |  |  |  |  |
| NL | L | 52 | 100.0 | 0.167 | 17.8 | LOS B | 8 | 0.61 | 0.76 | 53.0 |
| NR | R | 42 | 66.7 | 0.154 | 24.8 | LOS B | 10 | 0.86 | 0.74 | 44.6 |
| Approach |  | 94 | 85.1 | 0.167 | 20.9 | LOS B | 10 | 0.72 | 0.75 | 49.5 |
| Appin Rd (west) |  |  |  |  |  |  |  |  |  |  |
| WL | L | 32 | 100.0 | 0.074 | 24.6 | LOS B | 6 | 0.64 | 0.75 | 48.7 |
| WT | T | 666 | 0.5 | 0.428 | 7.4 | LOS A | 36 | 0.77 | 0.64 | 77.3 |
| Approach |  | 698 | 5.0 | 0.428 | 8.2 | LOS A | 36 | 0.76 | 0.65 | 75.6 |
| All Vehicles |  | 1318 | 12.4 | 0.629 | 9.8 | LOS A | 56 | 0.79 | 0.70 | 71.2 |

Symbols which may appear in this table:
Following Degree of Saturation
\# $x=1.00$ for Short Lane with resulting Excess Flow

* $x=1.00$ due to minimum capacity

Following LOS
\# - Based on density for continuous movements

Following Queue
\# - Density for continuous movement

Site: FU (2019) - PM (plus project) - Signals
T:\2007\07267\Survey Results \& Modelling\SIDRA Files\Appin Rd \& West Cliff Access.aap Processed Apr 29, 2009 02:50:36AM

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## attachment e

additional project traffic distribution at key intersections

AM TURNING COUNT MOVEMENTS (ADDITIONAL)
ASAT 21 MAY 2009

## PM TURNING COUNT MOVEMENTS (ADDITIONAL)

$$
\begin{aligned}
& \text { Man Access Rd }
\end{aligned}
$$

$$
\begin{aligned}
& \text { Springhill Rd }
\end{aligned}
$$

| AM PEAK HOUR STAFF MOVEMENTS (includes car pooling of | .2 per car) |  |  |
| :--- | ---: | ---: | ---: | ---: |
| APPIN WEST (80\% of Appin Staff) | IN | OUT | COMBINED |
| appin | 5 | 5 | 10 |
| Bulli pass | 31 | 31 | 62 |
| Camden | 2 | 2 | 3 |
| Campbelltown | 6 | 6 | 11 |
| Douglas Park | 2 | 2 | 4 |
| Mittagong | 2 | 2 | 4 |
| Picton | 5 | 5 | 10 |
| Sydney | 2 | 2 | 4 |
| Wgong south | 50 | 50 | 100 |
| Wilton | 2 | 2 | 3 |
| TOTAL | 105 | 105 | 210 |


| AM PEAK HOUR STAFF MOVEMENTS (includes car pooling of | 1.2 per car) |  |  |
| :--- | :---: | :---: | ---: |
| APPIN EAST (20\% of Appin Staff) | IN | OUT | COMBINED |
| appin |  | 1 | 1 |
| Bulli pass | 8 | 8 | 3 |
| Camden | 0 | 0 | 15 |
| Campbelltown | 1 | 1 | 1 |
| Douglas Park | 1 | 1 | 3 |
| Mittagong | 0 | 0 | 1 |
| Picton | 1 | 1 | 1 |
| Sydney | 0 | 0 | 2 |
| Wgong south | 12 | 12 | 1 |
| Wilton | 0 | 0 | 25 |
| TOTAL | 26 | 26 | 1 |
|  |  |  |  |


| AM PEAK HOUR STAFF MOVEMENTS (includes car pooling of | 1.5 per car) |  |  |
| :--- | ---: | ---: | ---: |
| WEST CLIFF | IN | OUT | COMBINED |
| appin |  | 1 | 1 |
| Bulli pass | 32 | 32 | 2 |
| Camden | 2 | 2 | 64 |
| Campbelltown | 2 | 2 | 4 |
| Douglas Park | 0 | 0 | 3 |
| Mittagong | 3 | 3 | 1 |
| Picton | 2 | 2 | 7 |
| Sydney | 4 | 4 | 5 |
| Wgong south | 42 | 42 | 8 |
| Wilton | 1 | 1 | 84 |
| TOTAL | 89 | 89 | 179 |

FUTURE with Project

| AM PEAK HOUR STAFF MOVEME | TS | des | ng o | car) | PM PEAK HOUR STAFF MOVEM | NTS | udes | ing | of 1.2 per car |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| APPIN WEST (80\% of Appin Staff) | IN |  |  | NED | APPIN WEST (80\% of Appin Staff) | IN |  |  | COMBINED |
| appin |  | 12 | 12 | 24 | appin |  | 18 | 18 | 35 |
| Bulli pass |  | 71 | 71 | 141 | Bulli pass |  | 106 | 106 | 212 |
| Camden |  | 3 | 3 | 7 | Camden |  | 5 | 5 | 10 |
| Campbelltown |  | 13 | 13 | 26 | Campbelltown |  | 20 | 20 | 39 |
| Douglas Park |  | 5 | 5 | 10 | Douglas Park |  | 7 | 7 | 15 |
| Mittagong |  | 4 | 4 | 8 | Mittagong |  | 6 | 6 | 12 |
| Picton |  | 11 | 11 | 22 | Picton |  | 17 | 17 | 33 |
| Sydney |  | 4 | 4 | 8 | Sydney |  | 6 | 6 | 12 |
| Wgong south |  | 114 | 114 | 228 | Wgong south |  | 171 | 171 | 342 |
| Wilton |  | 3 | 3 | 7 | Wilton |  | 5 | 5 | 10 |
| TOTAL |  | 241 | 241 | 481 | TOTAL |  | 361 | 361 | 722 |

AM PEAK HOUR STAFF MOVEMENTS (includes car pooling of 1.2 per car)

| APPIN EAST (20\% of Appin Staff) | IN | OUT | COMBINED |
| :--- | ---: | ---: | ---: |
| appin | 3 | 3 | 6 |
| Bulli pass | 18 | 18 | 35 |
| Camden | 1 | 1 | 2 |
| Campbelltown | 3 | 3 | 7 |
| Douglas Park | 1 | 1 | 2 |
| Mittagong | 1 | 1 | 2 |
| Picton | 3 | 3 | 6 |
| Sydney | 1 | 1 | 2 |
| Wgong south | 28 | 28 | 57 |
| Wilton | 1 | 1 | 2 |
| TOTAL | 60 | 60 | 120 |

AM PEAK HOUR STAFF MOVEMENTS (includes car pooling of 1.5 per car)
WEST CLIFF IN OUT COMBINED
appin
Bulli pass COMBINED

Camden

| 0 | 0 | 0 |
| ---: | ---: | ---: |
| 7 | 7 | 14 |
| 0 | 0 | 1 |
| 0 | 0 | 1 |
| 0 | 0 | 0 |
| 1 | 1 | 1 |
| 1 | 1 | 1 |
| 1 | 1 | 2 |
| 9 | 9 | 18 |
| 0 | 0 | 0 |
| 20 | 20 | 39 |

PM PEAK HOUR STAFF MOVEMENTS (includes car pooling of 1.2 per car) APPIN EAST (20\% of Appin Staff) IN OUT COMBINED
appin

| 4 | 4 | 9 |
| ---: | ---: | ---: |
| 26 | 26 | 53 |
| 1 | 1 | 3 |
| 5 | 5 | 10 |
| 2 | 2 | 4 |
| 2 | 2 | 3 |
| 4 | 4 | 8 |
| 2 | 2 | 3 |
| 43 | 43 | 85 |
| 1 | 1 | 3 |
| 90 | 90 | 180 |

Campbelltown
Douglas Park
Mittagong
Camden
3
10
4

| Campbelltown | 5 | 5 | 10 |
| :--- | :--- | :--- | ---: |
| Douglas Park | 2 | 2 | 4 |


| Mittagong | 2 | 2 | 3 |
| :--- | :--- | :--- | :--- |
| Picton | 4 | 4 | 8 |


| Sydney | 2 | 2 | 3 |
| :--- | ---: | ---: | ---: |
| Wgong south | 43 | 43 | 85 |
| Wilton | 1 | 1 | 3 |


| TOTAL | 90 | 90 | 180 |
| :--- | :--- | :--- | :--- |

PM PEAK HOUR STAFF MOVEMENTS (includes car pooling of 1.5 per car)

| WEST CLIFF | IN | OUT | COMBINED |
| :--- | ---: | ---: | ---: | ---: |
| appin | 0 | 0 | 1 |
| Bulli pass | 10 | 10 | 21 |
| Camden | 1 | 1 | 1 |
| Campbelltown | 1 | 1 | 1 |
| Douglas Park | 0 | 0 | 0 |
| Mittagong | 1 | 1 | 2 |
| Picton | 1 | 1 | 2 |
| Sydney | 1 | 1 | 3 |
| Wgong south | 14 | 14 | 28 |
| Wilton | 0 | 0 | 0 |
| TOTAL | 29 | 29 | 59 |

RELATIVE CHANGE

| AM PEAK HOUR STAFF MOVEMENTS (includes car pooling of | .2 per car) |  |  |  |
| :--- | ---: | ---: | ---: | ---: |
| APPIN WEST (80\% of Appin Staff) | IN | OUT | COMBINED |  |
| appin |  | 7 | 7 | 13 |
| Bulli pass |  | 40 | 40 | 79 |
| Camden | 2 | 2 | 4 |  |
| Campbelltown | 7 | 7 | 15 |  |
| Douglas Park | 3 | 3 | 5 |  |
| Mittagong | 2 | 2 | 5 |  |
| Picton | 6 | 6 | 12 |  |
| Sydney | 2 | 2 | 5 |  |
| Wgong south | 64 | 64 | 128 |  |
| Wilton | 2 | 2 | 4 |  |
| TOTAL | 135 | 135 | 271 |  |


| AM PEAK HOUR STAFF MOVEMENTS (includes car pooling of | .2 per car) |  |  |  |
| :--- | ---: | ---: | ---: | ---: |
| APPIN EAST (20\% of Appin Staff) | IN | OUT | COMBINED |  |
| appin |  | 2 | 2 | 3 |
| Bulli pass | 10 | 10 | 20 |  |
| Camden | 0 | 0 | 1 |  |
| Campbelltown | 2 | 2 | 4 |  |
| Douglas Park | 1 | 1 | 1 |  |
| Mittagong | 1 | 1 | 1 |  |
| Picton | 2 | 2 | 3 |  |
| Sydney | 1 | 1 | 1 |  |
| Wgong south | 16 | 16 | 32 |  |
| Wilton | 0 | 0 | 1 |  |
| TOTAL | 34 | 34 | 67 |  |

AM PEAK HOUR STAFF MOVEMENTS (includes car pooling of 1.5 per car)

| WEST CLIFF | IN | OUT | COMBINED |  |
| :--- | ---: | ---: | ---: | ---: |
| appin |  | -1 | -1 | -1 |
| Bulli pass |  | -25 | -25 | -50 |
| Camden | -2 | -2 | -3 |  |
| Campbelltown | -1 | -1 | -3 |  |
| Douglas Park | 0 | 0 | 0 |  |
| Mittagong | -3 | -3 | -5 |  |
| Picton | -2 | -2 | -4 |  |
| Sydney | -3 | -3 | -6 |  |
| Wgong south | -33 | -33 | -66 |  |
| Wilton | 0 | 0 | -1 |  |
| TOTAL | -70 | -70 | -140 |  |


| PM PEAK HOUR STAFF MOVEMENTS (includes car pooling of | .2 per car) |  |  |
| :--- | ---: | ---: | ---: |
| APPIN WEST (80\% of Appin Staff) | OUT | COMBINED |  |
| appin | 10 | 10 | 20 |
| Bulli pass | 60 | 60 | 119 |
| Camden | 3 | 3 | 6 |
| Campbelltown | 11 | 11 | 22 |
| Douglas Park | 4 | 4 | 8 |
| Mittagong | 4 | 4 | 7 |
| Picton | 9 | 9 | 19 |
| Sydney | 4 | 4 | 7 |
| Wgong south | 96 | 96 | 193 |
| Wilton | 3 | 3 | 6 |
| TOTAL | 203 | 203 | 406 |

PM PEAK HOUR STAFF MOVEMENTS (includes car pooling of 1.2 per car)

| APPIN EAST (20\% of Appin Staff) | IN | OUT | COMBINED |  |
| :--- | ---: | ---: | ---: | ---: |
| appin | 2 | 2 | 5 |  |
| Bulli pass |  | 15 | 15 | 30 |
| Camden | 1 | 1 | 1 |  |
| Campbelltown | 3 | 3 | 6 |  |
| Douglas Park | 1 | 1 | 2 |  |
| Mittagong | 1 | 1 | 2 |  |
| Picton | 2 | 2 | 5 |  |
| Sydney | 1 | 1 | 2 |  |
| Wgong south | 24 | 24 | 48 |  |
| Wilton | 1 | 1 | 1 |  |
| TOTAL | 51 | 51 | 101 |  |

PM PEAK HOUR STAFF MOVEMENTS (includes car pooling of 1.5 per car)

| WEST CLIFF | IN | OUT | COMBINED |  |
| :--- | ---: | ---: | ---: | ---: |
| appin |  | -1 | -1 | -2 |
| Bulli pass |  | -37 | -37 | -75 |
| Camden | -3 | -3 | -5 |  |
| Campbelltown | -2 | -2 | -4 |  |
| Douglas Park | 0 | 0 | -1 |  |
| Mittagong | -4 | -4 | -8 |  |
| Picton | -3 | -3 | -6 |  |
| Sydney | -5 | -5 | -10 |  |
| Wgong south | -49 | -49 | -99 |  |
| Wilton | -1 | -1 | -1 |  |
| TOTAL | -105 | -105 | -209 |  |


[^0]:    1 On the 12 June 2009, the Minister for Planning approved the PKCT Project.

[^1]:    ${ }^{1}$ With the exception of a 600 m (route back to Appin Road from Port Kembla) and 1 km (route back to Appin Road from F6 Southern Freeway [north]) section of Princes Highway at the Mount Ousley Road/Appin Road Interchange.

[^2]:    ${ }^{2}$ Mid-block peak hour surveys only.

[^3]:    * Data fitted from 2007 High Output Surveys in PKCT Traffic Study (i.e. $80 \%$ truck trips occur on weekdays, 20\% weekends) (Cardno Eppell Olsen, 2008).
    ** ROM coal from Appin East pit top.

[^4]:    ${ }^{1} 2008$ Survey data from Cardno Eppell Olsen (2008).
    ${ }^{2} 2007$ Survey data from Metropolitan Traffic Study (Masson Wilson Twiney, 2008).
    Note: No turning vehicles in PM peak at Corrimal Coke Works intersection.
    $\mathrm{s} / \mathrm{veh}=$ seconds per vehicle.

[^5]:    

[^6]:    Site: EX - PM
    T:\2007\07267\Survey Results \& Modelling\SIDRA Files\Appin Rd \& West Cliff Access.aap Processed Apr 17, 2009 05:06:27PM

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[^7]:    Site: EX - AM Processed Apr 21, 2009 11:56:12AM

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[^8]:    Site: EX - PM
    T:\2007\07267\Survey Results \& Modelling\SIDRA Files\Picton Rd \& Almond St.aap Processed Apr 21, 2009 11:56:47AM

[^9]:    Site: FU (2019) - PM (plus Development)
    $\mathrm{T}: \ 2007 \backslash 07267 \backslash$ Survey Results \& Modelling\SIDRA 090521\Picton - Almond.aap Processed May 21, 2009 11:37:57PM

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